



May 11, 2026

Project Name: Madison County Schools - Madison Central Press Box Improvements

Project Number: 25-053

## **ADDENDUM NO. 01**

### **NOTICE TO ALL DOCUMENT HOLDERS:**

The following additions, deletions, changes, and clarifications to the drawings and specifications are to be included as part of the Contract Documents.

### **GENERAL**

**ITEM NO. 01      Pre-Bid Conference**

A Pre-Bid Conference is scheduled for Wednesday, May 13, 2026, at 2:00 pm.  
The Conference will be held at the Madison County Schools Central Office: 476  
Highland Colony Pkwy; Ridgeland, MS 39157

### **SPECIFICATIONS**

**ITEM NO. 02      Geotechnical Report**

ADD the attached Report of Geotechnical Exploration as part of the Project  
Manual.

Encl:    **RFIs:** n/a  
          **Specifications** (22 pages, 8.5x11): Report of Geotechnical Exploration  
          **Drawings** (24x36): n/a  
Cc:      All document holders  
          File : 23-034/C/C2

# **LADNER TESTING, INC.**

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## **REPORT OF GEOTECHNICAL EXPLORATION**

**FOR**

### **MADISON CENTRAL HIGH SCHOOL STADIUM UPGRADES MADISON COUNTY, MISSISSIPPI**

**FEBRUARY 2026**

**Prepared By:**

**W Geotechnical and Testing, Inc.  
301 Central Avenue East  
Wiggins, MS 39577**

# LADNER TESTING, INC.

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February 27 2026

Wier Boerner Allin Architecture  
2727 Old Canton Road, Suite 200  
Jackson, MS 39216

RE: Report of Geotechnical Exploration  
Madison Central High School Stadium Upgrades  
Madison County, Mississippi

W Geotechnical Project No. G-1808Q  
Ladner Project No. 0122-26-A

Dear Sir or Madam:

Thank you for retaining Ladner Testing Inc. to complete a geotechnical exploration for the above referenced site. The results of the subsurface exploration, along with boring logs and our engineering report, are attached to this letter.

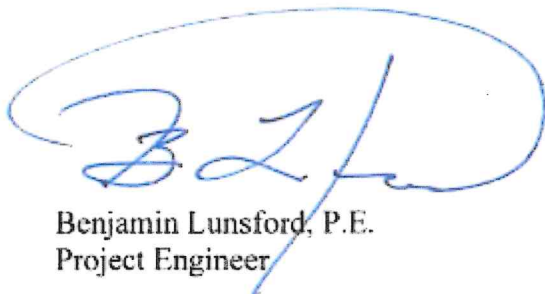
In general, fair to good soil conditions predominantly consisting of lean clay were observed at this site. We recommend that the new structures are supported on shallow foundations. After removing unwanted materials such as topsoil, organic materials, soft soils and debris, the entire building pads and paved areas should pass a proofroll as described in the subgrade preparation section of this report. Assuming proper site preparation, we recommend that foundations are designed for a maximum net allowable soil bearing pressure of **2,000** psf. These recommendations are more detailed in the appropriate sections of this report along with pavement design and site preparation recommendations.

Thank you for the opportunity to provide geotechnical engineering services on this project. Should you have questions regarding our findings or need additional consultations, please do not hesitate to contact our office.

Respectfully,

**Ladner Testing Laboratories, Inc.**

**Represented by:**



Benjamin Lunsford, P.E.  
Project Engineer



2/27/26

Heath S. Williams, P.E.  
Principal Engineer  
MS Registration No. 17702

**REPORT OF GEOTECHNICAL EXPLORATION**  
**MADISON CENTRAL HIGH SCHOOL STADIUM UPGRADES**  
**MADISON COUNTY, MS**  
**G-1808Q/0122-26-A**  
**FEBRUARY 27 2026**

**General**

This report presents the results of our geotechnical exploration findings and our geotechnical recommendations for new buildings, parking and drive areas in Madison County, Mississippi.

**Project Information**

The information presented in this section is based on information provided and our own site reconnaissance. The site is located at 1417 Highland Colony Parkway in Madison County, Mississippi. The approximate global positioning coordinates near the center of the site are 32.470357°, -90.155285°. We understand that the proposed project consists of the construction of new restrooms, a new press box, new gathering areas, new parking and new fencing at the subject site. Typical commercial construction techniques are anticipated.

If any of the information presented is incorrect or has changed, please advise Ladner Testing, Inc. to allow us to reevaluate our recommendations in the light of changes in the present project concept.

**Purposes of Exploration**

The purposes of this exploration were to explore the soil and groundwater conditions at the site and to identify any foreseeable special geotechnical considerations needed for the proposed construction. We accomplished the purposes of the study by:

1. Performing a general site reconnaissance and drilling borings to explore the subsurface soil and groundwater conditions,
2. Performing laboratory tests on selected representative soil samples from the borings to evaluate pertinent engineering properties, and
3. Evaluating the field and laboratory data to develop appropriate geotechnical engineering recommendations.

**Field Exploration**

To explore the subsurface conditions at this site, a total of eight (8) Standard Penetration Test (SPT) borings were drilled to depths of 8 to 10 feet below the existing ground surface within the footprint of the proposed new building and fencing areas. Five (5) auger borings were performed to a depth of 5 feet below the existing ground within new pavement areas. Boring locations were determined in the field by a Ladner Testing representative who measured distances and estimated right angles from existing site features, or by use of a handheld GPS. The boring locations should be considered approximate and boring elevations should be considered from the ground surface elevation at the time of our fieldwork on February 5, 2026. The soil test borings were performed with a truck mounted drill rig, which utilized continuous flight solid stem augers to advance the boreholes. Representative soil samples were taken for visual classification and further laboratory testing. The drill crew maintained a field log of the soils encountered in the borings.

### **Laboratory Testing Program**

Representative soil samples were selected and tested in our laboratory to check visual classifications and to determine pertinent engineering properties. The laboratory testing program included visual classifications of all soil samples, and natural moisture content, Atterberg Limit, and sieve analysis testing of selected samples. The laboratory test results are presented on the attached boring logs. An experienced geotechnical technician classified each soil sample on the basis of texture and plasticity in accordance with the Unified Soil Classification System. The group symbols for each soil type are indicated in the appropriate column of the attached boring logs. The stratification lines designating the interfaces between soil types on the boring logs and profiles are approximate; in-situ, the transitions may be gradual. The soil samples will be retained in our laboratory for a period of 60 days, after which, the samples will be discarded unless other instructions are provided.

### **Subsurface Soil Conditions**

The subsurface conditions discussed in the following paragraphs and those shown on the boring logs represent an estimate of the subsurface conditions based on interpretation of the boring data using normally accepted geotechnical engineering judgments. We note that the transition between different soil strata is usually less distinct than those shown on the boring logs. Subsurface conditions in unexplored locations may vary somewhat from those reported herein.

The borings performed for this exploration generally encountered Lean Clay within the top 10 feet below the existing ground surface. The clay material was typically medium stiff to stiff, with SPT-n values ranging from 6 to 16 blows per foot. Clayey sand and heavy clay were infrequently observed. For more specific information, refer to the boring logs in the appendix.

### **Groundwater Conditions**

Visual observation of the soil samples retrieved during the drilling exploration can often be used in evaluating the groundwater conditions. Groundwater was not observed during the geotechnical investigation at this site. Groundwater can sometimes be perched or localized based on soil and geological conditions at the site. Variations in the location of the long-term water table may occur as a result of changes in precipitation, evaporation, surface water runoff, and other factors not immediately apparent at the time of this exploration.

### **Geotechnical Recommendations**

The following geotechnical recommendations are based on our observations at the site, interpretation of the field data obtained during the exploration, laboratory test results, and our experience with similar subsurface conditions. In general, fair to good soil conditions were observed at this site. We recommend that the new buildings and fence posts are supported on shallow foundations.

Subgrade stability is dependent on moisture control around the footprint of the structures. Excavations and earthwork should be kept dry and covered during rain events. Moist soils must be excavated, moisture conditioned and recompacted. After construction, good site drainage is required to prevent runoff water from infiltrating the subgrade near the structures. We recommend that runoff is not allowed to infiltrate the subgrade within 10 feet of any structures. If runoff and irrigation cannot be diverted away from the structure using surface drainage, we recommend the use of subsurface drainage systems such as French drains.

### **Foundation Design Recommendations**

Assuming proper site preparation (removal of topsoil, soft/loose soils, organic materials and miscellaneous debris, proofrolling, footing bearing surface observation and good site drainage), this site appears suitable for the construction of the proposed structures on shallow foundations. We recommend that foundations are designed for a maximum net allowable soil bearing pressure of **2,000** psf. We also recommend that the slab and foundation system be made more rigid using perimeter and interior grade beams, and by incorporating both top and bottom reinforcing steel into both the footings and grade beams. Grade beams should have a maximum spacing of 20 feet.

Lateral support should be considered in the design of fence post foundations. Lateral bearing pressures can be estimated assuming 110 psf/ft of lateral resistance with depth below the finished ground for undisturbed or recompacted native soils. A cohesion of 130 psf can be applied to the contact area at the bottom of the foundation to determine sliding resistance.

Settlement values are based on the stated assumption that the site is properly prepared, and any deleterious material will be removed if found during construction. Conventional values for allowable settlement are typically 1 inch of total settlement with 1/2 inch of differential settlement. It is our opinion that for footings constructed in accordance with the requirements outlined in this report, the maximum total settlement is expected to be less than 1 inch, with the maximum differential settlement between adjacent columns expected to be approximately 1/2 inch or less.

To reduce the risk of foundation bearing failure and excessive settlement due to local shear or "punching" action, we recommend that continuous footings have a minimum width of **1.5** feet and that isolated spread footings have a minimum lateral dimension of **2** feet. In addition, footings should be placed at a depth to provide adequate bearing capacity and resist undermining of the footing by erosion. For this site, we recommend footing bottoms be placed at a minimum depth of **2** feet below lowest adjacent finished grade. Additionally, a lateral distance of at least **5** feet must separate all footing bottoms from the edges of adjacent finished ground slopes, or a distance of 1/3 the slope height, if greater.

The above bearing capacity and settlement values are based on our engineering experience with similar soil conditions and the anticipated structural loadings, and are to guide the structural engineer with the design. To minimize difficulties during the foundation installation phase, we recommend that Ladner Testing, Inc. be retained to observe the foundation bearing surfaces and to confirm that the recommended bearing pressures are developed.

**Pavement Design Recommendations**

Assuming that the subgrades are properly prepared by removing soft/loose soils, organic materials and debris as described in the subgrade preparation section, and thoroughly proofrolled, typical minimum pavement sections for the expected soil subgrades are shown below. If CBR values of less than 5 are encountered at the subgrade elevation at the time of construction, the subgrade may need to be stabilized by over excavation and replacement or by use of a geogrid reinforcement such as a Tensar or BaseLok product. Pavement design should be performed according to Mississippi (MDOT) Standard Specifications for State Aid Road and Bridge Construction, latest edition. Lime treatment should meet Class C specifications according to MDOT. For satisfactory performance of lime treated base, treated materials must consist of lean clay or clayey sand. Recommended sections should be reevaluated for axial loads greater than 18 kips.

**Typical Asphalt Pavement Sections**

<b>Material Type</b>	<b>Light Duty Driveways And Parking</b>	<b>Heavy Duty Truck Driveways</b>
AC Surface Course (ST 9.5mm)	2.0 inches	2.0 inches
AC Base Course (ST 12.5mm)	4.0 inches	6.0 inches
Crushed Aggregate Base (#610 Gradation) OR Lime Treated Base (6% by Weight)	6.0 inches  12.0 inches	8.0 inches  12.0 inches

The heavy-duty truck driveways section should be used in areas expecting heavy trucks, deliveries and/or garbage truck service. An alternative for rigid pavement sections is also provided below. At a minimum, we recommend that rigid concrete sections are used in areas with tight turns, frequent braking, entry and exit aprons, dumpster pads, and large or sustained loads. Concrete joint spacing, reinforcement and layout should be designed in accordance with American Concrete Institute’s “Guide for the Design and Construction of Concrete Parking Lots” - ACI 330.

**Typical Rigid Pavement Sections**

<b>Material Type</b>	<b>Light Duty Driveways And Parking</b>	<b>Heavy Duty Truck Driveways</b>
4,000 psi Concrete	6.0 inches	8.0 inches
Crushed Aggregate Base (#610 Gradation) OR Lime Treated Base (6% by Weight)	6.0 inches  12.0 inches	8.0 inches  12.0 inches

Pavement recommendations for sidewalk and gathering areas are also provided below.

### Typical Sidewalk Sections

Material Type	Thickness
4,000 psi Concrete	4.0 inches
Crushed Aggregate Base (#610 Gradation) OR Lime Treated Base (6% by Weight)	4.0 inches  8.0 inches

An important consideration with the design and construction of pavements is surface and subsurface drainage. Where standing water develops, either on the pavement surface or within the base course layer, softening of the subgrade and other problems related to the deterioration of the pavement can be expected. Furthermore, good drainage should minimize the risk of the subgrade materials becoming saturated over a long period of time. The use of a geosynthetic grid may be used to stabilize the subgrade if needed.

### Subgrade Preparation

The subgrade preparation should consist of removing all topsoil, organic materials, soft/loose soils, heavy clay, miscellaneous debris and organic materials. Observation is required to ensure that all the unwanted material is removed from the site. We also recommend that the subgrade preparation is extended to the expanded project limits, which are defined as a minimum of 5 feet beyond the footprint of the building structures and 3 feet beyond the edge of pavement. Site preparation limits should be extended laterally an additional 1 foot for each additional foot of vertical fill required at any location.

The prepared subgrade must be observed to be free of a substantial amount of organic material and of sufficient consistency to support the required structural loads. The resulting subgrade should be evaluated by a qualified geotechnical technician. *Proofrolling of the entire site using a loaded dump truck, having an axle weight of at least 10 tons, is required to aid in identifying any additional localized soft or unsuitable material that should be removed.* Any soft or unsuitable materials encountered during this proofrolling, revealed by pumping or rutting, should be removed and replaced with an approved backfill compacted to the criteria below.

### Fill Placement

The preparation of the proposed subgrade should be observed on a periodic basis by a representative of a qualified construction materials testing company to document that the subgrade is suitable for support of the proposed construction and/or fills. Structural fill materials should consist of approved material with less than 2 percent organic matter, free of debris, with rocks no greater than 6 inches and a Liquid Limit less than 40 percent and a Plasticity Index between 10 and 20 percent. Unacceptable fill materials include topsoil, ash, low-density soils with a maximum unit weight less than 95 pcf, organic materials, and highly plastic silts and clays. Any unsuitable materials removed during grading operations should be either stockpiled for later use in landscaped areas or placed in approved disposal areas either on site or off site.

Grade control should be maintained throughout the fill placement operations. All fill operations should be observed on a periodic basis by a qualified soil technician from Ladner Testing, Inc. to determine that minimum compaction requirements are being met. A minimum of one compaction test per 2,500 square foot area should be performed in every other lift placed. The elevation and location of the tests should be clearly identified and recorded at the time of fill placement. Fill materials should be placed in lifts not exceeding 8 inches in loose thickness and moisture conditioned to within  $\pm 2$  percent of the optimum moisture content to facilitate proper compaction. Structural fill and pavement base materials should be compacted to a minimum of **98 percent** of the maximum dry density obtained in accordance with ASTM Specification D-698, Standard Proctor Method.

### **Additional Considerations**

Exposure to the environment may weaken the soils at the subgrade level if excavations remain open for too long a time. If surface water intrusion or exposure softens the bearing soils, the softened soils must be removed from the excavation bottom immediately prior to placement of fill.

Positive site drainage should be maintained during earthwork operations, which should help maintain the integrity of the soil. Placement of fill on the near surface soils, which have become saturated, may be very difficult. When wet, these soils will degrade quickly with disturbance from contractor operations and will be extremely difficult to stabilize for fill placement.

The surface of the site should be kept properly graded in order to enhance drainage of the surface water away from the proposed building areas during the construction phase. We recommend that an attempt be made to enhance the natural drainage without interrupting its pattern.

The surficial soils contain fines, which are considered moderately erodible. All erosion and sedimentation shall be controlled in accordance with Best Management Practices and current City and DEQ requirements. At the appropriate time, we would be pleased to provide a proposal for construction materials testing and related services.

### **Closing**

This report has been prepared in accordance with generally accepted geotechnical engineering practice. No other warranty is expressed or implied. The evaluations and recommendations presented in this report are based on the available project information, as well as on the results of the exploration. Ladner Testing, Inc. should be given the opportunity to review the final drawings and site plans for this project to determine if changes to the recommendations outlined in this report are needed. Should the nature of the project change, these recommendations should be reevaluated. No third party is given permission to rely on this report or data without the express written consent of Ladner Testing, Inc. We recommend that the construction activities be observed by a qualified geotechnical engineer to provide the necessary overview and to check the suitability of the subgrade soils for supporting the footings. We would be pleased to provide an estimated cost for these services at the appropriate time.



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 P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> B-1 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<b>AUGER(ASTM D-1452)</b>	<b>TUBE(ASTM D-1587)</b>	<b>X</b>	<b>PENETRATION TEST(ASTM D-1586)</b>
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		ASPHALT 6" GRAY LEAN CLAY ( 0 - 2 1/2' )		22.5	31.0	10.0	94.1	CL	
	X		STIFF						13
		TAN & GRAY LEAN CLAY ( 2 1/2' - 3 1/2' )		21.3	34.0	14.0	92.8	CL	
	X		STIFF	16.8	30.0	13.0	73.0	CL	12
5		TAN & GRAY LEAN CLAY W/SAND ( 3 1/2' - 10' )							
	X		STIFF						15
10									
		- RESTROOM BUILDING -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 10 FT.



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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input checked="" type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		ASPHALT 6"							
		TAN & GRAY LEAN CLAY ( 0 - 1' )		23.2	36.0	17.0	94.9	CL	
	X	GRAY & TAN LEAN CLAY ( 1' - 5' )	STIFF	19.7	34.0	16.0	90.3	CL	16
	X		STIFF						10
5		TAN & GRAY LEAN CLAY W/SAND ( 5' - 8 1/2' )		19.4	38.0	21.0	71.0	CL	
	X	TAN, GRAY, & RED SANDY LEAN CLAY ( 8 1/2' - 10' )	STIFF	17.4	34.0	16.0	63.2	CL	13
10									
		- RESTROOM BUILDING -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 10 FT.



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<b>SAMPLES:</b>	<b>AUGER(ASTM D-1452)</b>	<b>TUBE(ASTM D-1587)</b>	<b>X</b>	<b>PENETRATION TEST(ASTM D-1586)</b>
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST			PASS #200 %	UNIFIED CLASS	STD. PEN
				%	LL%	PI %			
0		CONCRETE 4"							
		GRAY LEAN CLAY ( 0 - 1' )		27.9	38.0	16.0	92.7	CL	
		GRAY LEAN CLAY ( 1' - 5' )		29.0	36.0	14.0	96.9	CL	
5									
		- NORTH PLAZA -							
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.



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<b>SAMPLES:</b>	<b>AUGER(ASTM D-1452)</b>	<b>TUBE(ASTM D-1587)</b>	<b>X</b>	<b>PENETRATION TEST(ASTM D-1586)</b>
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST			PASS #200 %	UNIFIED CLASS	STD. PEN
				%	LL%	PI %			
0		TAN & GRAY LEAN CLAY ( 0 - 3' )		18.3	33.0	12.0	91.4	CL	
		TAN & GRAY LEAN CLAY ( 3' - 5' )		26.7	37.0	16.0	95.8	CL	
5		- ADA PARKING AREA -							
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.



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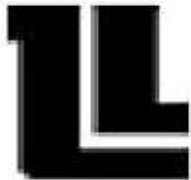
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<b>SAMPLES:</b>	<b>AUGER(ASTM D-1452)</b>	<b>TUBE(ASTM D-1587)</b>	<b>X</b>	<b>PENETRATION TEST(ASTM D-1586)</b>
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST			PASS #200 %	UNIFIED CLASS	STD. PEN
				%	LL%	PI %			
0		TAN & GRAY SILTY LEAN CLAY ( 0 - 1' )		23.8	28.0	9.0	92.4	CL	
		TAN, GRAY, & RED LEAN CLAY ( 1' - 5' )		20.1	36.0	17.0	92.6	CL	
5									
		- ADA PARKING AREA -							
10									
15									
20									
25									
30									

<b>WATER DEPTH</b> <u>0</u> <b>FT.</b>	<b>AFTER</b> <u>0</u> <b>HRS.</b>	<b>BORING ELEVATION</b> <u>0</u> <b>FT.</b>
<b>WATER DEPTH</b> <u>0</u> <b>FT.</b>	<b>AFTER</b> <u>0</u> <b>HRS.</b>	<b>BORING TERMINATED AT</b> <u>5</u> <b>FT.</b>



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<b>SAMPLES:</b>	<b>AUGER(ASTM D-1452)</b>	<b>TUBE(ASTM D-1587)</b>	<b>X</b>	<b>PENETRATION TEST(ASTM D-1586)</b>
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST			PASS #200 %	UNIFIED CLASS	STD. PEN
				%	LL%	PI %			
0		ASPHALT 3 1/2"							
---		TAN & GRAY LEAN CLAY ( 0 - 1')		20.8	37.0	15.0	93.1	CL	
---		TAN & GRAY LEAN CLAY ( 1' - 3')		19.4	28.0	10.0	93.9	CL	
---		TAN & GRAY LEAN CLAY ( 3' - 5')		16.8	27.0	10.0	87.6	CL	
5									
---		- SIDEWALK AREA -							
---									
10									
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15									
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20									
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25									
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WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.



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 P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> S-2 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<b>AUGER(ASTM D-1452)</b>	<b>TUBE(ASTM D-1587)</b>	<b>X</b>	<b>PENETRATION TEST(ASTM D-1586)</b>
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST			PASS #200 %	UNIFIED CLASS	STD. PEN
				%	LL%	PI %			
0		ASPHALT 3 1/2"							
		TAN LEAN CLAY ( 0 - 1' )		19.1	28.0	17.0	98.0	CL	
		TAN & GRAY LEAN CLAY ( 1' - 5' )		17.7	33.0	13.0	92.2	CL	
5									
		- SIDEWALK AREA -							
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.



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<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> F-1 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input checked="" type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		TAN SILTY LEAN CLAY ( 0 - 1' )		28.2	32.0	9.0	96.3	CL	
		TAN & GRAY LEAN CLAY ( 1' - 5' )	MEDIUM	23.9	36.0	12.0	95.1	CL	
	X		STIFF						8
5		TAN & GRAY LEAN CLAY ( 5' - 8' )		20.6	30.0	13.0	85.4	CL	
	X		MEDIUM						8
10		- FENCE AREA -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 8 FT.



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<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> F-2 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input checked="" type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0	X	TAN LEAN CLAY ( 0 - 2 1/2' )	MEDIUM	20.7	33.0	13.0	89.1	CL	6
	X	TAN & GRAY LEAN CLAY ( 2 1/2' - 3 1/2' )		22.6	31.0	11.0	92.6	CL	
	X	TAN & GRAY SILTY CLAYE SAND ( 3 1/2' - 6 1/2' )	STIFF	20.5	21.0	4.0	29.3	SC-SM	10
	X	TAN & GRAY SILTY SAND ( 6 1/2' - 8' )	MEDIUM	23.1	NA	NP	17.6	SM	7
10		- FENCE AREA -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 8 FT.



# ladner testing laboratories, inc

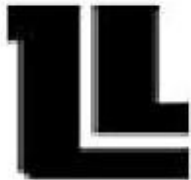
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 P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> F-3 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input checked="" type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0	X	TAN LEAN CLAY ( 0 - 2 1/2' )	STIFF	16.3	32.0	10.0	93.9	CL	10
	X	TAN & GRAY LEAN CLAY ( 2 1/2' - 5' )	MEDIUM	19.4	32.0	13.0	93.2	CL	8
5		TAN & GRAY HEAVY CLAY ( 5' - 6 1/2' )		23.5	52.0	33.0	90.9	CH	
	X	TAN & GRAY LEAN CLAY W/SAND ( 6 1/2' - 8' )	STIFF	19.3	46.0	29.0	83.4	CL	9
10		- FENCE AREA -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 8 FT.



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<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> F-4 <b>TECHNICIAN:</b> B.H. / M.S.
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**SAMPLES:**  AUGER(ASTM D-1452)  TUBE(ASTM D-1587)  PENETRATION TEST(ASTM D-1586)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		TAN & GRAY LEAN CLAY ( 0 - 1')		22.5	33.0	13.0	88.0	CL	
	X	TAN & GRAY LEAN CLAY ( 1' - 2 1/2')	MEDIUM	20.6	34.0	12.0	93.0	CL	6
		TAN & GRAY LEAN CLAY ( 2 1/2' - 5')		18.6	30.0	9.0	93.9	CL	
	X		MEDIUM						8
5		TAN & GRAY LEAN CLAY ( 5' - 8')		21.9	32.0	10.0	94.6	CL	
	X		STIFF						11
10		- FENCE AREA -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 8 FT.



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<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> F-5 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input checked="" type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		TAN & GRAY LEAN CLAY ( 0 - 1')		29.0	35.0	12.0	93.2	CL	
	X	TAN & GRAY LEAN CLAY ( 1' - 5')	MEDIUM	24.7	34.0	11.0	95.7	CL	6
	X		MEDIUM						7
5		TAN & GRAY LEAN CLAY ( 5' - 8')		22.5	30.0		94.0	CL	
	X		STIFF						10
10		- FENCE AREA -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 8 FT.



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<b>PROJECT:</b> MADISON CENTRAL HIGH SCHOOL STADIUM IMPROVEMENTS MADISON COUNTY, MS	<b>CLIENT:</b> WIER BOERNER ALLIN ARCHITECTURE 2727 OLD CANTON ROAD SUITE 200 JACKSON, MS 39216	<b>DATE:</b> 2/5/2026 <b>LAB NO:</b> 0122-26-A <b>BORE NO:</b> F-6 <b>TECHNICIAN:</b> B.H. / M.S.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input checked="" type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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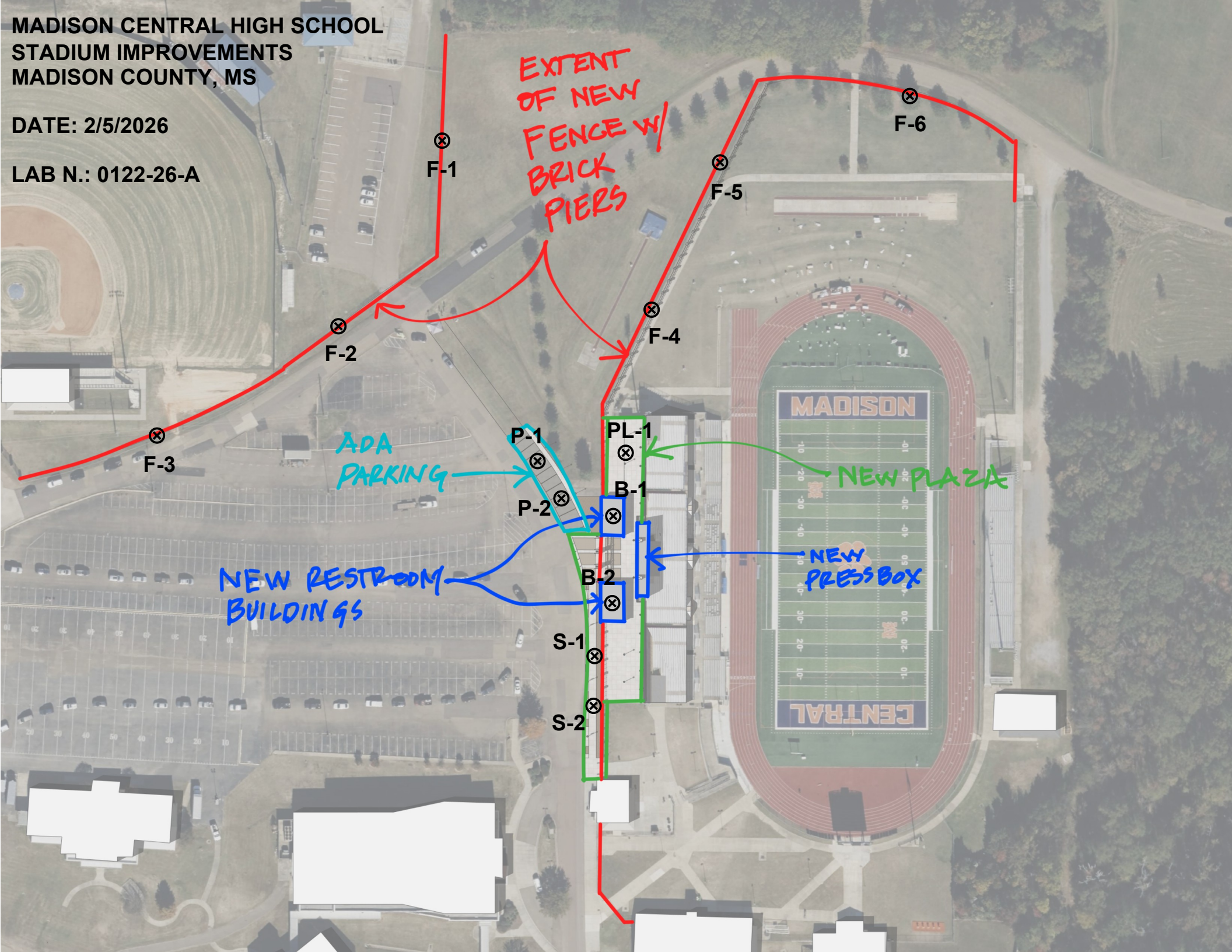
DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0	X	TAN & GRAY LEAN CLAY ( 0 - 2 1/2' )	MEDIUM	26.0	34.0	12.0	94.2	CL	8
	X	TAN & GRAY LEAN CLAY ( 2 1/2' - 5' )	MEDIUM	25.5	35.0	13.0	97.4	CL	6
5	X	TAN & GRAY LEAN CLAY ( 5' - 8' )	STIFF	19.3	35.0	14.0	97.1	CL	12
10		- FENCE AREA -							
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 8 FT.

**MADISON CENTRAL HIGH SCHOOL  
STADIUM IMPROVEMENTS  
MADISON COUNTY, MS**

**DATE: 2/5/2026**

**LAB N.: 0122-26-A**



**EXTENT  
OF NEW  
FENCE W/  
BRICK  
PIERS**

**ADA  
PARKING**

**NEW RESTROOM  
BUILDINGS**

**NEW PLAZA**

**NEW  
PRESSBOX**

F-1

F-2

F-3

F-5

F-4

F-6

P-1

P-2

PL-1

B-1

B-2

S-1

S-2