

July 6, 2020

ADDENDUM NUMBER ONE (1)

Project: New Justice Court Facility  
Marshall County Justice Complex  
Holly Springs, MS  
PN: 20005

FROM: Dean and Dean/Associates Architects, P.A.  
4400 Old Canton Road, Suite 200  
Jackson, MS 39211  
(601) 939-7717

The following additions, changes, clarifications and/or substitutions to the Project Drawings as indicated, are hereby made a part of the Contract Documents. Acknowledge receipt of this Addendum by inserting its number and date in the Proposal Form where indicated.

Clarifications

- Item #1:** Attached is the sign-in sheet for the Pre-Bid Conference held on Wednesday, July 1, 2020. This is for information purposes only.
- Item #2:** Contractor is advised that any soil material that needs to be hauled off the site shall be disposed of by the county. The general contractor shall coordinate this with the county and load material in the county dump trucks for the county to haul away.
- Item #3:** The exterior wall sheathing for this project shall be gypsum sheathing as specified in Section 054000, paragraph 2.04.

Exterior treated plywood is limited to parapet roof decking and entry canopy decking per details 2 and 3 on A303 and 2/A304.

Architectural Specifications:

- Item #1:** Section 011000, Summary Of Work, change paragraph 1.04 C.1 to read as follows:
  - 1. Permits – County and City – A permit is required by the general contractor but the permit fee has been waived.

Civil

SEE ATTACHED CIVIL ITEMS PROVIDED BY ELLIOTT & BRITT ENGINEERING

Electrical

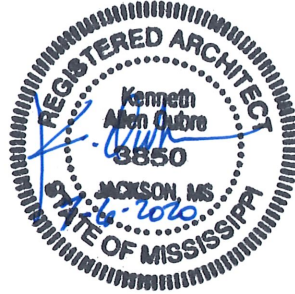
SEE ATTACHED ELECTRICAL ITEMS PROVIDED BY EDMONDS ENGINEERING

New Justice Court Facility  
Marshall County Justice Complex  
Holly Springs, MS  
PN: 20005  
Addendum #1

END OF ADDENDUM NUMBER ONE (1)

Dean and Dean/Associates  
architects p.a.

  
Kenneth A. Oubre, AIA, Principal



PLEASE ATTACH THIS ADDENDUM TO THE INSIDE FRONT COVER OF EACH SET OF SPECIFICATIONS.

# PRE-BID MEETING MEETING ATTENDEES

**PROJECT NAME** New Justice Court Facility      **OWNER / REFERENCE NUMBER** Marshall County Justice Complex      **DDA PN** 20005      **DATE** July 2, 2020

Name/Title	Entity	Telephone	E-Mail	Prime Bidder
KENNETH OUBRE	ARCHITECT	601-989-7117	koubre@deandean.com	No.
Cecil Mason	Flagstar Construction	601-278-2308	Cecil@Flagstarconstruction.com	Yes
Ron Tatham	Johnson Controls Fire Protection	901-487-0385	Ronald.Tatham@JCI.com	NO
MIKE MOORE P.M.	C+M BUILDERS	662-342-7182	m.moore@cmbuilders.com	YES
Barrett Gray/ales	American Fire Protection Group (731) 571-3384		barrett.gray@afpausa.us	NO
Russell Stewart	CI G	662-415-3711	Russell.stewart@cicontractors.com	Yes
Ty Williams	Hooker Law	662-489-0565	hew@hookerlaw.com	Yes
Tommy Williams	Hooker Law	662-489-0565		Yes
Harper Wagner / PM.	Wagner General Contractors Inc.	901-586-0060	allen@wagnergeneral.com	yes
Casper Briggs	SIEMENS	901-765-0013	casper.briggs@siemens.com	NO
JOSH PARKER	J.P. CORP GENERAL CONTRACTORS	662-571-4880	jpcorpms@gmail.com	Yes
Jonathan Logan	Castle Black Construction	901-489-0151	jo.logan@castleblackinc.com	Yes
<del>Scott Logan</del> Minton	Musky and Saffie	662-393-3330	Minton@muskyandsoffie.com	Yes
Andy Clemente	Elliott Britt Engineer	662-816-5887	andy@elliottbritt.com	NO



**Dean and Dean/Associates**  
Architects  
a professional association  
DEAN • GEDDIE • GRANT • OUBRE

DEAN AND DEAN/ASSOCIATES ARCHITECTS P.A. - P.O. BOX 4685 • JACKSON, MS • 39296-4685 • TELEPHONE: 601.939.7717  
4400 OLD CANTON ROAD • SUITE 200 • JACKSON, MS • 39211-5922 • FAX: 601.939.3420 • WEB: www.deandean.com

**JUSTICE COURT FACILITY ADDITION TO  
MARSHALL COUNTY JUSTICE COMPLEX  
HOLLY SPRINGS, MISSISSIPPI**

**ADDENDUM NO.1**

July 6, 2020

**1. CHANGES TO CONTRACT DOCUMENTS**

**SITE PREPARATION:**

**Clarification:**

The existing site has been rough graded, however there are isolated areas that the Contractor may need to finish grubbing the grass from and bring to finish grade. All areas under the proposed parking lots and buildings will be scarified to an 8 inch depth and re-compacted. There may be areas that require minimal undercut or up to 2 feet as outlined on page 3, "Site Preparation" of the GEO-technical Investigation, Prepared by Precision Engineering Corporation, Dated September 18, 2017.

For any areas that have to be undercut, that material can be disposed of on site. Any borrow material required can be found on site as well.

Any granular material needed for finish grading or to meet minimal depths under parking and buildings shall be provided by Contractor.

All other provisions of the Contract Documents and Construction Drawings shall remain unchanged and in full effect.

This addendum shall be attached to and become part of the Contract Documents referred to herein.

**2. CHANGES TO CONSTRUCTION DRAWINGS**

**SHEET C.2 "SITE LAYOUT":**

**Clarification:**

Concrete header curb was added at the edge of the proposed parking areas around the grassed lawn areas. Remove Sheet C.2 and Replace with the attached revised Sheet C.2.

**SHEET C.3 "GRADING PLAN / EROSION CONTROL":**

**Clarification:**

A note added for instructions to bidders on site preparation. Remove Sheet C.3 and Replace with the attached revised Sheet C.3.

**SHEET C.6 “CONSTRUCTION DETAILS”:**

**Clarification:**

A detail for concrete header curb was added to Sheet C.6 for clarification to contractor.  
Remove Sheet C.6 and Replace with the attached revised Sheet C.6.

This addendum shall be attached to and become part of the Contract Documents referred to herein.

DATE: July 6, 2020

  
\_\_\_\_\_  
Larry L. Britt, PE/PS  
Consulting Engineer

RECEIVED:

**This Addendum should be signed and dated by Bidder and submitted as proof of receipt with the submission of bids. The Bidder, by identifying the addendum number in the bid proposal and by the signing and submission of their bid, shall serve as proof of receipt of this addendum. Failure to comply may result in disqualification.**

DATE: \_\_\_\_\_

\_\_\_\_\_  
Signature

July 6, 2020

**ADDENDUM NO. 1**

PROJECT: Marshall County Justice Court Building  
Marshall County, Mississippi

FROM: Edmonds Engineering, Inc.  
1900 Lakeland Drive, Suite 1  
Jackson, Mississippi 39216

The following additions, changes, clarifications and/or substitutions to the Project Drawings as indicated are hereby made a part of the Contract Documents. Acknowledge receipt of this Addendum by inserting its number and date in the Proposal Form where indicated.

- Item No. 1: Specifications, Section 270910, Closed Circuit TV, Part 1 General, 1.00 Summary of Work, Paragraph A, add the following: Provide infrastructure only consisting of a double gang junction box with ½" EMT routed to communications room. Coordinate exact location with CCTV provider prior to rough-in.
- Item No. 2: Specifications, Section 270910, Closed Circuit TV, Part 1 General, 1.00 Summary of Work: Delete Paragraph B.
- Item No. 3: Specifications, Section 270921, Door Locking Control System: Delete entire section.
- Item No. 4: Specifications, Section 270960, Security System: Delete entire section.

**END OF ADDENDUM NO. 1**



# GEOTECHNICAL INVESTIGATION

MARSHALL COUNTY JUSTICE COMPLEX  
HOLLY SPRINGS, MISSISSIPPI

REPORT DATE:  
SEPTEMBER 18, 2016

PREPARED FOR:  
ELLIOTT AND BRITT ENGINEERING, PA  
PRECISION ENGINEERING JOB NO. 17210



Geotechnical Engineering  
Hydraulic Engineering  
Civil Engineering  
Surveying

276 County Road 101  
Oxford, MS 38655  
[oxford@pecorpms.com](mailto:oxford@pecorpms.com)



Land Planning/Subdivisions  
Road and Bridge Design  
Utility System Design  
Materials Testing

Phone 662-234-8539  
Fax 662-234-8639  
[www.pecorpms.com](http://www.pecorpms.com)

September 18, 2017

Elliott and Britt Engineering, PA  
Attn: Larry Britt  
823 North Lamar Boulevard  
Oxford, Mississippi 38655

RE: GEOTECHNICAL INVESTIGATION  
MARSHALL COUNTY JUSTICE COMPLEX  
HOLLY SPRINGS, MISSISSIPPI  
PROJECT NUMBER 17210

Dear Mr. Britt:

Precision Engineering Corporation (PEC) appreciates the opportunity to serve as your geotechnical consultant on the referenced project. Submitted here are the results of our geotechnical investigation. Should the scope of work be altered, we respectfully request an opportunity to assess this geotechnical investigation to verify the recommendations are applicable for the new scope. This investigation was performed following procedures developed by our firm and common to the local area. All field and laboratory procedures have been accomplished in general accordance with applicable ASTM standard specifications.

If you have any questions regarding your geotechnical investigation or need additional services please feel free to contact us.

Sincerely,  
**Precision Engineering Corporation**

A handwritten signature in blue ink, appearing to read "B. P. Byars", is written over the circular professional seal.



Brian P. Byars, P.E., P.S.  
Vice President Geotechnical Department

# Marshall County Justice Complex

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## **INTRODUCTION**

Precision Engineering Corporation was retained by Elliott and Britt Engineering, PA to perform a geotechnical investigation at the site of proposed construction. The intent of this investigation is to determine the general subsurface conditions and provide recommendations for construction and foundation design. The scope of this report was developed in conjunction with Dean and Dean/Associates Architects and includes the following:

- Site Reconnaissance: We observed site conditions and reviewed available topographic maps and aerial photographs.
- Soil Borings: Our drill crew advanced soil borings within the proposed construction and collected samples for laboratory analysis.
- Laboratory Analysis: Our trained technicians performed laboratory analysis on representative samples to determine the engineering properties of the soils.
- Evaluation of Data: Our geotechnical engineer evaluated the data compiled during field exploration and laboratory analysis.
- Preparation of Report: We provided recommendations for construction and foundation design in a clear and concise report.

Marshall County is currently planning the construction of a Justice Court facility addition to the Marshall County Detention Center located in Holly Springs, Mississippi. According to the Public Land Survey System, the site is located in the southwest quarter of Section 29, Township 3 South, Range 2 West in Marshall County, Mississippi. The site is adjacent to the existing Marshall County Justice Court. The site drains to the west to a ditch along West Street. Predominate groundcover on the site consists of grass in areas that have not been stripped. There are a few hardwood trees along the south edge of the existing parking lot. A large portion of the proposed building footprint is in an area being used as a borrow pit. Proposed improvements include construction of a single story Justice Court Addition.

## **FIELD INVESTIGATION**

Field investigation consisted of drilling 10 borings within the proposed project limits. Each boring was advanced by dry augering to the completion depth utilizing a CME 55 truck mounted drill. Borings located within the building footprint were advanced to depths ranging from 15 to 25 feet. Boring locations were selected by Dean and Dean/Associates Architects and were field located by Elliott and Britt Engineering, PA. Boring locations have been plotted on the Boring Location Plan in Appendix B. After completion, the boreholes were then backfilled using soil cutting.

Representative samples were collected from auger cuttings or a 2-inch outside diameter split barrel sampler. Samples were collected continuously for the first 6 feet and at a maximum of 5-foot intervals to boring termination. Additionally, Standard Penetration Resistance values (See ASTM D 1586) were determined and recorded on the boring logs for the various soils encountered. The Standard Penetration Resistance, or “N” value, is the number of blows required to drive an 18-inch standard split barrel sampler the final 12 inches utilizing a 140-pound hammer and a free fall height of 30 inches. An automatic hammer was utilized during Standard Penetration Testing. As the samples were collected, they were field classified (see ASTM D 2488) and immediately placed in airtight containers for future testing and classification.

## **LABORATORY ANALYSIS**

Laboratory work included grain-size analysis, moisture content determination, Atterberg Limit determination and Unified Soil Classification of select samples. Results of laboratory testing are shown on the boring logs and laboratory data sheets located in the Appendix. A geotechnical engineer prepared the final boring logs from field logs, collected samples and laboratory test results.

To aid in the general interpretation of the soil conditions at the project site, in-situ moisture contents (See ASTM D 2216 and 4643) were determined within the various soil strata. This determination was made possible by placing extracted samples in sealed containers immediately upon removal from the subsurface.

Atterberg limits were conducted in an effort to estimate the susceptibility of the cohesive soils encountered to shrink and swell with changes in moisture content. Liquid and plastic limit tests (See ASTM D 4318) were conducted on selected representative samples taken from the various soil strata encountered. The liquid limit (LL) is the moisture content above which a soil behaves as a viscous fluid, whereas the plastic limit (PL) is the moisture content below which the soil behaves as a solid. The plasticity index (PI) is the numerical difference between the liquid and plastic limit and is indicative of the relative activity of a cohesive soil.

Soils exhibiting a low plasticity index are relatively inactive and are ordinarily suitable as a foundation material. Conversely, soils having a high plasticity index are susceptible to varying degrees of volume change (i.e. shrinkage and swelling) with fluctuations in moisture content.

Grain size analyses (See ASTM D 422) were conducted on representative samples of the various soils encountered to determine the particle size distribution of materials comprising the strata. Results of these tests can be utilized in classifying the soils in accordance with the Unified Soil Classification System (See ASTM D 2487).

For your convenience, a listing of the symbols recognized by the Unified Soil Classification System and their meaning is provided with the boring logs (See Appendix C).

## **SUBSURFACE CONDITIONS**

### **Site Geology**

Based on geologic mapping and soils encountered the site is situated in the Tallahatta formation. This formation is of the Eocene Age. The Tallahatta formation is made up of sand, locally glauconitic containing claystone and clay lenses and abundant clay stringers.

### **Soil Conditions**

Details of subsurface conditions encountered by the soil borings are shown on the boring logs located in Appendix C. The boring logs represent our interpretation of the subsurface conditions based upon examination of the collected samples and laboratory testing of selected samples. Stratification lines on the boring logs represent approximate boundaries between soil types; however, the actual transition between soil types may be gradual. The general soil conditions and their pertinent characteristics are discussed below.

The borings generally encountered approximately 5 feet of lean clays and silts underlain by clayey sand. The borings did not encounter groundwater. Approximately 4 inches of topsoil was encountered in the borings that had not been previously stripped.

## **SITE PREPARATION**

Based on preliminary site plans the site will have a finished floor elevation of at or near existing ground surface. The site will require minimal grading to reach proposed finished subgrade. Our investigation was conducted after a relatively significant rain event. Our rig required assistance to traverse the site due to wet, soft soils at the surface. The underlying soils were found to be competent and suitable to support the anticipated loads. Once the areas with grass have been stripped, we recommend the exposed soils be scarified 8 inches and recompacted as outlined below as structural fill. There may be a few isolated spots that have eroded material from the stripped area that may require minimal undercut to stabilize the surficial soils. We anticipate undercut to be limited to two feet in these areas.

On-site soils are suitable for use as structural fill. If imported structural fill is required, the material should be a silty sand, clayey sand or lean clay (Unified Classification SM, SC or CL) type soil. The plasticity index of fill should be a maximum of 20 and have a maximum liquid limit of 40. If material of this type is not locally available, an alternate material may be selected upon approval of the geotechnical engineer. Materials should be compacted to a minimum of 95% of maximum dry density per Standard Proctor (ASTM D 698) under areas which will support structures or pavements. Compaction should be achieved prior to placing subsequent lifts. Fill soils should be placed in maximum loose lifts of 8 inches at a moisture content comparable ( $\pm 3.0\%$ ) to the optimum moisture content established in the laboratory. Structural fill should extend a minimum of 5 feet outside the building footprint. Compaction may be accomplished by steel-wheel rollers or other approved equipment well suited to the soil being compacted.

Prior to placing fill or construction of any structures any topsoil, vegetation, abandoned utilities and old foundations should be removed from areas receiving fill, foundations, slabs or pavements. During construction and grading any unstable areas identified by excessive movement under equipment or “pumping” of the soils should be undercut and the area backfilled with structural fill as outlined in this section. Areas receiving fill should be in a moist ( $\pm 2.0\%$  above optimum moisture) condition. Fill adjacent to existing slopes shall be stepped or benched into the existing slopes a minimum two horizontal feet for each lift of fill material. No material should be placed on surfaces that are muddy, frozen or that contain frost.

In order to verify that compaction and moisture requirements are satisfied, it is imperative that competent laboratory personnel conduct sufficient field density tests. We recommend 1 density test per 2,500 square feet per lift within the building footprint and 1 density test per 5,000 square feet per lift under pavements.

Final grading should be such that surface water and stormwater—including discharge from roof drains, landscape beds, and irrigation systems—is expediently removed from the proximity of the structures.

Groundwater was not encountered during our drilling activities in the building area. Be advised that groundwater levels may fluctuate seasonally, with changes in rainfall. Groundwater levels can also be affected by changes in site development. It is not anticipated to encounter groundwater during foundation excavations.

## **FOUNDATION RECOMMENDATIONS**

Based upon our analysis and adherence to our recommendations outlined in SITE PREPARATION, the proposed structure can be supported by a conventional shallow foundation system consisting of continuous footings, turned down footings or grade beams under load bearing walls and isolated reinforced concrete spread footings beneath columns. Any required earthwork should be accomplished in strict accordance with the criteria stipulated under SITE PREPARATION.

Continuous footings should bear a minimum of 18 inches below the finish subgrade elevation and have a minimum width of 18 inches. Continuous footings may be designed for an allowable bearing capacity of 3,500 psf. Reinforced concrete spread footings may be designed for an allowable bearing capacity of 3,500 psf bearing a minimum of 18 inches below the finish subgrade. Spread footings should have a minimum width of 24 inches. Interior grade beams should bear a minimum of 12 inches below finished grade and have a minimum width of 12 inches. Interior grade beams may be designed for an allowable bearing capacity of 1,500 psf. Size and location of reinforcing steel should be determined by the structural consultant.

Floor slabs can be designed utilizing a modulus of subgrade reaction ( $k_s$ ) of 100 pci (pounds per square inch of contact area per inch of deflection). A granular material (relatively clean sand or gravel) may be utilized for leveling and as a capillary break beneath the floor slabs. Additionally, a minimum 6 mil vapor barrier should be used beneath the slab.

Footing excavations should not be left open for extended periods of time. If a delay in placement of structural concrete is expected, a 2 to 3 inch "mud mat" should be placed in the excavation. Any loose soils should be removed from the excavation prior to placement of a "mud mat" or structural concrete.

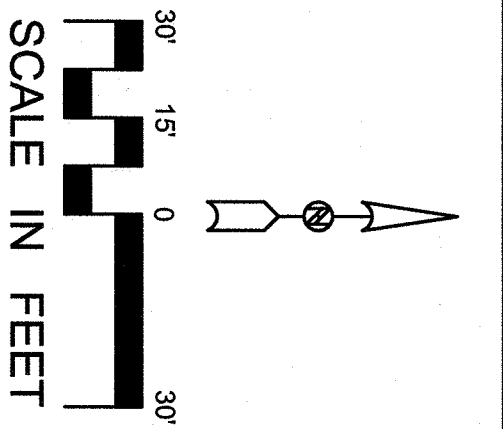
The bearing capacities recommended are applicable for structural fill provided the fill material meets the requirements described under SITE PREPARATION.

## **LIMITATIONS AND RESTRICTIONS**

The analysis and recommendations provided in this report are based on data obtained from the boreholes shown on the boring location plan. It is assumed that these borings are representative of conditions throughout the site. If site conditions are determined to be different than those described in this report or the scope of the project changes this office should be promptly notified. Precision Engineering Corporation will, if necessary, re-evaluate the recommendation in this report.

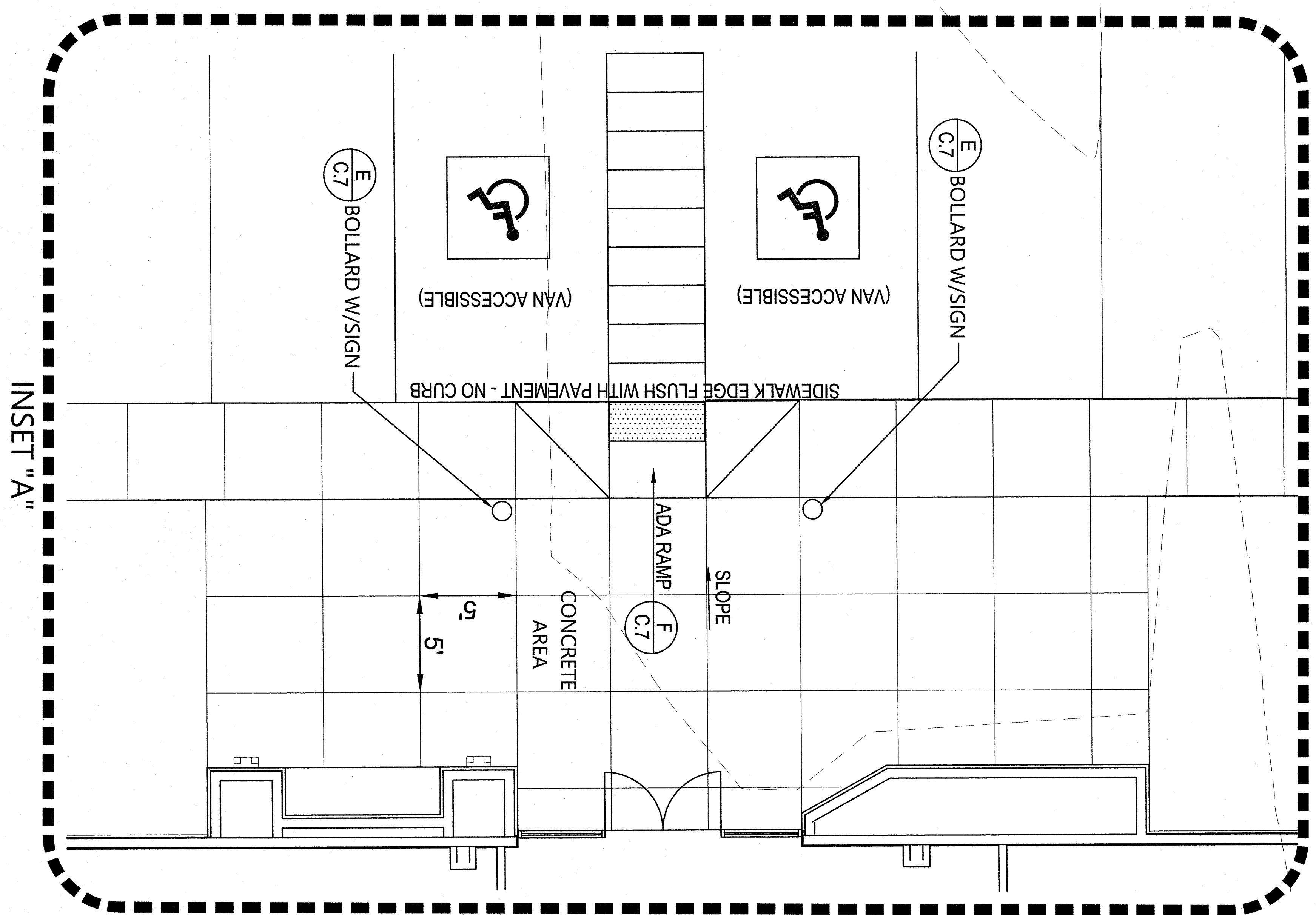
This report is intended for the specific project and location described in the INTRODUCTION. The recommendations in this report are not necessarily valid for other projects or other project locations. This report is intended for use by professional personnel competent to evaluate the significance and limitations of its contents and responsible for the application of the information and recommendations presented. Precision Engineering Corporation has used a standard of care and skill commonly accepted by geotechnical engineers in the local area. No other warranty is expressed or implied.

The geotechnical engineer should be retained to review final plans and specifications to verify that recommendations in this report are properly interpreted and incorporated in the design. Should testing and observation services during construction be provided by others, Precision Engineering Corporation will cease to be the Geotechnical Engineer of Record. Precision Engineering Corporation is not responsible for misuse or misinterpretation of our report by others.

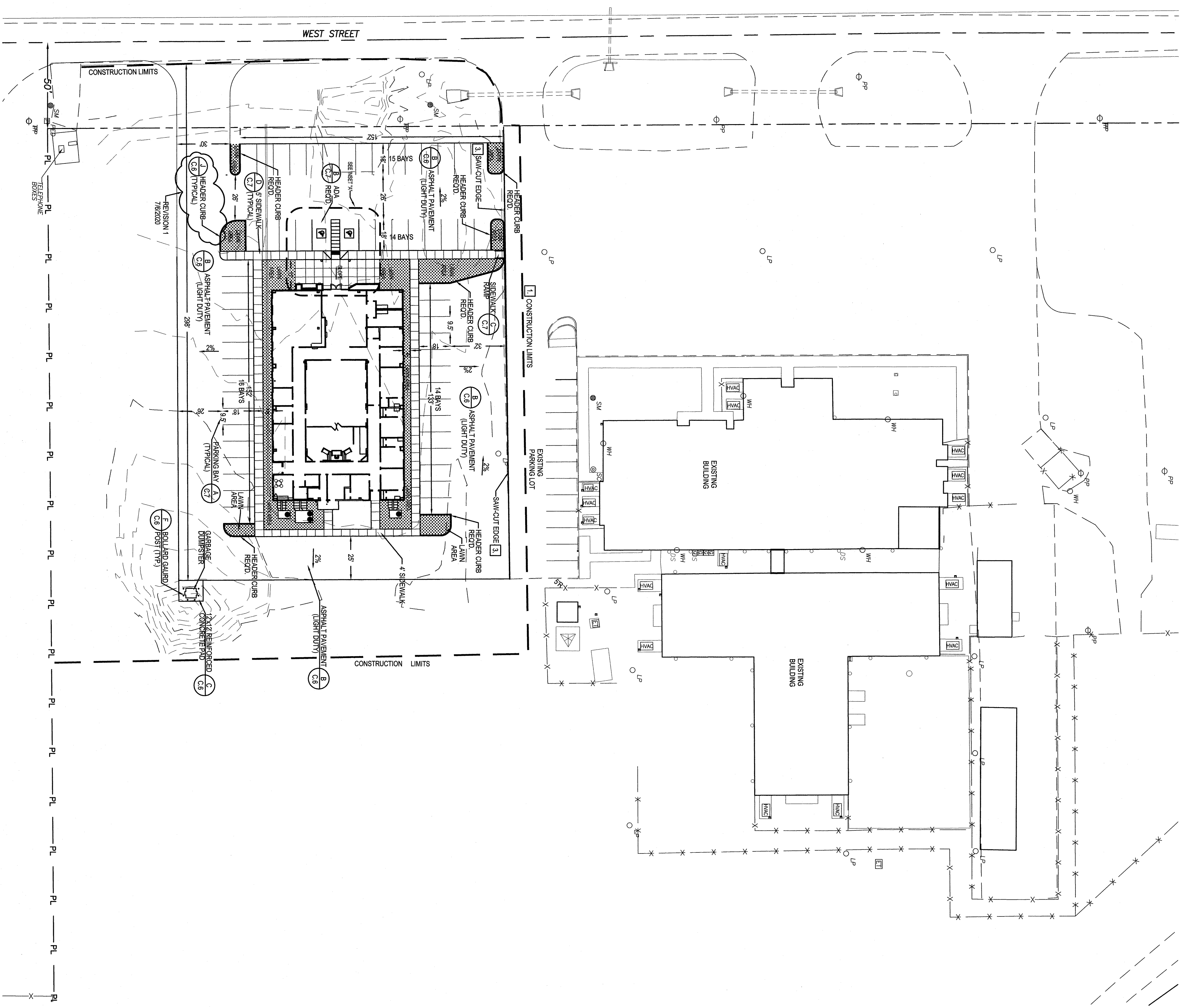


**NOTES**

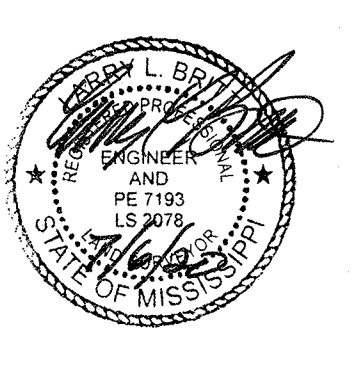
1. CONSTRUCTION LIMITS WHICH ARE THE LIMITS OF CLEARING AND GRUBBING ARE AS SHOWN. ALL CONSTRUCTION SHALL BE WITHIN THESE LIMITS AND SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. CONTRACTOR SHALL PROVIDE, INSTALL, MAINTAIN, AND RELOCATE AS DIRECTED ORANGE SAFETY BARRIERS TO PROTECT COMPLETE OR PART SUCH TIME AS THE ARCHITECT DIRECTS IT TO BE RELOCATED. THIS SHALL BE AN ASSUMED BY ITEM.
2. UTILITIES ARE SHOWN AS THEY WERE MARKED BY UTILITY OWNERS, OR AS TRANSMITTED FROM UTILITY OWNERS MAPS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT MISSISSIPPI AND HAVE ALL UTILITIES LOCATED PRIOR TO BEGINNING CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO UTILITIES WHICH ARE NOT ON THE MISSISSIPPI SYSTEM. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO LOCATE AND LOCATE HOSE UTILITIES.
3. SINK-CUT EDGES SHALL BE MADE IN THE EXISTING ASPHALT TYPICAL LAPS OF PROPOSED ASPHALT DRIVEWAYS IN ORDER TO MAINTAIN A SMOOTH TRANSITION FROM EXISTING ASPHALT TO NEW ASPHALT AND CONCRETE PAVEMENT.



REVISION 1:  
 ARMED CONCRETE HEADER CURB AT EDGE OF PAVEMENT AND/OR GRASSED DRIVEWAYS.

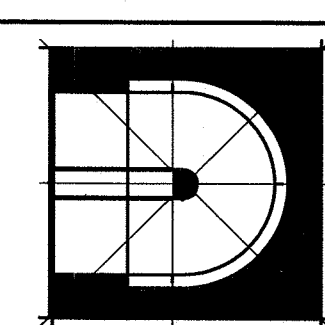


**B ELLIOTT & BRITT**  
**ENGINEERING, P.A.**  
 OXFORD • BATESVILLE • NEW ALBANY • HOLLY SPRINGS  
 (662) 234-1763  
 WWW.ELLIOTTBRITT.COM

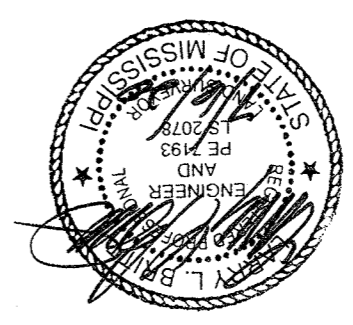
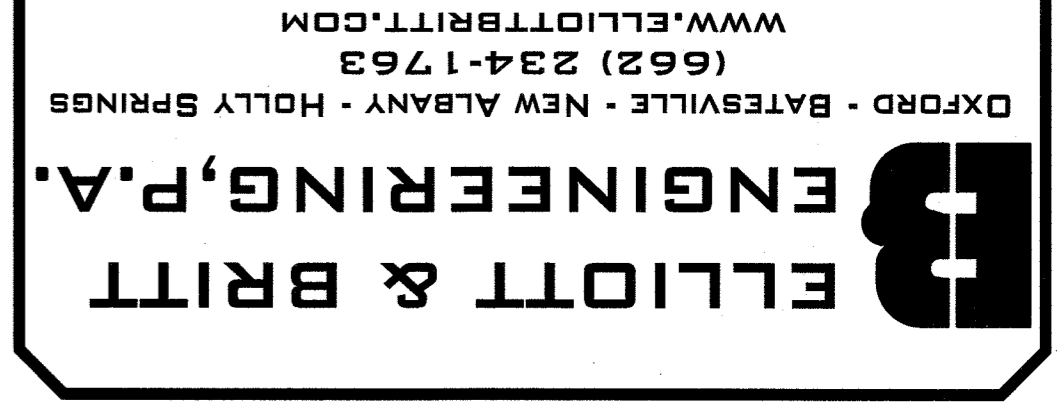


Sheet Number : **C.2**  
 A NEW JUSTICE COURT FACILITY  
 FOR  
 MARSHALL COUNTY JUSTICE COMPLEX  
 HOLLY SPRINGS, MISSISSIPPI

Project No: 20005 Revisions: JULY 6, 2020  
 Date: JUNE 11, 2020  
 Drawn: ALC  
 Checked: LLB



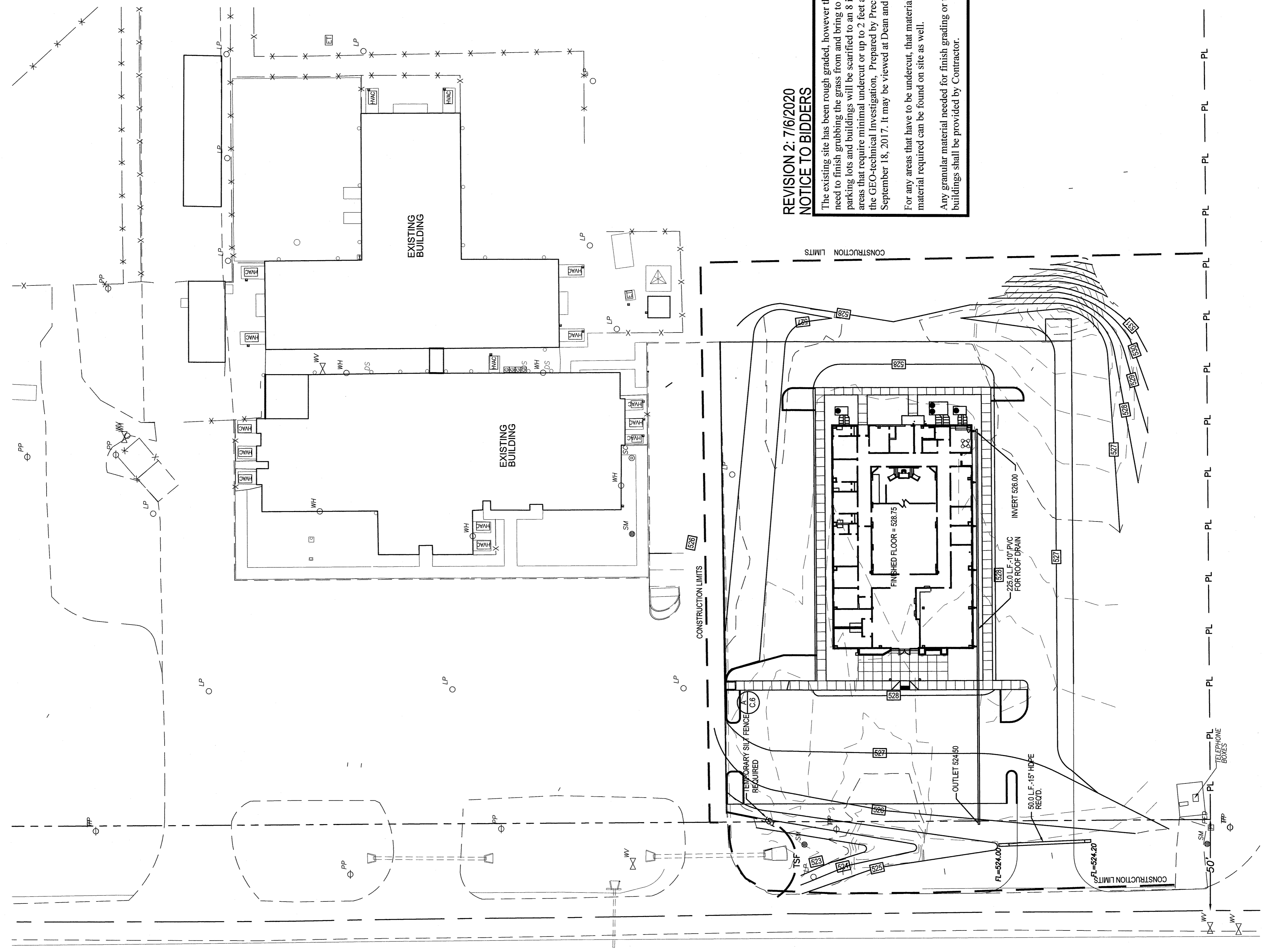
**Dean and Dean / Associates**  
 architects  
 a professional association  
 DEAN • GEDDIE • GRANT • OUBRE



Project No: 20005 Revisions: JULY 6, 2020  
Date: JUNE 11, 2020  
Drawn: ALC  
Checked: LTB

Dean and Dean / Associates  
a professional association  
DEAN • GEDDIE • GRANT • OUBRE

P.O. Box 4685 Jackson, MS 39296 4400 Old Canton Rd. Suite 200 Highland Bluff Jackson, MS 39211 (601) 939-7717 www.deanden.com

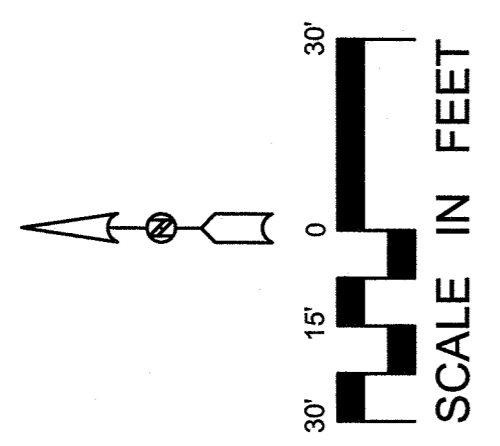


**REVISION 2: 7/6/2020  
NOTICE TO BIDDERS**

The existing site has been rough graded, however there are isolated areas that the Contractor may need to finish grading the grass from and bring to finish grade. All areas under the proposed parking lots and buildings will be scarified to an 8 inch depth and re-compacted. There may be areas that require minimal undercut or up to 2 feet as outlined on page 1, "Site Preparation" of the GEO-technical Investigation, Prepared by Precision Engineering Corporation, Dated September 18, 2017. It may be viewed at Dean and Dean/Associates.

For any areas that have to be undercut, that material can be disposed of on site. Any borrow material required can be found on site as well.

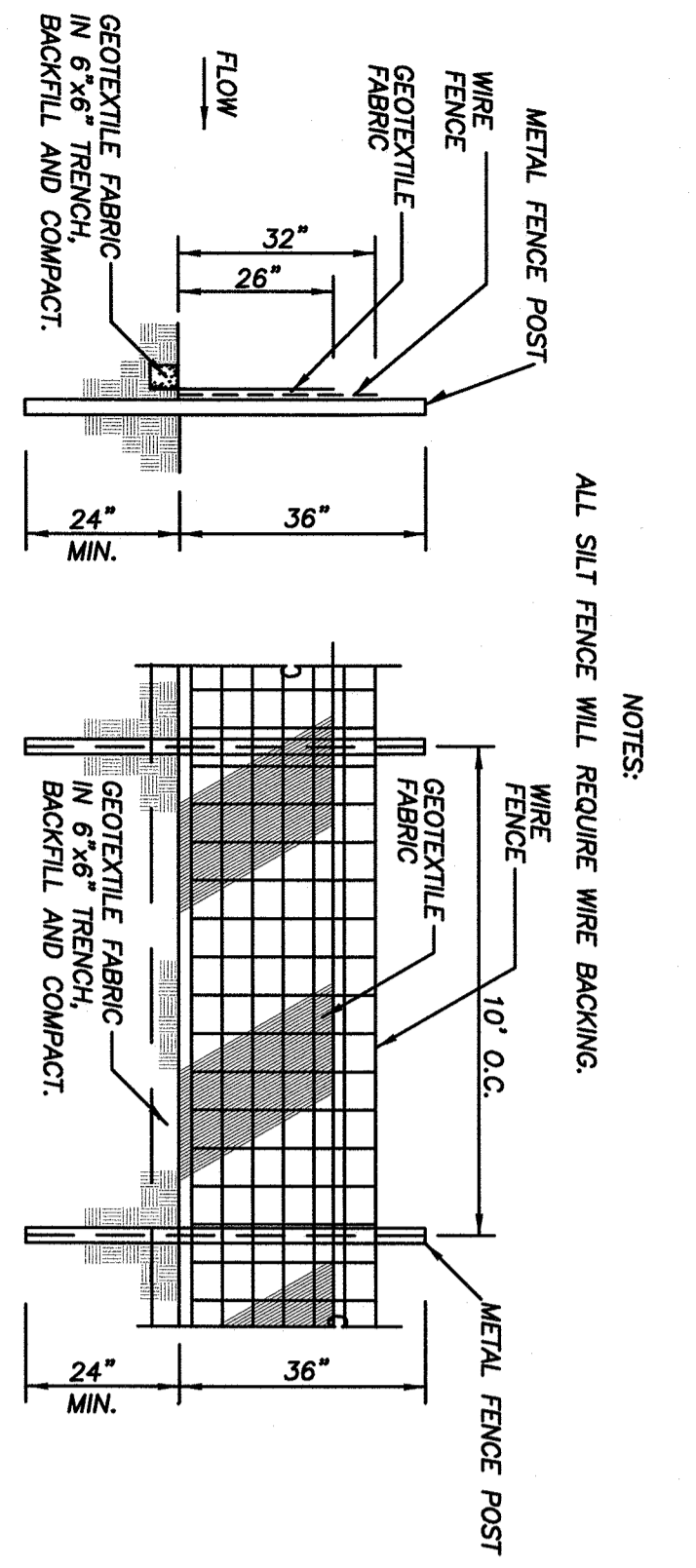
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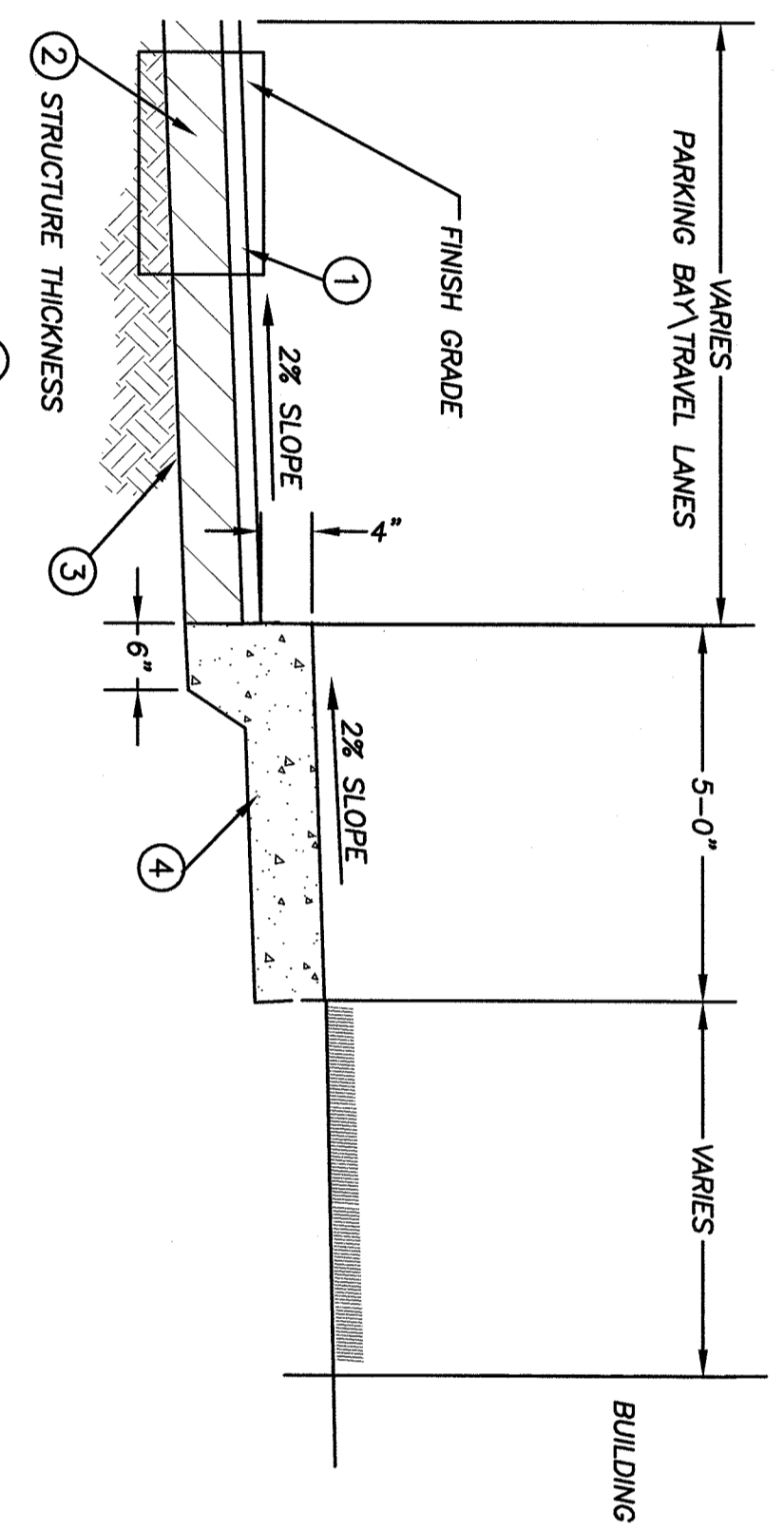
**NOTES**

- 1 TEMPORARY SILT FENCE SHALL BE PLACED AS SHOWN OR AS DIRECTED BY THE ENGINEER AT NO ADDITIONAL COST TO OWNER. LINES WITH "TSF" DENOTE TEMPORARY SILT FENCE.
- 2 ALL EROSION AND/OR SEDIMENT CONTROLS SHALL BE INSTALLED PROPERLY AND IN PLACE PRIOR TO BEGINNING ANY CONSTRUCTION ACTIVITIES.
- 3 SITE INFORMATION  
THE CONSTRUCTION OF THE PROPOSED PROJECT WILL DISTURB APPROXIMATELY 4.0 ACRES, AND THEREFORE WILL FALL UNDER MD.EC.3 SMALL CONSTRUCTION GENERAL PERMIT.
- 4 HOUSEKEEPING PRACTICES: THE CONTRACTOR AND ALL SUB-CONTRACTORS PERFORMING CONSTRUCTION ACTIVITIES ON THIS SITE SHALL SUBMIT TO THE SUPERVISOR OF THE PROJECT A HOUSEKEEPING PLAN. THE HOUSEKEEPING INTERIM LOCATION SHALL BE USED AS GENERAL PRACTICES FOR APPROXIMATE A "NOTICE TO PROCEED" WILL BE ISSUED. THE CONTRACTOR AND ALL SUB-CONTRACTORS ALSO SHALL PERFORM THE FOLLOWING:  
  - DESIGNATE AREAS ON SITE FOR EQUIPMENT MAINTENANCE AND REPAIR
  - PROVIDE WASTE RECEPTACLES AT CONVENIENT LOCATIONS AND PROVIDE REGULAR COLLECTION OF WASTE.
  - PROVIDE PROTECTED STORAGE AREAS, AS NEEDED, FOR CHEMICALS, FERTILIZERS, AND OTHER POTENTIALLY TOXIC MATERIALS
  - PROVIDE ADEQUATELY MAINTAINED SANITARY FACILITIES.
  - PROVIDE WATER TRUCK TO CONTROL DUST
  - PROVIDE PROPER CONCRETE TRUCK WASH-OUT AREA.
- 5 MANAGEMENT OF EROSION CONTROL MEASURES: THE CONTRACTOR SHALL PREPARE AND MAINTAIN EROSION CONTROL MEASURES IN CONSTRUCTION BY THE PLACEMENT OF SILT FENCE AND STRAW MATS AT THE LOCATIONS SHOWN TO THE RIGHT, AND AT LOCATIONS AS DESIGNATED BY THE ENGINEER. THIS SHALL BE DONE IN ORDER TO PREVENT DOWNSTREAM SITUATION OF ANY DITCHES, PIPES, DRAINAGE CANALS, OR OTHER STRUCTURES. THE CONTRACTOR SHALL PROVIDE ANY ADDITIONAL EROSION CONTROL MEASURES, AS NEEDED, AS DIRECTED BY THE ENGINEER.
- 6 IMPLEMENTATION SEQUENCE:  
  - BUILD CONSTRUCTION ENTRANCE/EXIT
  - INSTALL SILT FENCE AT CONSTRUCTION LIMITS OR AS DIRECTED BY ENGINEER
  - CLEAN SITE
  - SEED/SOO DISTURBED AREAS AND MAINTAIN UNTIL GRASS GROWTH
- 7 VEGETATIVE EXISTING TREES, SHRUBS, OR LANDSCAPING SHALL BE PRESERVED AND MAINTAINED THROUGHOUT CONSTRUCTION. ANY TREES OR SHRUBS THAT ARE SEVERED OR DAMAGED SHALL BE REPLACED WITH EXCESSIVE BLANKET AND/OR SOIL AS SOON AS POSSIBLE AFTER DISTURBANCE TO EFFECTIVELY CONTROL SOIL EROSION. ALL SEEDING OR SOOED AREAS SHALL BE MAINTAINED BY THE CONTRACTOR UNTIL FINAL ACCEPTANCE OF THE PROJECT. ANY "WASH" AREAS THAT APPEAR AFTER SEEDING SHALL BE FILLED WITH TOPSOIL AND RESEEDED AS NECESSARY. SEEDING SHALL BE PERMANENTLY STABILIZED, AND NOT SCHEDULED FOR ANY ACTIVITY TO TAKE PLACE FOR 14 DAYS OR MORE, ARE TO BE RESEED IMMEDIATELY.
- 8 STRUCTURAL CONTROLS: TEMPORARY SILT FENCE COMPLETE WITH WIRE BACKING, SHALL BE USED ALONG THE BOTTOM AND TOPS OF SLOPES TO PREVENT SEDIMENT FROM CONSTRUCTION ACTIVITIES TO LEAVE THE PROJECT SITE.
- 9 CONTRACTORS MAINTENANCE REQUIREMENTS:  
  - THE CONTRACTOR SHALL REMOVE SEDIMENT FROM SILT FENCE WHEN THE ACCUMULATED SEDIMENT HAS REACHED 1/3 OF THE SILT FENCE HEIGHT.
  - THE CONTRACTOR SHALL REMOVE AND REPLACE ALL NON-FUNCTIONAL SILT FENCE.

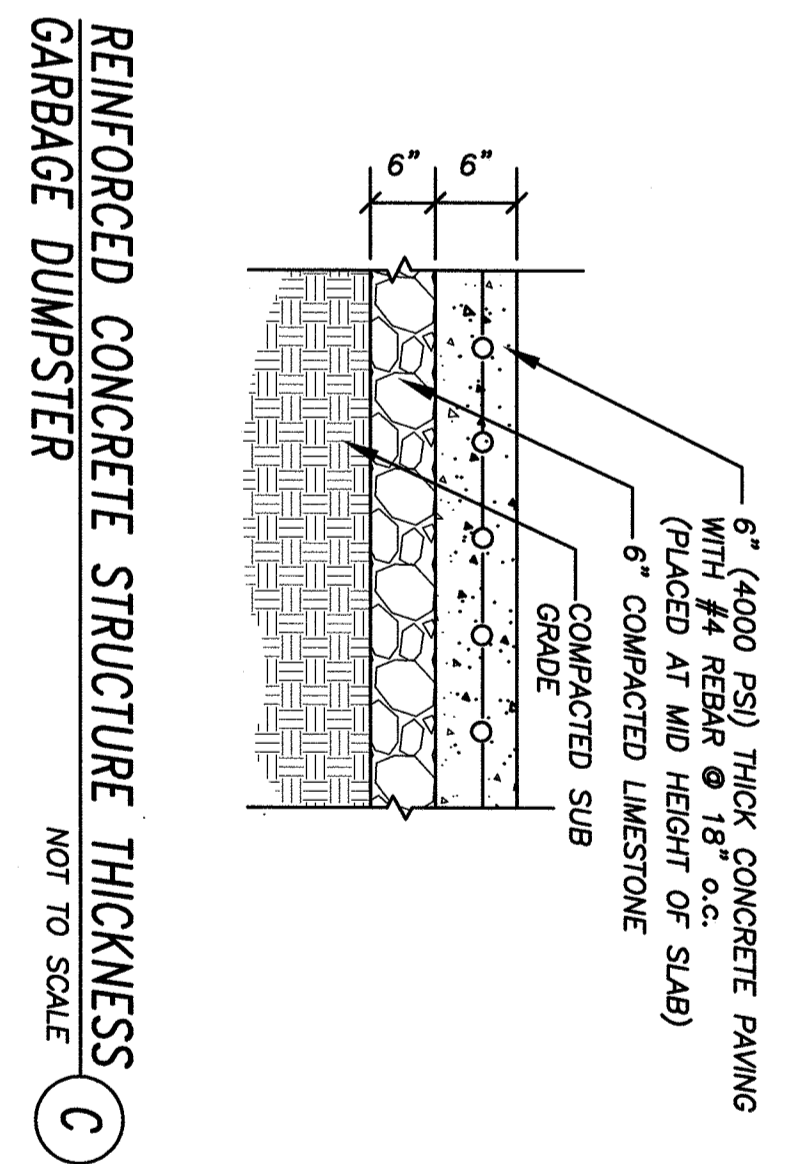
THE CONTRACTOR SHALL NOT DISTURB MORE THAN TWENTY (20) ACRES AT ANY ONE TIME. THE CONTRACTOR SHALL PERMANENTLY STABILIZED BEFORE DISTURBING ANY FURTHER ACREAGE.
- 10 RATES OF APPLICATION:  
  - COMMERCIAL FERTILIZER (13-13-13) 2 TONS/ACRE
  - VEGETATIVE MATERIALS FOR MULCH 3 TONS/ACRE



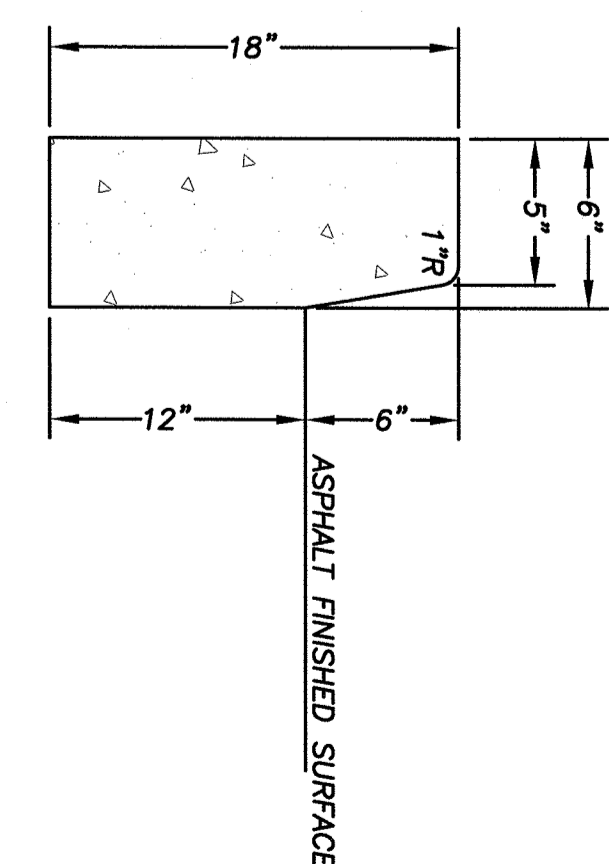
SIDE ELEVATION  
FRONT ELEVATION  
TEMPORARY SILT FENCE DETAIL  
NO SCALE  
A



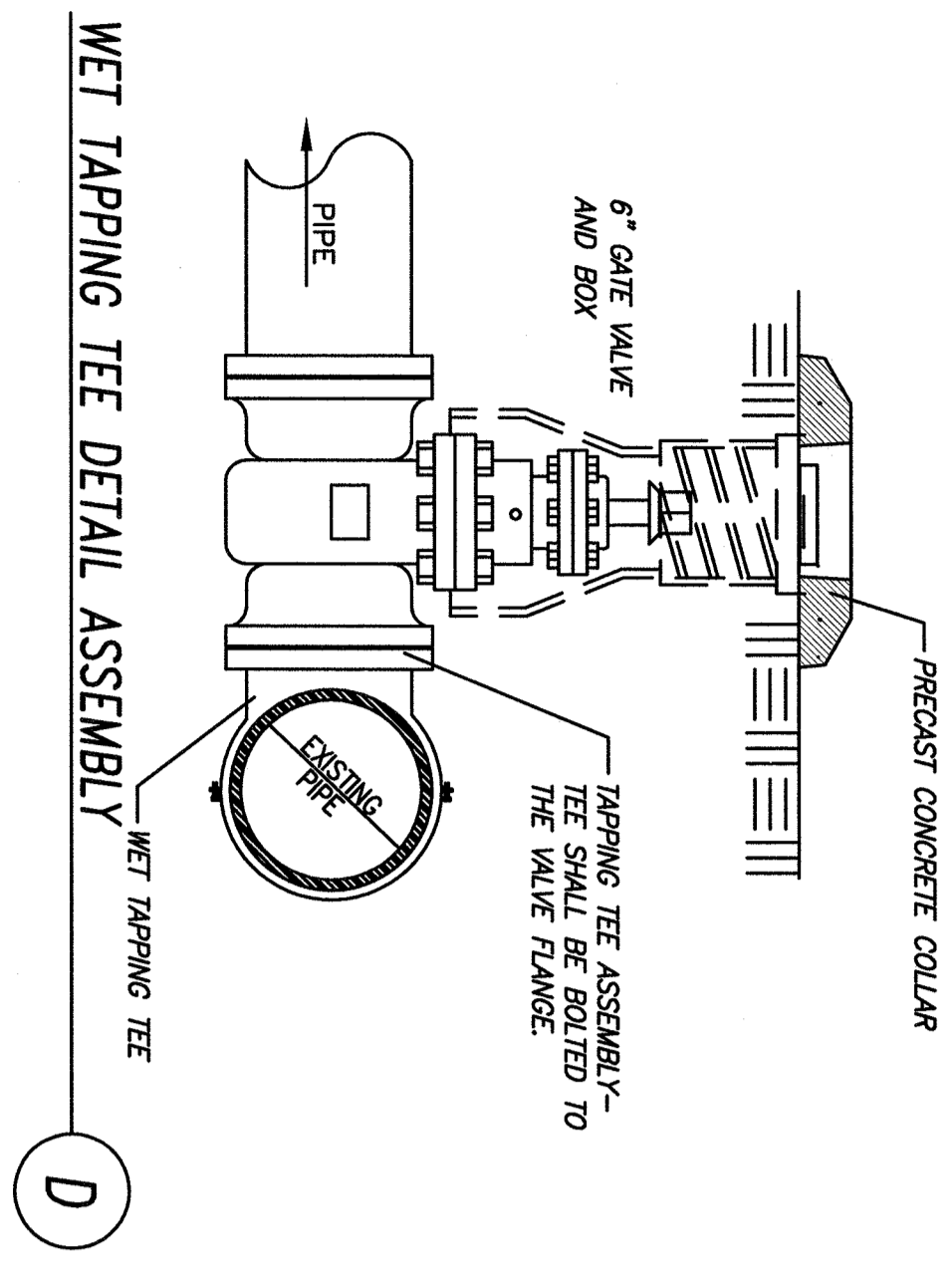
TYPICAL BASE AND PAVE SECTION FOR:  
PARKING BAYS AND TRAVEL LANES  
NO SCALE  
B



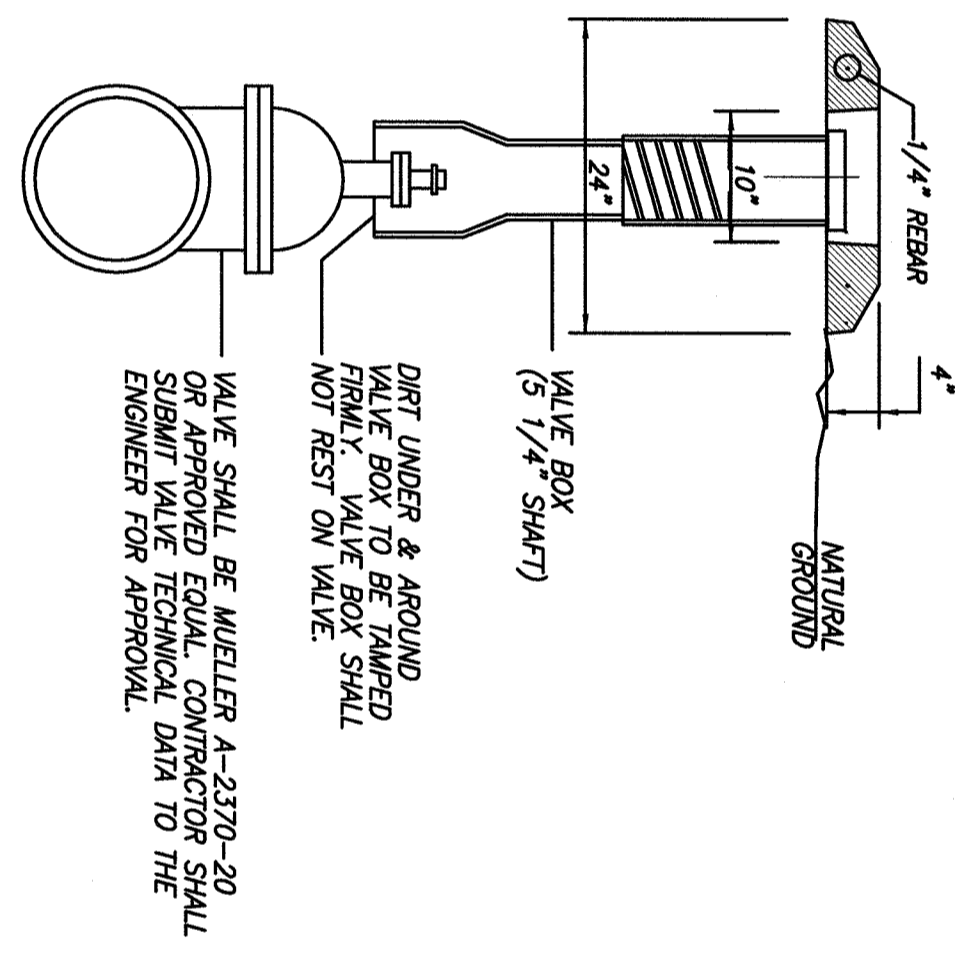
REINFORCED CONCRETE STRUCTURE THICKNESS  
FOR GARBAGE DUMPSTER  
NO SCALE  
C



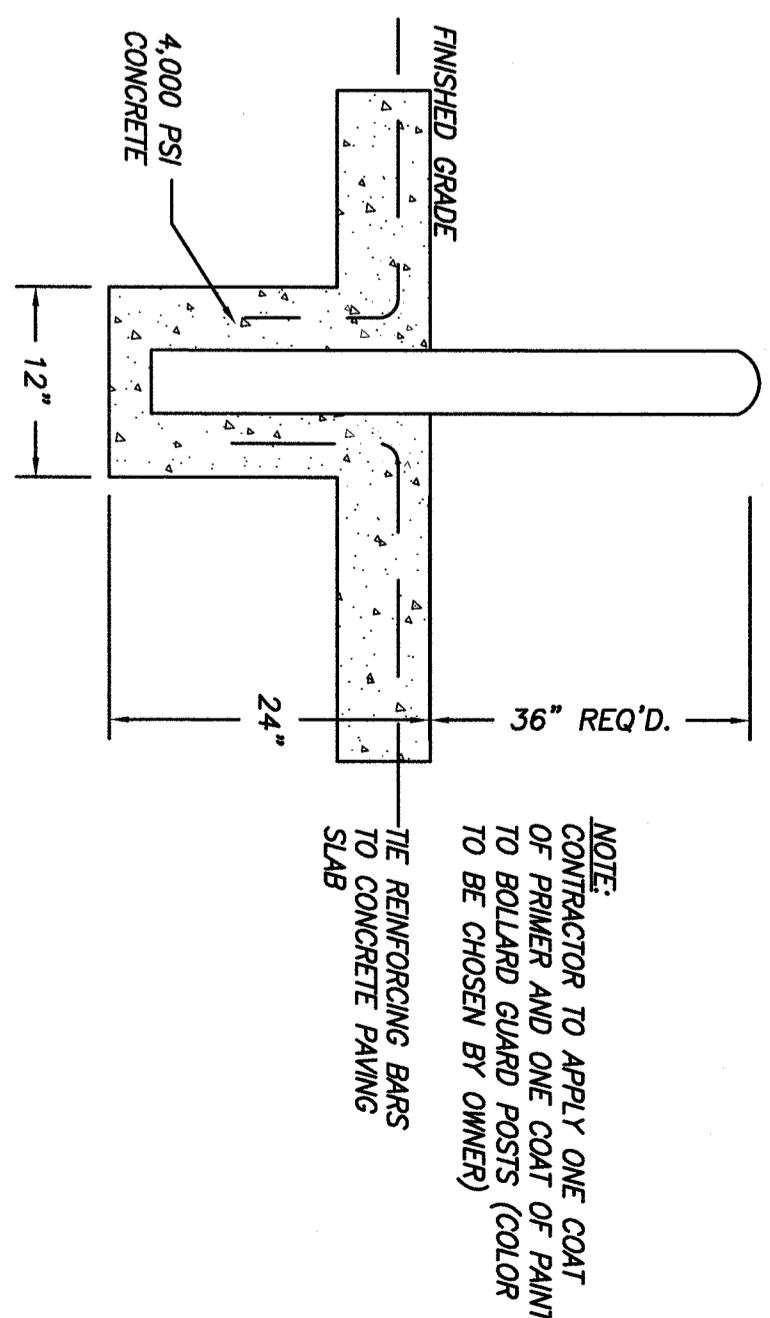
PAVEMENT HEADER CURB DETAIL  
NO SCALE  
J



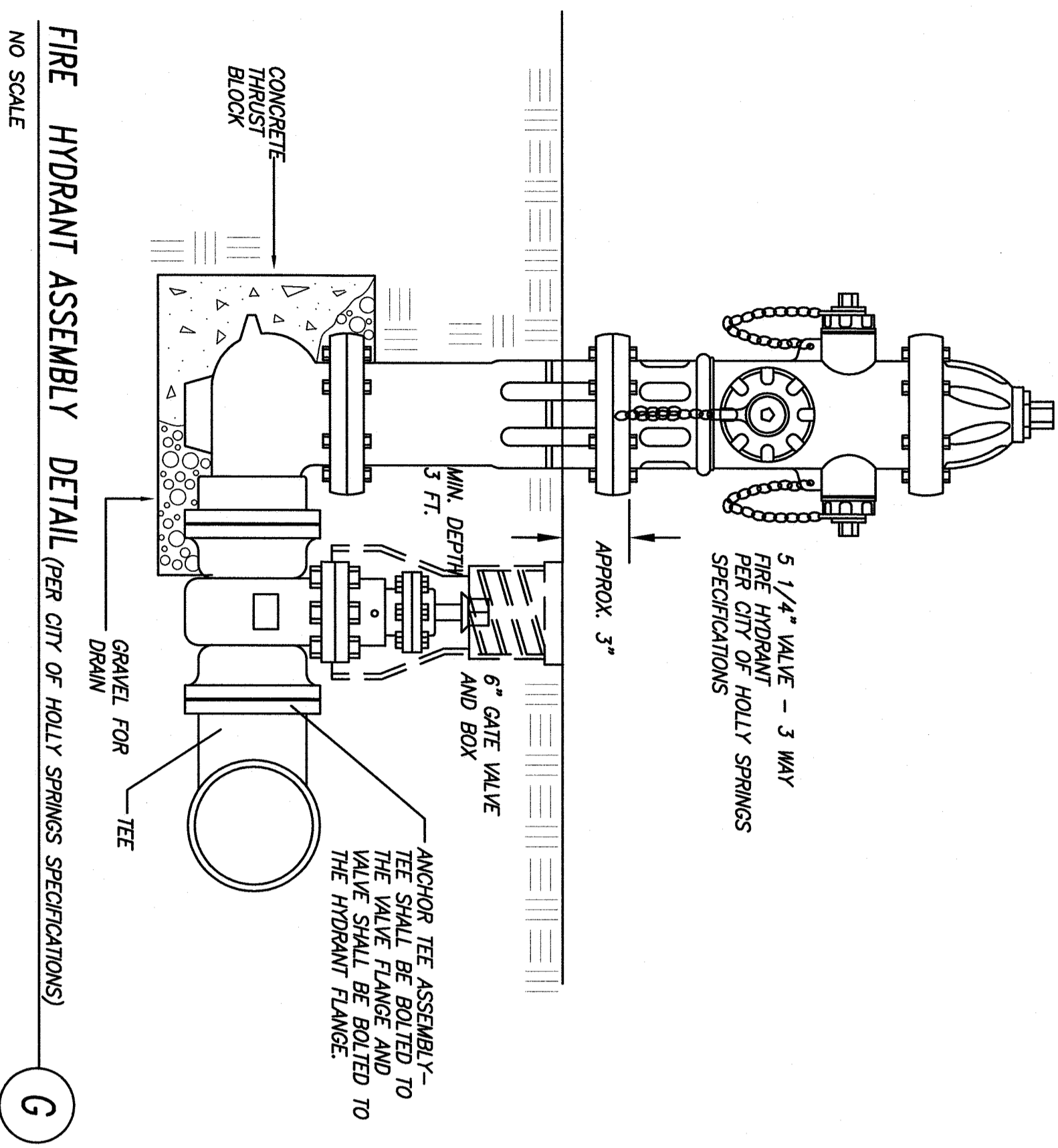
WET TAPPING TEE DETAIL ASSEMBLY  
NO SCALE  
D



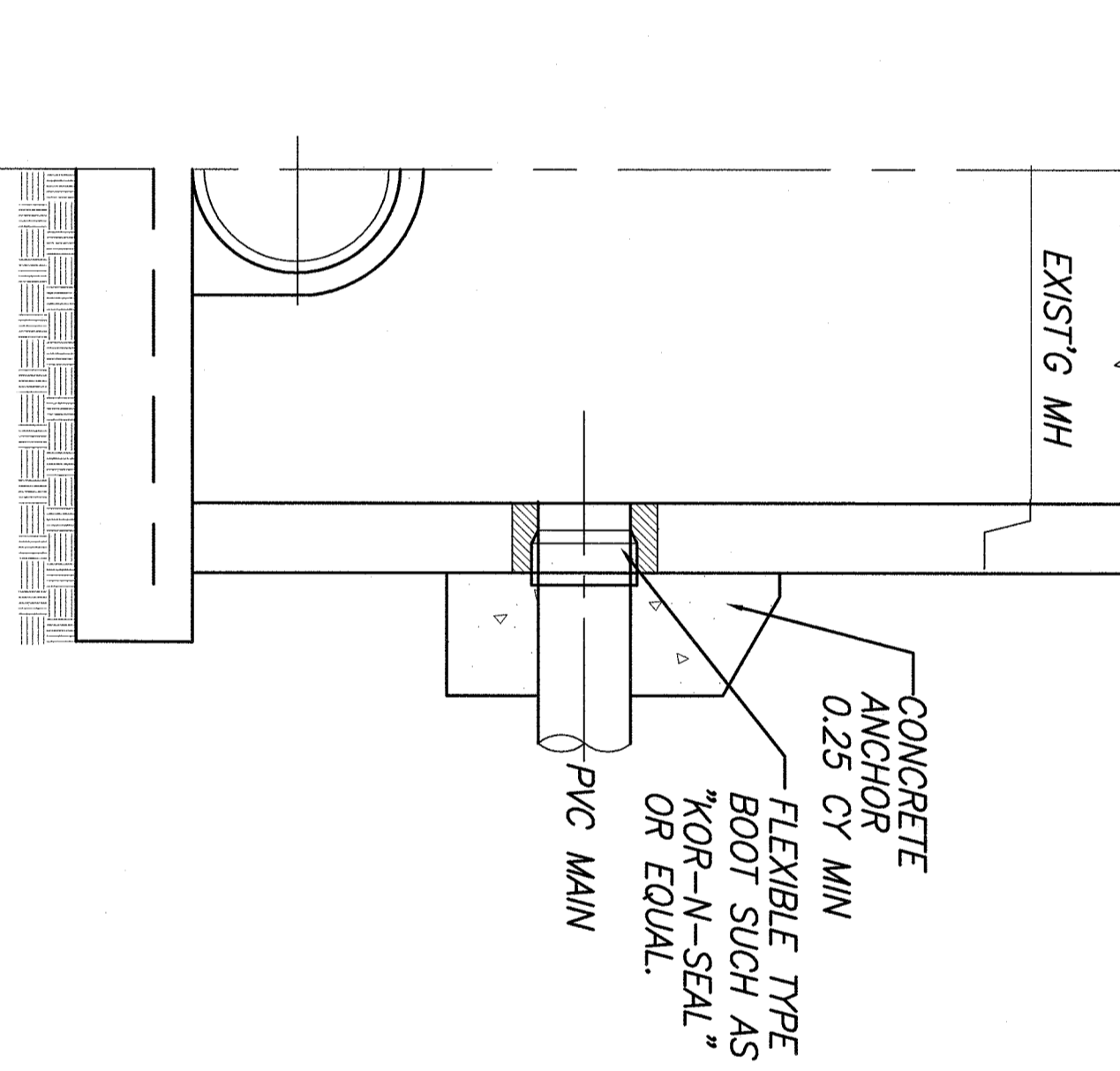
GATE VALVE AND BOX ASSEMBLY  
NO SCALE  
E



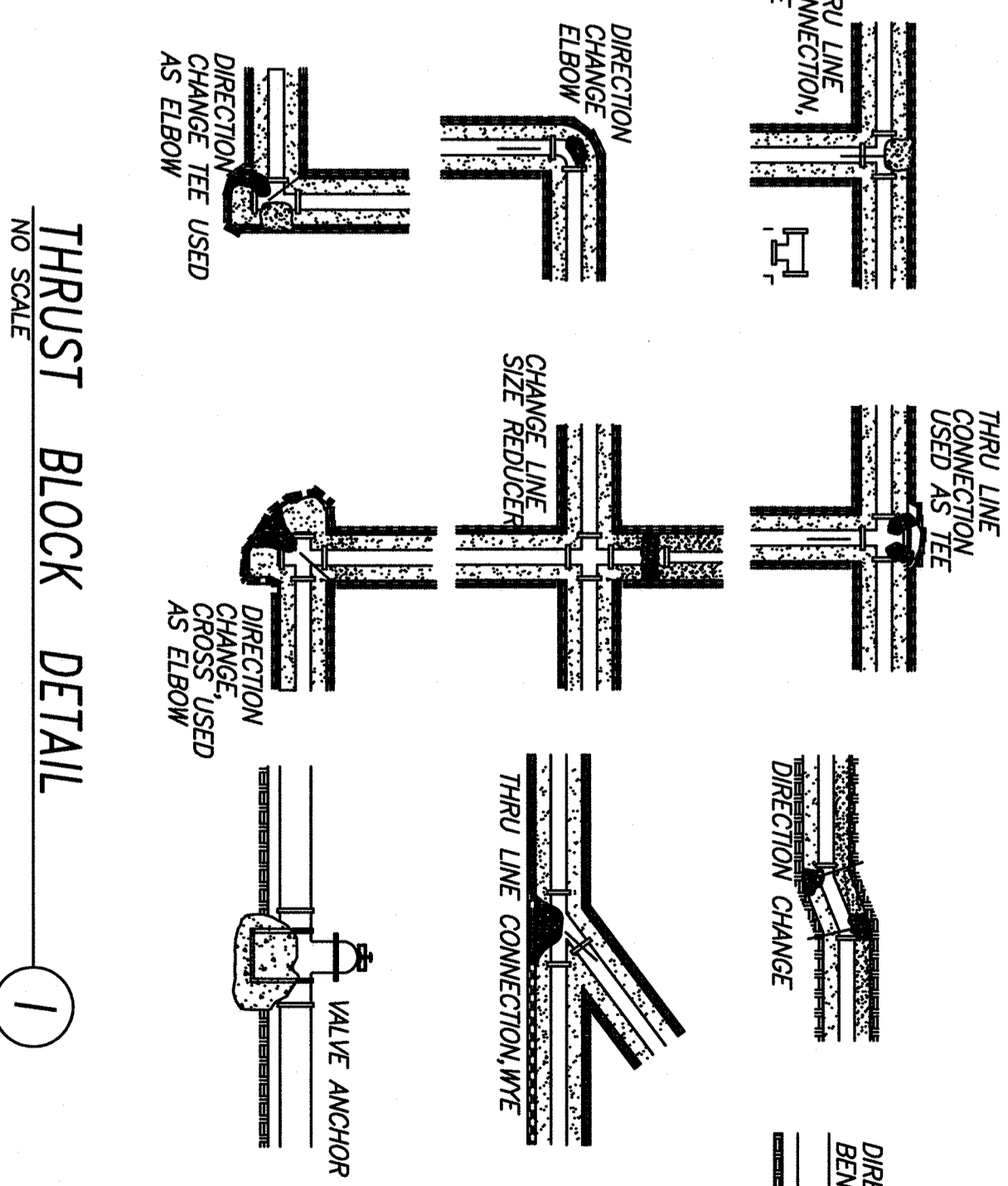
TYPICAL BOLLARD GUARD POST DETAIL  
NO SCALE  
F



FIRE HYDRANT ASSEMBLY DETAIL (PER CITY OF HOLLY SPRINGS SPECIFICATIONS)  
NO SCALE  
G



FOR USE WITH CONNECTION TO EXISTG MANHOLE  
MANHOLE CORE DETAIL  
NO SCALE  
H



THRUST BLOCK DETAIL  
NO SCALE  
I

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