WBA
ARCHITECTURE

19 August 2019

Pearl High School Multipurpose Building

ADDENDUM NO. 01

NOTICE TO ALL DOCUMENT HOLDERS:
The following additions, deletions, changes and clarifications to the drawings and specifications are to be included as part of the Contract Documents.

SPECIFICATIONS

ITEM NO. 01 **PROJECT MANUAL TABLE OF CONTENTS**
REPLACE the table of contents with the attached table of contents

ITEM NO. 02 **00.4322 UNIT PRICES FORM**
ADD the attached Specification **00.4322 UNIT PRICES FORM**

ITEM NO. 03 **02.2000 SITE CLEARING**
Part 3, Paragraph 3.02
REMOVE paragraph 3.02 "Disposal of waste vegetation if burning is allowed," in its entirety

ITEM NO. 04 **02.2100 SOIL EROSION AND SEDIMENT CONTROL**
Part 3, Paragraph 3.06
REMOVE paragraph 3.06 "Level Spreader," in its entirety

ITEM NO. 05 **02.2200 EXCAVATION AND EMBANKMENT**
Part 1, Paragraph 1.02, Section C
REVISE to read as follows: Report of Geotechnical Investigation for Pearl High School Multipurpose Building, Pearl, Mississippi Dated July 2019

ITEM NO. 06 **04.2000 UNIT MASONRY**
Part 1, Paragraph 1.2
ADD subsection "D. 01.2100 – Allowances"

ITEM NO. 07 **04.2000 UNIT MASONRY**
Part 2, Paragraph 2.1
Revise Paragraph 2.1 to read as follows:

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2.1 Concrete Masonry Units and Accessories

A. Concrete Masonry Units

1. Hollow concrete masonry units shall be normal weight or lightweight and shall comply with ASTM C-90, Grade N, Type I in color “Natural Gray,” having a compressive strength of 1,900 PSI. All block surfaces shall be uniform in texture and color.

2. Standard Block: Block shall be 8”x16” nominal modular face, thickness indicated on drawings.

3. Special Shape Concrete Masonry Units: Proved accessory shapes as indicated or otherwise required to include flush end, halves, lintel block, “U” blocks, jamb blocks, solid bull-nose wall caps and other special shapes.”

B. Reinforcement, Ties and Anchors

1. Steel Reinforcement Bars: ASTM A 615/A 615M, Grade 60 as indicated in the Contract Documents.

2. Joint Reinforcement: ASTM A 951.

a. Coating: Hot-dip galvanized.

b. Wire Diameter for Side Rods: W1.7 or 0.148 inch (3.8 mm).

c. Wire Diameter for Cross Rods: W1.7 or 0.148 inch (3.8 mm).

d. For single-wythe masonry, provide ladder design.

C. Accessories

1. PVC control joint: shall be equal to Hohmann & Barnard #VS-Standard. PVC material shall conform to ASTM D-2287 and tested in conformance with ASTM D-2240.

2. Control joint wall ties: shall be equal to Hohmann & Barnard Slip-set Stabilizer, hot-dipped galvanized using Type “H” for horizontal mortar joint and type “V” to connect to intersecting wall. Ties shall be placed 16” o.c. max. vertically.

D. Movement (Control & Expansion) Joint Accessories

1. General: Install control and expansion joints in unit masonry where indicated. Build in related items as the masonry progresses. Do not form a continuous span through movement joints unless provisions are made to prevent in-plane restraint of wall or partition movement.

2. Install pre-formed control joint gaskets designed to fit standard sash block. Install caulking on finish side (refer to division 7).

ITEM NO. 08 **04.2000 UNIT MASONRY**
Part 2, Paragraph 2.5
REVISE subsection A to read as follows:
"A. Metal Flashing Materials: Pre-Finished Galvanized Steel: ASTM A653/A653M, with G90/Z275 zinc coating; minimum 22 gauge thick base metal, shop pre-coated with PVDF coating, as specified in section 07.6200."

ITEM NO. 09 **04.2200 CONCRETE MASONRY**
REMOVE section in its entirety

ITEM NO. 10 **05.5000 METAL FABRICATIONS**
Part 2, Paragraph 2.4
REMOVE all references to elevators

ITEM NO. 11 **07.4646 FIBER-CEMENT SIDING**
REMOVE section in its entirety.

ITEM NO. 12 **07.5423 THERMOPLASTIC-POLYOLEFIN ROOFING (TPO)**
Part 2, Paragraph 2.2, Subsection A
Revise to read as follows:
"A. Roofing System: Thermoplastic polyolefin (TPO) single-ply membrane occurs only at roof side parapet wall applications."
Part 3, Paragraph 3.1
ADD subsection J: "J. install at roof-side of parapet walls as indicated in the drawings."
Part 3, Paragraph 3.7
REMOVE paragraph 3.7 "Finishing and walkway installation," in its entirety.

ITEM NO. 13 **07.6200 SHEET METAL FLASHING AND TRIM**
Part 2, Paragraph 2.7
***REMOVE* paragraph 2.7 "Manufactured copings,"** in its entirety.

ITEM NO. 14 **07.7100 ROOF SPECIALTIES**
Part 2, Paragraph 2.1, Subsection B
Revise to read as follows:
"Copings and edge trim flashings: shop Factory fabricated to sizes required; miter, welded corners; concealed fasteners."

ITEM NO. 15 **08.1416 FLUSH WOOD DOORS**
Part 2, Paragraph 2.4
ADD subsection C: "C. Veneer species: plain sliced cherry."

ITEM NO. 16 **08.3323 OVERHEAD COILING DOORS**
Part 2, Paragraph 2.3, Subsection B
CLARIFICATION Minimum thickness to be 22 gauge

ITEM NO. 17 **08.3323 OVERHEAD COILING DOORS**
Part 2, Paragraph 2.3, Subsection C
REMOVE subsection C in its entirety

ITEM NO. 18 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 1, Paragraph 1.9, Subsection A, Item 1
CLARIFICATION manufacture agrees to repair or replace defective storefront components for a period of 5 years from date of shipment.

ITEM NO. 19 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 1, Paragraph 1.9, Subsection B, Item 1, Subitem a
CLARIFICATION Paint Coatings to be as follows
1) AAMA 2605 70% PVDF 10 years
2) AAMA 2604 50% PVDF: 5 years
3) AAMA 2603 Baked Enamel: 1 year (adhesion only)
a. Anodized Coatings
i. AAMA 611 Class I: 5 years

ITEM NO. 20 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 2, Paragraph 2.1, Subsection A, Item 1
CLARIFICATION Tubelite Inc. 14000 outside plane SSG storefront: 2" x 4-1/2" to be thermally broken.

ITEM NO. 21 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 2, Paragraph 2.1, Subsection A, Item 2, Subitem a
REMOVE text "and be submitted at least ten working days prior to the bid date,"

ITEM NO. 22 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 2, Paragraph 2.2, Subsection A, Item 1, Subitem b
CLARIFICATION glazing thickness to be 1"
CLARIFICATION method to be "outside glazed".

ITEM NO. 23 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 2, Paragraph 2.4
REMOVE Subsection A, Item 1 in its entirety.
CLARIFICATION Subsection A, Item 5: exposed flashing to be 0.080" thick aluminum sheet.
CLARIFICATION Subsection A, Item 6: concealed flashing to be 22 ga. thick galvanized steel.
CLARIFICATION Subsection A, Item 7: structural steel reinforcement and anchors necessary to meet the performance requirements of 1.04. ASTM A36/A36M; galvanized per ASTM A123/A123M.

ITEM NO. 24 **08.4315 ALUMINUM-FRAMED STOREFRONTS**
Part 2, Paragraph 2.6,
REMOVE Subsections C, D, E & F in their entirety

ITEM NO. 25 **08.7100 DOOR HARDWARE**
Part 2, Paragraph 2.2, Subsection O
REPLACE the phrase "Seals shall be furnish as listed in schedule" with "Provide continuous seals at all exterior doors and at other locations as indicated in the door schedule and where specified herein."

ITEM NO. 26 **08.8723 SAFETY AND SECURITY FILMS**
Part 2, Paragraph 2.2
REMOVE Subsection G in its entirety.
REMOVE Subsection H, Item 7 in its entirety
REMOVE Subsection I in its entirety.

ITEM NO. 27 **09.2116 GYPSUM BOARD ASSEMBLIES**
Part 2, Paragraph 2.1
REMOVE Subsection D in its entirety

ITEM NO. 28 **09.3000 TILING**
Part 2, Paragraph 2.6
ADD Subsection E "Waterproof membrane – Liquid applied

ITEM NO. 29 **09.3000 TILING**
Part 3, Paragraph 3.3
Add Subsection N "Apply half a gallon of liquid applied waterproof membrane over 55 SF. Once dry, apply the remaining half gallon as a second coat."

ITEM NO. 30 **09.6566 RESILIENT ATHLETIC FLOORING**
Part 2, Paragraph 2.1
ADD Subsection B "See drawings and finish schedule for manufacture's info and pattern/ additional products information."

ITEM NO. 31 **09.8430 SOUND ABSORBING WALL AND CEILING UNITS**
Part 2, Paragraph 2.1, Subsection C, Item 4
CLARIFICATION panel thickness to be 1 ½".

ITEM NO. 32 **10.1400 SIGNAGE**
Part 2, Paragraph 2.5 - Plaques
ADD the following paragraph:
A. Basis of design –Gemini Incorporated www.geminisignproducts.com
Material: Aluminum 514 Alloy
Size: 18"x 24"
Depth: 5/16"
Border Style: Single Line
Background Texture: Sand
Background Clear Coat: Matte
Background Color: Black
Raised Copy: horizontal brushed grain direction
Letter Style: Selected by Arch. From manuf. Standard options
Logo: .eps file provided
Mounting: standard Blind mount hardware for mounting on masonry.

ITEM NO. 33 **10.1400 SIGNAGE**
Part 2, Paragraph 2.2, Subsection B
CLARIFICATION typical sign dimension to be 6"x12" unless noted otherwise.

ITEM NO. 34 **10.1400 SIGNAGE**
Part 2, Paragraph 2.2, Subsection B
CLARIFICATION Mens and Womens restroom signage dimensions to be 8.5"x8.5" unless noted otherwise.

ITEM NO. 35 **10.1400 SIGNAGE**
Part 2, Paragraph 2.6
REMOVE Paragraph 2.6 in its entirety

ITEM NO. 36 **10.2800 TOILET, BATH AND LAUNDRY ACESSORIES**
ADD the attached Specification **10.2113.19 PLASTIC TOILET COMPARTMENTS**

ITEM NO. 37 **APPENDIX**
REMOVE exhibit section in its entirety.

DRAWINGS

ITEM NO. 38 **SHEET C102 TYP. SECTIONS & MISC. DETAILS**
REPLACE sheet with attached revised sheet **C102**.
REVISES typical asphalt pavement sections. Subgrade preparation method noted on sheet per Geotech report.

ITEM NO. 39 **SHEET C401 GENERAL CONSTRUCTION DETAILS**
REPLACE sheet with attached revised sheet **C401**.
REVISES Junction box detail.

ITEM NO. 40 **SHEET A101 FIRST FLOOR PLAN**
REPLACE sheet with attached revised sheet **A101**.
Adds roof access ladder in Elec 105.

ITEM NO. 41 **SHEET A103 ROOF PLAN**
REPLACE sheet with attached revised sheet **A103**.
Adds detail of roof access ladder.

ITEM NO. 42 **SHEET A201 BUILDING SECTIONS**
REPLACE sheet with attached revised sheet **A201**.
Adds roof access ladder in Building Section A.

ITEM NO. 43 **SHEET A202 BUILDING SECTIONS**
REPLACE sheet with attached revised sheet **A202**.
Adds roof access ladder in Building Section A.

ITEM NO. 44 **SHEET A301 BUILDING ELEVATIONS**
REPLACE sheet with attached revised sheet **A301**.
REVISES Graphics of metal wall panel and Roof assembly legend

ITEM NO. 45 **SHEET A302 BUILDING ELEVATIONS**
REPLACE sheet with attached revised sheet **A302**.
REVISES Graphics of metal wall panel and Roof assembly legend

ITEM NO. 46 **SHEET A401 WALL SECTIONS**
REPLACE sheet with attached revised sheet **A401**.
Revises detail 2 to include roof access ladder

ITEM NO. 47 **SHEET A402 WALL SECTIONS**
REPLACE sheet with attached revised sheet **A402**.
Revises detail 2 to include CMU at wall section

ITEM NO. 48 **SHEET A403 WALL SECTIONS**
REPLACE sheet with attached revised sheet **A403**.
Revises detail names

ITEM NO. 49 **SHEET A404 WALL SECTIONS**
REPLACE sheet with attached revised sheet **A404**.
Revises detail names

ITEM NO. 50 **SHEET A504 WALL SECTIONS**
REPLACE sheet with attached revised sheet **A504**.
Revises detail names

ITEM NO. 51 **SHEET A505 WALL SECTIONS**
REPLACE sheet with attached revised sheet **A505**.
Revises detail names

ITEM NO. 52 **SHEET S1.0 STRUCTURAL NOTES**
REPLACE sheet with attached revised sheet **S1.0**.
Revises structural design category to B.

ITEM NO. 53 **SHEET S1.1 STRUCTURAL QUALITY ASSURANCE**
REPLACE sheet with attached revised sheet **S1.1**.
Adds aggregate pier quality assurance information.

ITEM NO. 54 **SHEET S2.0 FOUNDATION PLAN**
REPLACE sheet with attached revised sheet **S2.0**.
Revises grade beam layout, contraction joint layout, and footing step locations.
Adds slab reinforcement at inside corners.

ITEM NO. 55 **SHEET S2.1 CMU WALL PLAN**
REPLACE sheet with attached revised sheet **S2.1**.
Revises CMU wall reinforcement.

ITEM NO. 56 **SHEET S2.2 LOWER ROOF FRAMING PLAN**
REPLACE sheet with attached revised sheet **S2.2**.
Revises roof joist sizes and RTU support framing.

ITEM NO. 57 **SHEET S2.3 UPPER ROOF FRAMING PLAN**
REPLACE sheet with attached revised sheet **S2.3**.
Adds section at steel beam location.

ITEM NO. 58 **SHEET S3.0 DETAILS**
REPLACE sheet with attached revised sheet **S3.0**.
Revises details 4 & 5.
Adds detail 6.

ITEM NO. 59 **SHEET S3.1 DETAILS**
REPLACE sheet with attached revised sheet **S3.1**.
Adds details 7, 8, & 9.

ITEM NO. 60 **SHEET S3.2 DETAILS**
REPLACE sheet with attached revised sheet **S2.3**.
Adds details 1 & 2.

ITEM NO. 61 **SHEET S3.3 DETAILS**
REPLACE sheet with attached revised sheet **S3.3**.
Adds details 4 & 5.

ITEM NO. 62 **SHEET S3.4 DETAILS**
REPLACE sheet with attached revised sheet **S3.4**.
Revises CMU lintel schedule.
Adds information for 12” CMU with two bars in each cell.

ITEM NO. 63 **SHEET E-200 POWER PLAN**

REPLACE sheet with attached revised sheet **E-200**.

Revises location of electrical panels in Elec 105.

Encl: Project Manual Table of Contents

Specifications (8.5x11):

00.4322 *UNIT PRICES FORM,* ***10.2113.19*** *PLASTIC TOILET COMPARTMENTS,* ***31.6000*** *AGGREGATE PIERS*

Drawings (19 Sheets - 24x36):

C102 *TYP. SECTIONS & MISC DETAILS,* ***C104*** *GENERAL CONSTRUCTION DETAILS,* ***A101*** *FIRST FLOOR PLAN,* ***A103*** *ROOF PLAN,* ***A201*** *BUILDING SECTIONS,* ***A202*** *BUIDING SECTIONS,* ***A301*** *EXTERIOR ELEVATIONS,* ***A302*** *EXTERIOR ELEVATIONS,* ***A401*** *WALL SECTIONS,* ***A402*** *WALL SECTIONS,* ***A403*** *WALL SECTIONS,* ***A404*** *WALL SECTIONS,* ***A504*** *ROOF DETAILS,* ***A505*** *DETAILS,* ***S1.0*** *STRUCTURAL NOTES,* ***S1.1*** *STRUCTURAL QUALITY ASSURANCE,* ***S2.0*** *FOUNDATION PLAN,* ***S2.1*** *CMU WALL PLAN,* ***S2.2*** *LOWER ROOF PLAN,* ***S2.3*** *UPPER ROOF FRAMING PLAN,* ***S3.0*** *DETAILS,* ***S3.1*** *DETAILS,* ***S3.2*** *DETAILS,* ***S3.3*** *DETAILS,* ***S3.4*** *DETAILS,* ***E-200*** *POWER PLAN*

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02.8000 – Water Distribution System
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02.9000– Wastewater Collection System
02.9310– Establishment of Vegetation

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DIVISIONS 32 | EXTERIOR IMPROVEMENTS

DRAWINGS

See Index on Sheet R101

APPENDIX

END OF SECTION

SECTION 00.4322

UNIT PRICES FORM

PARTICULARS

1.1 THE FOLLOWING IS THE LIST OF UNIT PRICES REFERENCED IN THE BID SUBMITTED BY:

1.2 (BIDDER) _____

1.3 DATED _____ AND WHICH IS AN INTEGRAL PART OF THE BID FORM.

UNIT PRICE LIST

2.1 UNDERCUT EXCAVATION, FM, AH QTY: 1195 CY UNIT COST _____ TOTAL COST _____

A. Quantity for UNDERCUT EXCAVATION Pay Item includes undercutting 3' below the bottom of the asphalt in all areas where the asphalt is being removed and replaced with New Asphalt and 2' beyond the back of curb in such areas. Payment for this item will be based on the amount of material removed. Determination of undercut areas shall be by proof roll with the engineer and 3rd party geotechnical engineer once the subgrade in the affected area is exposed. Measurement for payment will be by field measure performed by contractor of material that was removed and verified by engineer prior to filling. Payment shall be based on the quantity determined by the field measurements.

2.2 SELECT FILL MATERIAL, FM, AH QTY: 1195 CY UNIT COST _____ TOTAL COST _____

A. Quantity for SELECT FILL MATERIAL Pay Item includes providing select fill material, filling and compacting the undercut areas. Select fill material, placement and compaction of such l shall meet the requirements of the geotechnical report. Measurement for payment for select material shall be based on the field measurement of the undercut quantity and adjusted as needed where crushed stone is placed under pavement. Payment shall be based on the quantity determined by the field measurements.

END OF SECTION

SECTION 10.2113.19

PLASTIC TOILET COMPARTMENTS

PART 1 GENERAL

1.1 SECTION INCLUDES

A. Solid plastic toilet compartments.

B. Urinal and vestibule screens.

1.2 RELATED REQUIREMENTS

A. Section 05.5000 - Metal Fabrications: Concealed steel support members.

B. Section 10.2800 - Toilet, Bath, and Laundry Accessories.

1.3 REFERENCE STANDARDS

A. ASTM A666 - Standard Specification for Annealed or Cold-Worked Austenitic Stainless Steel Sheet, Strip, Plate, and Flat Bar; 2015.

B. ASTM E84 - Standard Test Method for Surface Burning Characteristics of Building Materials; 2015a.

C. NFPA 286 - Standard Methods of Fire Tests for Evaluating Contribution of Wall and Ceiling Interior Finish to Room Fire Growth; 2015.

1.4 SUBMITTALS

A. See Section 01.3000 - Administrative Requirements, for submittal procedures.

PART 2 PRODUCTS

2.1 MANUFACTURERS

A. Solid Plastic Toilet Compartments:
   1. Scranton Products (Santana/Comtec/Capital); _____: www.scrantonproducts.com/#sle.
   2. Substitutions: Section 01.6000 - Product Requirements.

2.2 PLASTIC TOILET COMPARTMENTS

A. Solid Plastic Toilet Compartments: Factory fabricated doors, pilasters, and divider panels made of solid molded high density polyethylene (HDPE), tested in accordance with NFPA 286; floor-mounted unbraced.

B. Doors:
   1. Thickness: 1 inch (25 mm).
   2. Width: 24 inch (610 mm).
   3. Width for Handicapped Use: 36 inch (915 mm), out-swinging.
   4. Height: 55 inch (1397 mm).

C. Panels:
   1. Thickness: 1 inch (25 mm).
   2. Height: 55 inch (1397 mm).

D. Pilasters:
   1. Thickness: 1 inch (25 mm).
   2. Width: As required to fit space; minimum 3 inch (76 mm).

E. Screens: Without doors; to match compartments; mounted to wall with two panel brackets.

2.3 ACCESSORIES

A. Pilaster Shoes: Stainless steel, satin finish, 3 inches (76 mm) high; concealing floor fastenings.

B. Wall and Pilaster Brackets: Stainless steel; manufacturer's standard type for conditions indicated on drawings.

C. Pilaster Brackets: Polished stainless steel.

D. Wall Brackets: Continuous type, polished stainless steel.

E. Attachments, Screws, and Bolts: Stainless steel, tamper proof type.

F. Hardware: Polished stainless steel:
   1. Pivot hinges, gravity type, adjustable for door close positioning; two per door.
   2. Door Latch: Slide type with exterior emergency access feature.
   3. Door strike and keeper with rubber bumper; mounted on pilaster in alignment with door latch.
   4. Coat hook with rubber bumper; one per compartment, mounted on door.
   5. Provide door pull for outswinging doors.

PART 3 EXECUTION

3.1 EXAMINATION

A. Verify that field measurements are as indicated.

B. Verify correct spacing of and between plumbing fixtures.

3.2 INSTALLATION

A. Install partitions secure, rigid, plumb, and level in accordance with manufacturer's instructions.

B. Maintain 3/8 inch to 1/2 inch (9 mm to 13 mm) space between wall and panels and between wall and end pilasters.

C. Attach panel brackets securely to walls using anchor devices.

D. Attach panels and pilasters to brackets. Locate head rail joints at pilaster center lines.

END OF SECTION

SECTION 31.6000

Aggregate Piers

PART 1 - GENERAL

1.1 RELATED DOCUMENTS: Drawings and general provisions of the Contract, including General and Supplementary Conditions and other Division 00 and Division 01 Specification Sections, apply to this Section.

1.2 DESCRIPTION: Work shall consist of designing, furnishing and install materials, and construction of a ground improvement system as specified herein. Ground improvement system shall be either vibro stone columns or rammed piers. "Aggregate piers" referenced in these specifications refer to both vibro stone columns and rammed piers.

1.3 WORK INCLUDED:

A. Provisions of all equipment, material, labor, and supervision to design and install aggregate pier elements. Design shall rely on subsurface information presented in the project geotechnical report. Removal of spoils from the site (which result from aggregate pier construction), removal of spoils off the working pad, footing excavation, and subgrade preparation following aggregate pier installation is not included.

B. Drawings and General Provisions of the Contract, including General and Supplemental Conditions, and Division 1 Specifications, apply to the work in this specification.

1.4 APPROVED INSTALLERS:

A. Installers of aggregate pier foundation systems shall have a minimum of 5 years of experience with the installation of aggregate piers.

1.5 RELATED WORK:

A. Section 03.3000 – Cast in Place Concrete.

B. Section 31.2000 – Earthwork for Buildings.

C. Geotechnical Report and Recommendations.

1.6 REFERENCE STANDARDS:

A. Design: The ground improvement installer shall be responsible for design of a vibro stone column or rammed pier ground improvement system that meets the global stability, allowable bearing capacity, and settlement requirements stated on the contract plans. Industry recognized standards or design methods specific to the installer's equipment and construction methods shall be used. The design shall be sealed by a Mississippi licensed Professional Engineer.

B. Modulus and Uplift Testing:
1. ASTM D-1143 – Pile Load Test Procedures.
2. ASTM D-1194 – Spread Footing Load Test
3. ASTM-D-3689 – Uplift Load Test

C. Materials and Inspection:

1. ASTM D-1241 – Aggregate Quality.
2. ASTM STP 399 – Dynamic Penetrometer Testing.
3. ASTM D-422 – Gradation Soils.

1.7 CONFLICTS IN SPECIFICATIONS/REFERENCES: Where specifications and reference documents conflict, the Architect/Engineer shall make the final determination of the applicable document

1.8 CERTIFICATIONS IN SPECIFICATIONS/REFERENCES:

A. The installer shall submit detailed design calculations and construction drawings to the Architect and to the Geotechnical Engineer of Record for approval at least three (4) weeks prior to the start of construction. All plans shall be sealed by a Professional Engineer in the State of Mississippi (referred in this specification as "the Designer").

B. The Stone Column or Aggregate Pier engineer shall have Errors and Omissions design insurance for the work. The insurance policy should provide a minimum coverage of $2 million per occurrence.

C. Modulus and uplift test data - The Installer shall furnish the General Contractor a description of the installation equipment, installation records, complete test data, analysis of the test data and recommended design parameter values based on the modulus test results. The report shall be prepared under supervision of a registered professional engineer.

D. Daily Progress Reports – The Installer shall furnish a complete and accurate record of aggregate pier installation to the General Contractor. The record shall indicate the pier location, length, average lift thickness and final elevations of the base and top of piers. The record shall also indicate the type and size of the densification equipment used. The Installer shall immediately report any unusual conditions encountered during installation to the General Contractor, to the Designer and to the Testing Agency.

PART 2 - PRODUCTS

2.1 MATERIALS:

A. Aggregate used for piers constructed above the water table shall be Type I Grade B in accordance with ASTM D-1241-68 or shall be other graded aggregate selected by the Installer and successfully used in the modulus test. It shall be compacted to a densification and strength, which provides resistance to the dynamic penetration test (ASTM STP 399) of a minimum average of 15 blows per 1.75-inch vertical movement.

B. For aggregate used for piers constructed below the water table, the gradation shall be the same as Type I Gradation B, except that particles passing the No. 40 sieve shall be eliminated. Alternatively, No.57 stone or other stone selected by the Stone Columns or Aggregate Pier Installer may be used. Dynamic penetration resistance testing is inappropriate for this material.

C. Potable water or other suitable source shall be used to increase aggregate moisture content where required. Access to water on site shall be provided to the Installer.

D. Installer to coordinate adequate and suitable marshalling areas on the project site for the use of the Installer for the storage of aggregate and equipment.

PART 3 - EXECUTION

3.1 STONE COLUMN AND AGGREGATE PIER DESIGN:

A. The Aggregate Pier design stiffness modulus value shall be verified by the results of the modulus test, GROUND SOIL IMPROVEMENT described in this specification.

B. Stone Columns or Aggregate piers shall be designed in accordance with generally accepted engineering practice and the methods described in Section 1 of these Specifications. The design shall meet the following criteria.
   1. Aggregate piers shall be designed to improve the soil for all footings, grade, beams, and slab-on-grade.
   2. Minimum Allowable Bearing Pressure for Aggregate Pier Reinforced Soils: 3,000 psf.
   3. Estimated Total Long-Term Settlement for Footings: ≤1-inch.
   4. Estimated Long-Term Differential Settlement of Adjacent Footings: ≤ ½-inch.

C. The design submitted by the Installer shall consider the bearing capacity and settlement of all footings and slab-on-grade supported by aggregate piers and shall be in accordance with acceptable engineering practice and these specifications. Total and differential settlement shall be considered. The design life of the structure shall be 50 years.

D. The Stone Column or Aggregate Pier system shall be designed to preclude plastic bulging deformations at the top-of–pier design stress and to preclude significant tip stresses as determined from the shape of the telltale test curve from telltales installed in modulus test piers. The results of the modulus test shall be used to verify the design assumptions.

3.2 DESIGN SUBMITTAL: The Installer shall submit detailed design calculations, construction drawings, and shop drawings, (the Design Submittal), for approval at least three (4) weeks prior to the beginning of construction. A detailed explanation of the design parameters for settlement calculations shall be included in the Design Submittal. Additionally, the quality control test program for stone columns or aggregate piers, meeting these design requirements, shall be submitted. All computer-generated calculations and drawings shall be prepared and sealed by a Professional Engineer, licensed in Mississippi.

PART 4 - CONSTRUCTION

4.1 STONE COLUMNS:

A. Install stone columns with a down-hole vibrator capable of densifying the aggregate by forcing it radially into the surrounding soil. The vibrator shall be of sufficient size and capacity to construct stone columns to the diameters and lengths shown on the installer's approved construction drawings.

B. The probe and follower tubes shall be of sufficient length to reach the elevations shown on the installer's approved construction drawings. The probe, used in combination with the available pressure to the tip jet, shall be capable of penetration to the required tip elevation. Preboring shall be permitted if it is specified in the installer's approved construction procedure submittal.

C. The probe and follower shall have visible markings at regular increments to enable measurement of penetration and repenetration depths.

D. Provide methods for supplying to the tip of the probe a sufficient quality of air or water to widen the probe hole to allow adequate space for stone backfill placement around the probe.

E. The probe shall penetrate into the foundation soil layer to the minimum depths required in the installer's construction plans

F. Lift thickness shall not exceed 4 feet. After penetration to the treatment depth, slowly retrieve the vibrator in 12-inch to 18-inch increments to allow backfill placement.

G. Compact the backfill in each lift by repenetrating it at least twice with the vibrating probe to densify and force the stone into the surrounding soil.

H. Install stone columns so that each completed column is continuous throughout its length.

4.2 RAMMED PIERS:

A. All Aggregate Pier elements shall be pre-augered using mechanical drilling or excavation equipment. Installation of piers without pre-augering shall not be allowed because this technique results in significant disturbance and remolding of the matrix soils surrounding the piers.

B. If cave-ins occur during excavation such that the sidewalls of the hole are deemed to be unstable, steel casing or a drilling slurry shall be used to stabilize the excavation

C. If cave-ins occur on top of a lift of aggregate such that the volume of the caved soils is greater than 10 percent of the volume of the aggregate in the lift, then the aggregate shall be considered contaminated and shall be removed and replaced with uncontaminated aggregate.

D. Special high-energy impact densification apparatus shall be employed to densify the Aggregate Pier elements during installation. The apparatus shall apply direct downward impact energy to each lift of aggregate.

E. A minimum tamper energy level of 250,000 foot-pounds of force per minute shall be applied by the energy source.

F. The bottom of the excavation shall be densified prior to the placement of the aggregate. If wet, soft or sensitive soils are present, open-graded aggregate, such as ASTM No.57 stone or other, shall be placed at the bottom of the excavation and compacted to stabilize the element bottom and may serve as the initial lift.

G. Densification shall be performed using a beveled tamper. The beveled tamper foot is required to adequately increase the lateral earth pressure in the matrix soil during installation.

H. Downward pressure shall be applied to the tamper shaft during tamping.

I. Each lift of aggregate shall be tamped for a minimum of 15 seconds.

4.3 PLAN LOCATION AND ELEVATION OF AGGREGATE PIER ELEMENTS: The center of each pier shall be within six inches of the plan locations indicated. The final measurement of the top of piers shall be the lowest point on the aggregate in the last compacted lift. Piers installed outside of the above tolerances and deemed not acceptable shall be rebuilt at no additional expense to the Owner.

4.4 REJECTED AGGREGATE PIER ELEMENTS: Aggregate pier elements improperly located or installed beyond the maximum allowable tolerances shall be abandoned and replaced with new piers, unless the

Designer approves other remedial measures. All material and labor required to replace rejected piers shall be provided at no additional cost to the Owner.

PART 5 - QUALITY CONTROL

5.1 QUALITY CONTROL REPRESENTATIVE:

A. The Installer shall have a full-time Quality Control (QC) representative to verify and report all QC installation procedures. The Installer shall immediately report any unusual conditions encountered during installation to the Design Engineer, the General Contractor, and to the Testing Agency. The QC procedures shall include the preparation of Aggregate Pier Progress Reports completed during each day of installation and containing the following information:
   1. Footing and Aggregate Pier location.
   2. Aggregate Pier length and drilled diameter.
   3. Planned and actual Aggregate Pier elevations at the top and bottom of the element.
   4. Average lift thickness for each Aggregate Pier.
   5. Soil types encountered at the bottom of the Aggregate Pier and along the length of the element
   6. Depth to groundwater, if encountered.
   7. Documentation of any unusual conditions encountered.
   8. Type and size of densification equipment used.

5.2 QUALITY CONTROL VERIFICATION PROGRAM:

A. The installer shall be responsible for design of a verification program to assure the quality of the construction. The program shall verify that the installed ground improvement system satisfies the performance requirements noted on the contract plans and the design requirements determined by the ground improvement system designer. As a minimum, the verification program shall include the following:
   1. Program to monitor performance of the ground improvement system during and after construction of the proposed structure or embankment to be supported. This program may include installation of settlement plates, monitoring points, inclinometers, piezometers, or other instrumentation.
   2. Stone column installation shall be monitored by an on-board computer monitoring system. Monitoring system shall log stone column number, time of installation, depth, hydraulic pressure applied during the boring process and during the compacting process. Recorded data for each stone column shall be plotted depth/pressure versus time. Installation records for each shall be made available upon request in electronic format within 24 hours of installation.
   3. Proposed means and methods for verification that the installed aggregate piers meet the strength and/or stiffness criteria required by the design. This may include, but shall not be limited to, modulus or load tests on individual elements and/or groups, soil borings, and other methods as approved by the Engineer.
   4. Quality control program to verify that the ground improvement system is installed in accordance with the designer's specifications and the requirements in this special provision. The quality control program shall include testing and observations by qualified personnel employed by the ground improvement installer or an independent testing laboratory.

PART 6 - QUALITY ASSURANCE

6.1 INDEPENDENT ENGINEERING TESTING AGENCY: The General Contractor is responsible for retaining an independent engineering testing firm to provide Quality Assurance services. The Testing Agency should be the Geotechnical Engineer of Record.

6.2 RESPONSIBILITIES OF GEOTECHNICAL ENGINEER & INDEPENDENT ENGINEERING TESTING AGENCY:

A. The Geotechnical Engineer of Record shall review and approve the Installer's Design Submittal.

B. The Testing Agency shall monitor the installation of aggregate pier elements to verify that all work is performed in accordance with the approved Design Submittal.

C. The Testing Agency & Geotechnical Engineer of Record shall observe footing excavations and densification GROUND SOIL IMPROVEMENT of aggregate piers and provide written reports per section 7.3.D.

D. The Testing Agency shall report any discrepancies to the Installer and General Contractor immediately.

PART 7 - RESPONSIBILITIES OF GENERAL CONTRACTOR

7.1 PREPERATION:

A. The Installer shall locate and protect underground and aboveground utilities and other structures from damage during installation of the Aggregate Pier elements.

B. The General Contractor will provide the site to the Installer, after earthwork in the area has been completed.

C. Site subgrade shall be established by the General Contractor within 6 inches of final design subgrade, as approved by the Design Engineer.

7.2 UTILITY EXCAVATIONS:

A. The General Contractor shall coordinate all excavations made subsequent to Aggregate Pier installations so that at least five feet of horizontal distance remains between the edge of any installed Aggregate Pier and the excavation. In the event that utility excavations are required at horizontal distances of less than five feet from installed Aggregate Piers, the General Contractor shall notify the Aggregate Pier Designer to develop construction solutions to minimize impacts on the installed Aggregate Piers.

B. Recommended procedures may include:
   1. Using cement-treated base to construct portions of the Aggregate Piers subject to future excavations.
   2. Replacing excavated soil with compacted crushed stone in the portions of excavations where the Aggregate Piers have been disturbed. The placement and compaction of the crushed stone shall meet the following requirements.
      a. The crushed stone shall meet the gradation specified by the Designer.
      b. The crushed stone shall be placed in a controlled manner using motorized impact compaction equipment.
      c. The aggregate should be compacted to 95% of the maximum dry density as determined by the modified Proctor method (ASTM D-1557).
      d. The Testing Agency shall be on site to observe placement, compaction, and provide density testing. The test results shall be submitted to the Designer and the General

Contractor. The subcontractor shall provide notification to the Testing Agency and the Designer when excavation, placement, and compaction will occur and arrange for construction observation and testing.

7.3 FOOTING BOTTOMS:

A. Excavation and surface compaction of all footings shall be the responsibility of the General Contractor

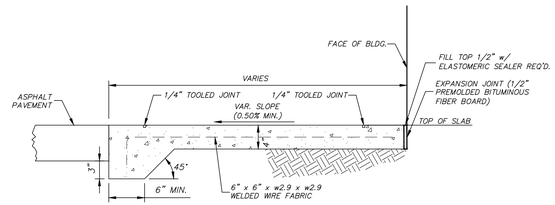
B. Foundation excavations to expose the tops of Aggregate Pier elements shall be made in a workmanlike manner, and shall be protected until concrete placement, with procedures and equipment best suited to (1) prevent softening of the matrix soil between and around the Aggregate Pier elements before pouring structural concrete, and (2) achieving direct and firm contact between the dense, undisturbed Aggregate Pier elements and the concrete footing.

C. Recommended procedures for achieving these goals are to:
   1. Limit over-excavation below the bottom of the footing to 3-inches (including disturbance from the teeth of the excavation equipment.
   2. Compaction of surface soil and top of Aggregate Pier elements shall be prepared using a motorized impact compactor ("Wacker Packer," "Jumping Jack," or similar). Sled-type tamping devices shall not be used. Compaction shall be performed over the entire footing bottom to compact any loose surface soil and loose surface pier aggregate.
   3. Place footing concrete immediately after footing excavation is made and approved, preferably the same day as the excavation. Footing concrete must be placed on the same day if the footing is bearing on expansive or sensitive soils.
   4. If same day placement of footing concrete is not possible, place a minimum 3-inch thick lean concrete seal ('mud mat") immediately after the footing is excavated and approved.

D. The following criteria shall apply, and a written inspection report sealed by the project Geotechnical Engineer shall be furnished to the Installer to confirm:
   1. That water (which may soften the unconfined matrix soil between and around the Aggregate Pier elements and may have detrimental effects on the supporting capability of the Aggregate Pier reinforced subgrade) has not been allowed to pond in the footing excavation at any time.
   2. That all Aggregate Pier elements designed for each footing have been exposed in the footing excavation.
   3. That immediately before footing construction, the tops of all the Aggregate Pier elements exposed in each footing excavation have been inspected and recompacted as necessary with mechanical compaction equipment, and that the tops of any Aggregate Pier elements which may have been disturbed by footing excavation and related activity have been recompacted to a dry density equivalent to at least 95% of the maximum dry density obtainable by the modified Proctor method (ASTM D-1557).
   4. That no excavations or drilled shafts have been made after installation of Aggregate Pier elements within horizontal distance of five feet from the edge of any pier, without the written approval of the Installer or Designer.

END OF SECTION

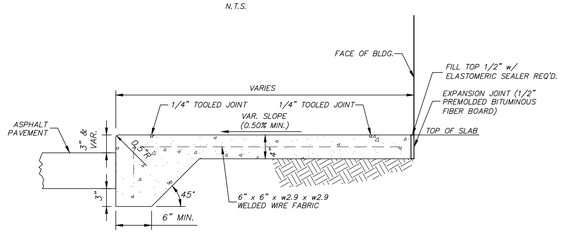
BENCHMARK
ENGINEERING & SURVEYING, LLC
101 Highpointe Court, Suite B, Brandon, Mississippi 39042
Office: 601-591-1077 Fax: 601-591-0711
E-mail: ghood@benchmarkms.net
PROJECT #: B-7258



NOTE: TOOLED JOINTS TO BE EQUALLY SPACED

SIDEWALK SECTION A–A

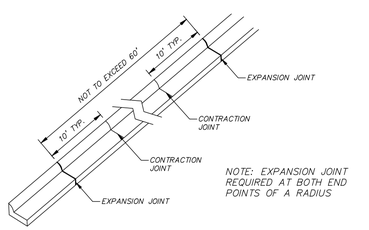
N.T.S.



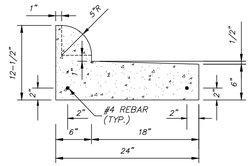
NOTE: TOOLED JOINTS TO BE EQUALLY SPACED

SIDEWALK SECTION B–B

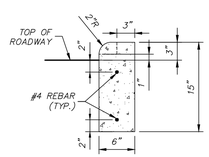
N.T.S.



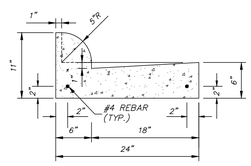
TYPICAL CURB AND GUTTER JOINT SPACING



STANDARD CURB WITH REVERSED GUTTER



HEADER CURB

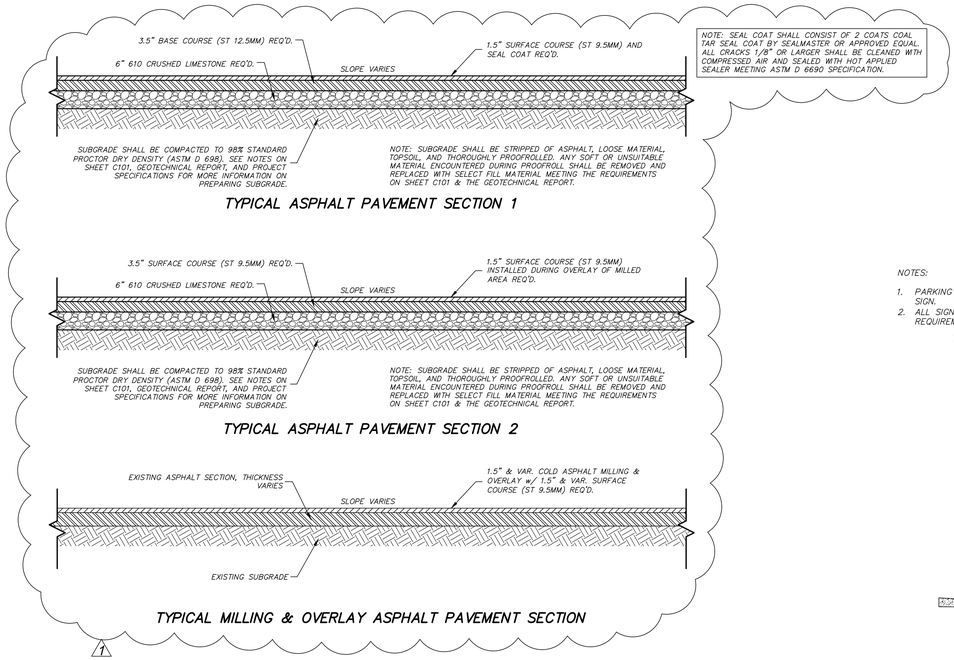


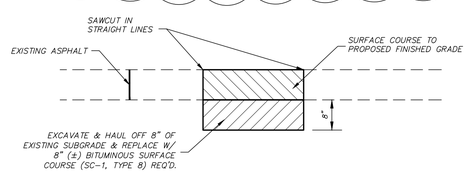
STANDARD CURB & GUTTER

CURB AND GUTTER NOTES:

1. CURB AND GUTTER SHALL BE 3500 PSI MINIMUM CONCRETE.
2. CURB AND GUTTER SHALL BE PLACED ON COMPACTED SUB-GRADE MEETING ROADWAY SPECIFICATIONS. SUB-GRADE SHALL PASS A PROOF ROLL WITH THE ENGINEER OR HIS REPRESENTATIVE PRIOR TO POURING ANY PORTION OF THE CURB AND GUTTER.
3. PROVIDE EXPANSION JOINTS WITH 1/2" EXPANSION MATERIAL AT INTERVALS NOT TO EXCEED SIXTY (60') FEET. TWO 3/4" DOWEL BARS HELD IN PLACE BY APPROVED CHAIRS OR SUPPORTS, 15" IN LENGTH REQ'D. AT EXPANSION JOINTS.
4. TOOLED CONTRACTION/CONTROL JOINTS (1/4" WIDE x 1" DEEP) ARE REQUIRED IN THE CURB AND GUTTER AT EVENLY SPACED INTERVALS NOT TO EXCEED TEN (10') FEET.
5. CONTRACTOR SHALL MAKE TRANSITION FROM STANDARD CURB TO REVERSED GUTTER THROUGH A RADIUS IS POSSIBLE AND IF NOT, GRADUALLY TRANSITION BETWEEN CONTRACTION JOINTS.
6. LOCATIONS FOR REVERSE GUTTER ARE NOTED ON THE DRAWINGS. THE CONTRACTOR MAY PROPOSE TO ADJUST LOCATIONS BY PROVIDING ENGINEER WITH A DRAWING WITH PROPOSED REVERSE GUTTER LOCATIONS FOR HIS REVIEW AND COMMENT/REJECTION/APPROVAL.

CURB AND GUTTER DETAILS

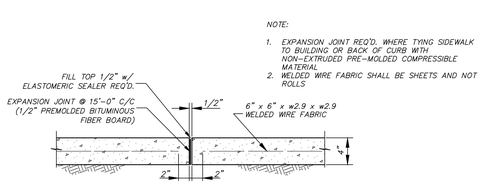




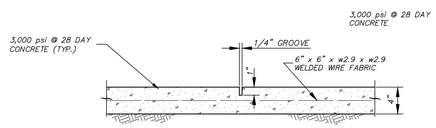
NOTES:

1. THIS DETAIL SHALL APPLY FOR EXISTING PAVEMENT FAILURES OR BASE FAILURE REPAIRS WITHIN THE PROJECT AREA OR DIRECTLY CAUSED BY CONSTRUCTION OPERATIONS OF THE GENERAL CONTRACTOR OR ANY OF THEIR SUB-CONTRACTORS.
2. IT IS THE CONTRACTORS RESPONSIBILITY TO REPAIR ALL FAILED PAVEMENT AREAS WITHIN THE PROJECT LIMITS OR OTHER SUCH AREAS CAUSED BY THEIR EQUIPMENT. THIS APPLIES TO ALL ROADWAYS ON THE PEARL HIGH SCHOOL PROPERTY. CONTRACTOR SHALL PROVIDE OWNER WITH PICTURE DOCUMENTATION AND SITE INSPECTION WITH ENGINEER OF ANY SUCH AREAS THAT ARE PRESENT ON SAID PROPERTY PRIOR TO BEGINNING THE PROJECT OR BE RESPONSIBLE FOR REPAIR.
3. ALL PAVEMENT REPAIR AREAS SHALL BE FILLED THE SAME DAY THE REPAIR IS MADE.

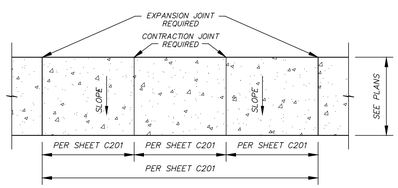
FAILED PAVEMENT REPAIR DETAIL



SIDEWALK EXPANSION JOINT DETAIL



SIDEWALK CONTRACTION JOINT DETAIL



NOTES:
1. SIDEWALK TO BE SLOPED TO DRIVES & AWAY FROM BUILDINGS
2. EXPANSION AND CONTRACTION JOINTS SHALL BE INSTALLED PER SHEET C201 OR AS DIRECTED BY ENGINEER.

SIDEWALK JOINT LAYOUT DETAIL

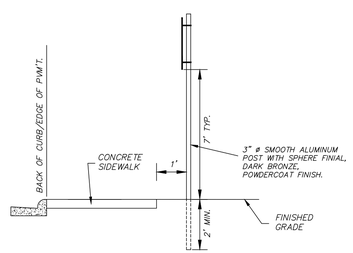
R7-8
12"x18"

(GREEN AND BLUE ON WHITE)

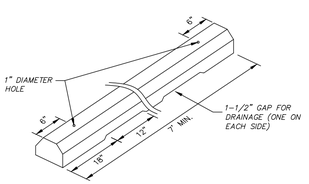
NOTES:

1. PARKING WHEEL STOPS REQUIRED WHERE NO CURB EXIST TO PROTECT SIGN.
2. ALL SIGNAGE (HEIGHT, LOCATION, COLORS, ETC.) TO MEET ADA & CITY REQUIREMENTS.

TYPICAL HANDICAPPED SIGNAGE DETAIL



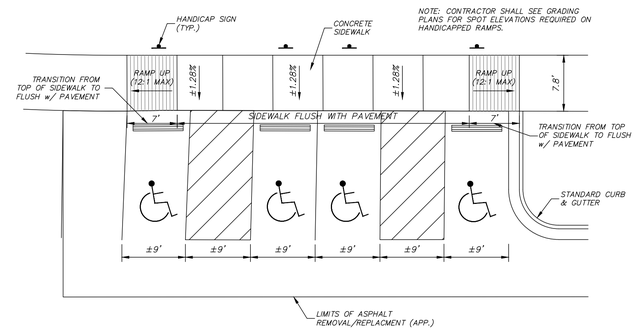
TYPICAL TRAFFIC SIGN INSTALLATION



NOTES:

1. CONCRETE SHALL BE 4,000psi AT 28 DAYS
2. WHEEL STOPS SHALL BE 3" OR 6" IN LENGTH
3. WHEEL STOPS REQUIRED WHERE SHOWN ON THE SITE PLAN
4. ADDITIONAL DESIGNS WILL BE CONSIDERED BUT MUST BE SUBMITTED FOR APPROVAL PRIOR TO BIDDING

PRECAST CONCRETE WHEEL STOP



TYPICAL HANDICAPPED PARKING & HANDICAPPED RAMP DETAILS

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

GARY ALLEN ROSS
REGISTERED PROFESSIONAL ENGINEER
No. 19035
08/01/19
STATE OF MISSISSIPPI

1 AUGUST 2019

NOT FOR CONSTRUCTION
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
	ADDENDUM 1	08/19/19

C102
TYP. SECTIONS & MISC. DETAILS

BENCHMARK
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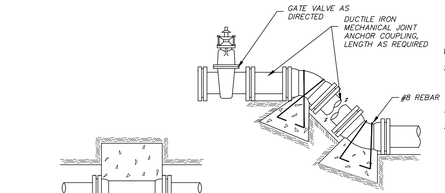
Office: 601-591-1077 Fax: 601-591-0711
E-mail: ghood@benchmarkms.net
PROJECT #: B-7258

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110



WATER LINE OBSTRUCTION NOTES:

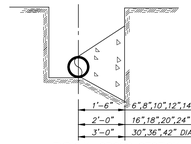
1. CONTRACTOR TO FOLLOW CLEARANCE REQUIREMENTS IN THE SPECIFICATIONS FOR WATER, STORM DRAIN AND SANITARY SEWER LINE CROSSINGS. WHEN THE WATER LINE MUST CROSS UNDER THE OBSTRUCTION THE PIPE SHALL BE DUCTILE IRON, CASED WITH STEEL CASING, OR FULLY ENCASED WITH CONCRETE.
2. WATER LINE TO PASS OVER OBSTRUCTION IF CLEARANCE REQUIREMENTS CAN BE MET.
3. CONTRACTOR TO FOLLOW REQUIREMENTS IN THE SPECIFICATIONS FOR THE PIPE FITTINGS REQUIRED TO DODGE OBSTRUCTION.

VERTICAL BENDS

TEE

PLUGGED TEE

90° BEND



TYPICAL CROSS SECTION

NOTES:

1. ON BENDS AND TEES, EXTEND THRUST BLOCKS FULL LENGTH.
2. PLACE BOARD IN FRONT OF ALL PLUGS BEFORE POURING THRUST BLOCKS.
3. POUR THRUST BLOCKS AGAINST UNDISTURBED SOIL. WHERE TRENCH WALL HAS BEEN DISTURBED, EXCAVATE LOOSE SOIL AND EXTEND THRUST BLOCKS TO UNDISTURBED SOIL.
4. IN BACK FILLING, ANY MUCK ENCOUNTERED SHALL BE REMOVED AND REPLACED WITH ACCEPTABLE MATERIAL.
5. WRAP ALL FITTINGS WITH 8-MIL POLYETHYLENE ENCASEMENT.
6. BACK FILL MATERIAL SHALL NOT INCLUDE ROCK OR BOULDERS.
7. ALL CONCRETE SHALL BE MINIMUM 2500 PSI.
8. THRUST BLOCKS SHALL BE AN ABSORBED COST.
9. MEGA LUG RESTRAINS OR APPROVED EQUAL REQUIRED ON ALL MECHANICAL JOINT FITTINGS.
10. ANCHOR COUPLINGS REQUIRED AT ALL GATE VALVES AND FITTINGS.

BEARING AREA IN SQ. FT.

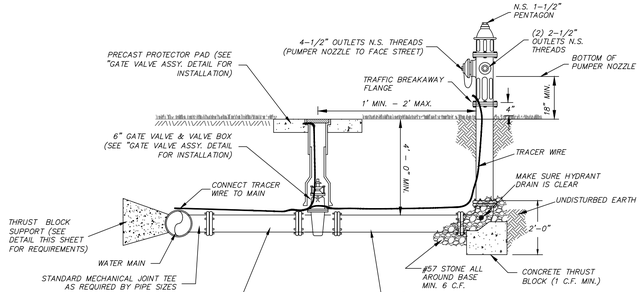
NORMAL PIPE DIAMETER (IN.)	DEAD END, TEE, PLUG	90° BEND	45° BEND	22 1/2° BEND	11 1/4° BEND
4" OR LESS	2	2	2	1	1
6	3	4	3	2	2
8	5	7	4	2	2
10	8	12	6	3	3
12	12	16	9	5	3
14	14	18	11	6	4
16	16	20	12	7	6

VERTICAL BENDS

4" OR LESS	–	–	6.0(.22)	4.0(.15)	4.0(.15)
6	–	–	14.0(.52)	6.0(.22)	4.0(.15)
8	–	–	27.0(1.0)	9.0(.33)	6.0(.22)
10	–	–	46.0(2.0)	16.0(.67)	7.5(.30)
12	–	–	68.0(2.5)	22.0(.80)	9.0(.33)
14	–	–	80.0(3.0)	40.0(1.5)	14.0(.52)
16	–	–	90.0(3.3)	52.0(1.9)	18.0(.67)

VOLUME OF BLOCKS INCLUDING SOIL LOAD CU. FT. (CU. YDS.)

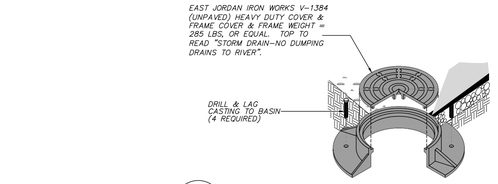
TYPICAL THRUST BLOCK

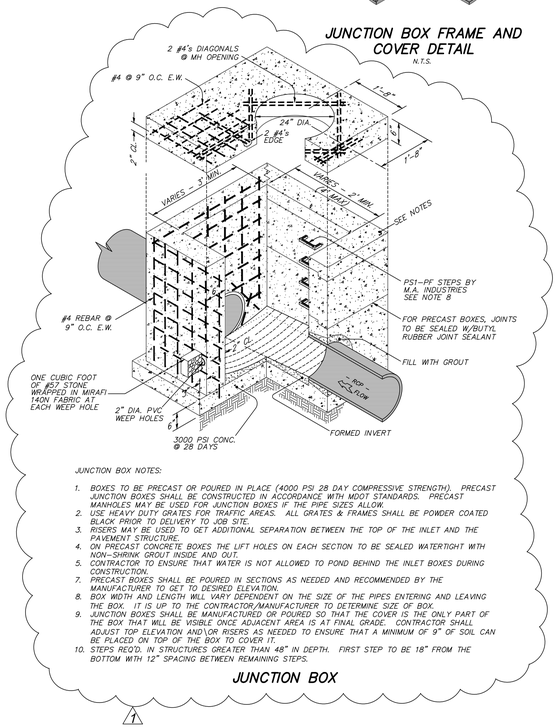


NOTES:

1. ALL FIRE HYDRANT ASSEMBLIES TO INCLUDE GATE VALVES.
2. CONTRACTOR TO USE MEGA-LUGS ON ALL RESTRAINED JOINTS.
3. FIRE HYDRANT SHALL BE MUELLER MODEL CENTURION OF THE COMPRESSION TYPE WITH A 5-1/4" VALVE OPENING. SHALL BE NOMINAL 6" SIZE, 3-WAY CONSTRUCTION w/ ONE 4-1/2" PUMPER NOZZLE AND TWO 2-1/2" HOSE CONNECTIONS.
4. SEE GATE VALVE ASSEMBLY DETAIL FOR MORE INFORMATION ON INSTALLATION.
5. ALL TRACER WIRE CONNECTORS SHALL BE 3M DBR AND SHALL BE WATERTIGHT TO PROVIDE ELECTRICAL CONDUCTIVITY.

FIRE HYDRANT ASSEMBLY





JUNCTION BOX NOTES:

1. BOXES TO BE PRECAST OR POURED IN PLACE (4000 PSI 28 DAY COMPRESSIVE STRENGTH). PRECAST JUNCTION BOXES SHALL BE CONSTRUCTED IN ACCORDANCE WITH MDOT STANDARDS. PRECAST MANHOLES MAY BE USED FOR JUNCTION BOXES IF THE PIPE SIZES ALLOW.
2. USE HEAVY DUTY GRATES FOR TRAFFIC AREAS. ALL GRATES & FRAMES SHALL BE POWDER COATED BLACK PRIOR TO DELIVERY TO JOB SITE.
3. RISERS MAY BE USED TO GET ADDITIONAL SEPARATION BETWEEN THE TOP OF THE INLET AND THE PAVEMENT STRUCTURE.
4. ON PRECAST CONCRETE BOXES THE LIFT HOLES ON EACH SECTION TO BE SEALED WATERTIGHT WITH NON-SHRINK GROUT INSIDE AND OUT.
5. CONTRACTOR TO ENSURE THAT WATER IS NOT ALLOWED TO POND BEHIND THE INLET BOXES DURING CONSTRUCTION.
7. PRECAST BOXES SHALL BE POURED IN SECTIONS AS NEEDED AND RECOMMENDED BY THE MANUFACTURER TO GET TO DESIRED ELEVATION.
8. BOX WIDTH AND LENGTH WILL VARY DEPENDENT ON THE SIZE OF THE PIPES ENTERING AND LEAVING THE BOX. IT IS UP TO THE CONTRACTOR/MANUFACTURER TO DETERMINE SIZE OF BOX.
9. JUNCTION BOXES SHALL BE MANUFACTURED OR POURED SO THAT THE COVER IS THE ONLY PART OF THE BOX THAT WILL BE VISIBLE ONCE ADJACENT AREA IS AT FINAL GRADE. CONTRACTOR SHALL ADJUST TOP ELEVATION AND\OR RISERS AS NEEDED TO ENSURE THAT A MINIMUM OF 9" OF SOIL CAN BE PLACED ON TOP OF THE BOX TO COVER IT.
10. STEPS REQ'D. IN STRUCTURES GREATER THAN 48" IN DEPTH. FIRST STEP TO BE 18" FROM THE BOTTOM WITH 12" SPACING BETWEEN REMAINING STEPS.

JUNCTION BOX

CONNECTION TO EXISTING WATER MAIN ASSEMBLY DETAILS

N.T.S.

NOTE:

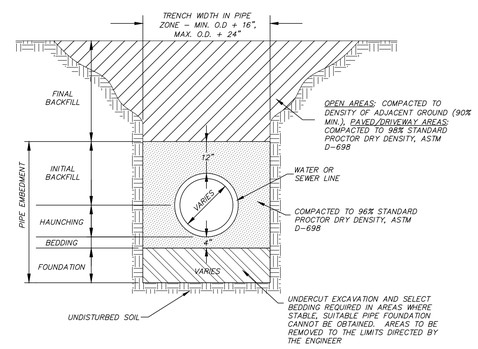
1. TRACER WIRE SHALL BE REQUIRED ON ALL WATER AND PRESSURIZED SANITARY SEWER LINES.
2. TRACER WIRE COLOR SHALL BE BLUE FOR WATER CONSTRUCTION AND GREEN FOR WASTEWATER CONSTRUCTION.
3. ALL TRACER WIRE CONNECTORS SHALL BE 3M DBR/Y6 AND SHALL BE WATERTIGHT TO PROVIDE ELECTRICAL CONDUCTIVITY.

TRACER WIRE DETAIL

NOTES:

1. GATE VALVE SHALL MATCH SIZE OF LINE ON WHICH IT IS INSTALLED UNLESS OTHERWISE NOTED.
2. VALVE BOX SHALL BE SET PLUMB AND CENTERED ABOVE THE VALVE NUT.
3. BLOCKS FOR VALVE BOX SHALL BE PLACED ON TAMPED SOIL MEETING SPECIFICATIONS FOR BEDDING MATERIAL OR ON CRUSHED LIMESTONE.
4. ALL TRACER WIRE CONNECTORS SHALL BE 3M DBR AND SHALL BE WATERTIGHT TO PROVIDE ELECTRICAL CONDUCTIVITY.
5. ALL BOLTS SHALL BE POLY WRAPPED TO REMAIN FREE OF CONCRETE AND SHALL BE FULLY ACCESSIBLE.
6. CONCRETE PROTECTOR PADS REQUIRED ON ALL VALVES INCLUDING HYDRANT VALVES.

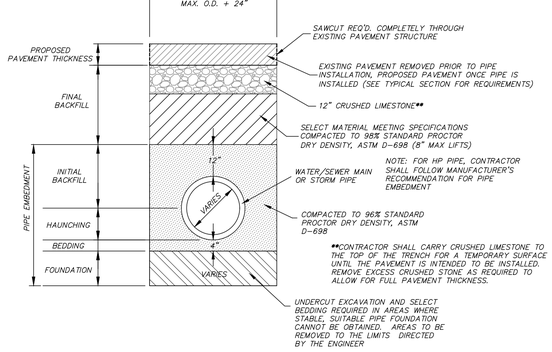
GATE VALVE ASSEMBLY



TYPICAL TRENCH NOTES:

1. PIPE EMBEDMENT MATERIALS SHALL COMPLY WITH REQUIREMENTS OF SPECIFICATIONS.
2. FINAL BACKFILL MAY BE NATIVE MATERIAL IN OPEN AREAS UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
3. FINAL BACKFILL SHALL BE SELECT MATERIAL IN ALL TRENCHES CONSTRUCTED UNDER ROADWAYS, CURBED OR PAVED AREAS. MATERIAL SHALL EXTEND 5' BEYOND THE EDGE OF PAVING STRUCTURE(S).
4. TRENCH SETTLEMENT REPAIR IS THE CONTRACTOR'S RESPONSIBILITY DURING WARRANTY PERIOD.
5. CONTRACTOR SHALL UNDERCUT TRENCH IF MATERIAL AT PLANNED GRADE WILL NOT PROVIDE STABLE TRENCH BOTTOM FOR PIPE LAYING. UNDERCUT EXCAVATION SHALL BE SPOILED ON-SITE IN A LOCATION AS DIRECTED BY THE ENGINEER.
6. DEWATERING OF ANY TRENCH IS THE RESPONSIBILITY OF THE CONTRACTOR AT NO EXTRA COST TO THE OWNER.

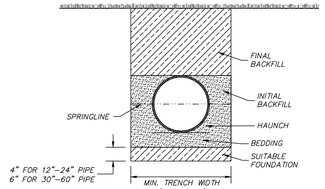
TYPICAL TRENCH DETAIL FOR WATER AND SEWER LINES



TYPICAL TRENCH NOTES:

1. PIPE EMBEDMENT MATERIALS SHALL COMPLY WITH REQUIREMENTS OF SPECIFICATIONS FOR TYPE OF PIPE.
2. TRENCH SETTLEMENT REPAIR IS THE CONTRACTOR'S RESPONSIBILITY DURING WARRANTY PERIOD.
3. CONTRACTOR SHALL UNDERCUT TRENCH IF MATERIAL AT PLANNED GRADE WILL NOT PROVIDE STABLE TRENCH BOTTOM FOR PIPE LAYING. UNDERCUT EXCAVATION SHALL BE SPOILED ON-SITE IN A LOCATION AS DIRECTED BY THE ENGINEER.
4. DEWATERING OF ANY TRENCH IS THE RESPONSIBILITY OF THE CONTRACTOR AT NO EXTRA COST TO THE OWNER.

TYPICAL UTILITY INSTALLATION UNDER EXISTING PAVED AREAS



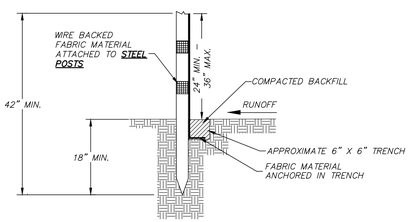
RECOMMENDED MINIMUM TRENCH WIDTHS

PIPE DIAM.	MIN. TRENCH WIDTH
12"	30"
15"	34"
18"	39"
24"	48"
30"	56"
36"	64"
42"	72"
48"	80"
60"	96"

NOTES:

1. ALL PIPE SYSTEMS SHALL BE INSTALLED IN ACCORDANCE WITH ASTM D2321, "STANDARD PRACTICE FOR UNDERGROUND INSTALLATION OF THERMOPLASTIC PIPE FOR SEWERS AND OTHER GRAVITY FLOW APPLICATIONS", LATEST ADDITION, WITH THE EXCEPTION THAT THE INITIAL BACKFILL MAY EXTEND TO THE CROWN OF THE PIPE. SOIL CLASSIFICATIONS ARE PER THE LATEST VERSION OF ASTM D2321, CLASS I\II\III MATERIALS (MH, CH) AS DEFINED IN PERVIOUS VERSIONS OF ASTM D2321 ARE NOT APPROPRIATE BACKFILL MATERIALS.
2. MEASURES SHOULD BE TAKEN TO PREVENT MIGRATION OF NATIVE FINES INTO BACKFILL MATERIAL, WHEN REQUIRED.
3. FOUNDATION: WHERE THE TRENCH BOTTOM IS UNSTABLE, THE CONTRACTOR SHALL EXCAVATE TO A DEPTH REQUIRED BY THE ENGINEER AND REPLACE WITH SUITABLE MATERIAL AS SPECIFIED BY THE ENGINEER. AS AN ALTERNATIVE AND AT THE DISCRETION OF THE DESIGN ENGINEER, THE TRENCH BOTTOM MAY BE STABILIZED USING A GEOTEXTILE MATERIAL.
4. BEDDING: SUITABLE MATERIAL SHALL BE CLASS I, II OR III. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR MATERIAL SPECIFICATION TO ENGINEER. COMPACTION SHALL BE A MINIMUM OF 95% UNLESS OTHERWISE NOTED BY THE ENGINEER, MINIMUM BEDDING THICKNESS SHALL BE 4" FOR 12"-24" DIAMETER PIPE; 6" FOR 30"-60" DIAMETER PIPE. THE MIDDLE 1/3 BENEATH THE PIPE INVERT SHALL BE LOOSELY PLACE.
5. INITIAL BACKFILL: SUITABLE MATERIAL SHALL BE CLASS I, II OR III IN THE PIPE ZONE EXTENDING TO THE CROWN OF THE PIPE. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR MATERIAL SPECIFICATION TO ENGINEER. MATERIAL SHALL BE INSTALLED AS REQUIRED IN ASTM D2321, LATEST EDITION. COMPACTION SHALL BE A MINIMUM OF 95% TO THE SPRINGLINE OF THE PIPE AND 90% TO THE CROWN OF THE PIPE.
6. FINAL BACKFILL: MINIMUM COVER, H, IN NON-TRAFFIC APPLICATIONS (GRASS OR LANDSCAPE AREA) IS 12" FROM THE TOP OF THE PIPE TO GROUND SURFACE. ADDITIONAL COVER MAY BE REQUIRED TO PREVENT FLOTATION. FOR TRAFFIC APPLICATIONS; CLASS I OR II MATERIAL COMPACTED TO 85% SPD AND CLASS III COMPACTED TO 90% SPD IS REQUIRED. FOR TRAFFIC APPLICATIONS, MINIMUM COVER, H, IS 12" UP TO 48" DIAMETER PIPE AND 24" OF COVER FOR 60" DIAMETER PIPE, MEASURED FORM TOP OF PIPE TO BOTTOM OF FLEXIBLE PAVEMENT OR TO TOP OF RIGID PAVEMENT.
7. MAXIMUM FILL HEIGHTS SHALL NOT EXCEED MANUFACTURER'S RECOMMENDATION.
8. SEE TRENCH DETAIL ON THIS SHEET FOR REQUIRED FINAL BACKFILL WHEN INSTALLING UNDER EXISTING AREAS THAT ARE PAVED.

HP STORM TRENCH INSTALLATION DETAIL



NOTES:

1. ALL SILT FENCING SHALL BE WIRE BACKED.
2. GEOTEXTILE FABRIC SHALL BE A MINIMUM OF 36" IN WIDTH.
3. STEEL POSTS SHALL BE 5' IN HEIGHT AND OF THE SELF FASTENER ANGLE STEEL TYPE SPACED AT 10' CENTERS..
4. ALLOW A 6" OVERLAP OF FABRIC AT JOINTS.

SILT FENCE DETAILS

GREGORY ALLEN HOOD — 19035 — STATE OF MISSISSIPPI — ENGINEER
08/01/19

1 AUGUST 2019

NOT FOR CONSTRUCTION
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
	ADDENDUM 1	08/19/19

C401
GENERAL CONSTRUCTION DETAILS

Room Schedule		
#	Name	Area
100	CORRIDOR	2206 SF
101	CLASSROOM	807 SF
102	CLASSROOM	797 SF
103	CLASSROOM	797 SF
104	CLASSROOM	807 SF
105	ELEC	115 SF
106	I/T	73 SF
107	JAN	77 SF
108	MENS TOILET	309 SF
109	WOMENS TOILET	307 SF
110	FR	50 SF
115	MENS LCKR	270 SF
117	WOMENS LCKR	268 SF
119	GYMNASIUM	6245 SF
120	DANCE	1816 SF
121	STG	167 SF
122	DANCE OFFICE	111 SF
123	DRAMA OFFICE	111 SF
124	DRAMA	1809 SF
125	DRAMA STG	1015 SF
126	OFFICE	172 SF
127	EQUIP STRG	673 SF

20,754 GROSS SQUARE FEET

DIMENSIONING CRITERIA

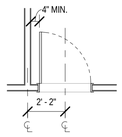
A. ALL DWGS. ARE INTENDED TO BE COMPLEMENTARY. DO NOT SCALE FOR DIM. NOTIFY THE ARCHITECT OF ANY DIM. DISCREPANCY PRIOR TO PROCEEDING.

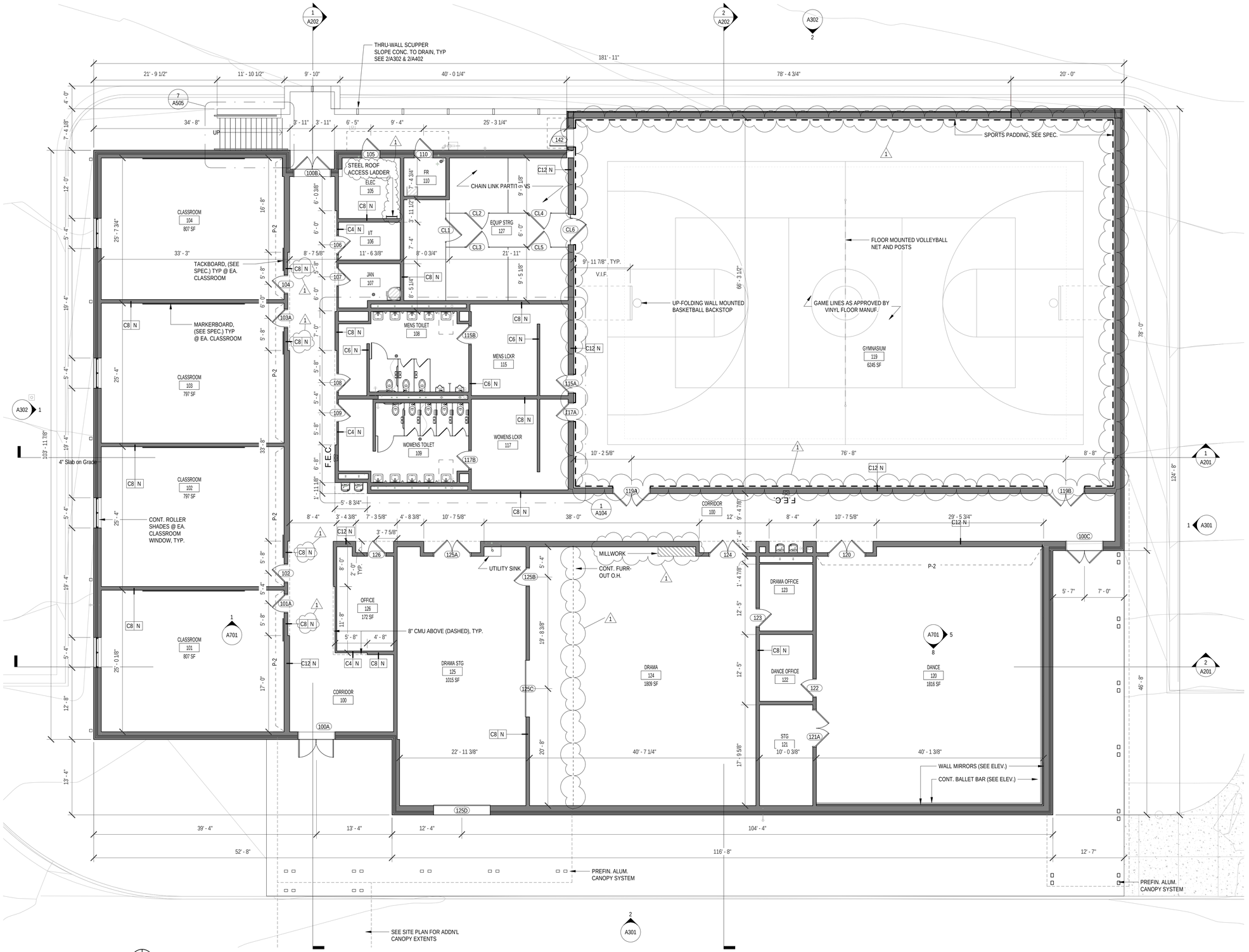
B. DIM. AT CONC. & MAS. PARTITIONS ARE TO FACE OF PARTITION (NOM. DIM.).

C. DIM. AT FRAMED PARTITIONS ARE TO FACE OF FRAMING, U.N.O.

D. DIM. ARE AS IDENTIFIED ON THE DOCUMENTS OR AS ESTABLISHED BY ADDITIONAL CRITERIA AS FOLLOWS:
1. DIM. ARE NOT SHOWN FOR THE FOLLOWING CONDITIONS:
   A. WHEN PARTITION IS CENTERED ON GRID LINE.
   B. WHEN FACE OF PARTITION IS CENTERED ON GLAZING MULLION.

2. FOR OPENINGS IN PARTITIONS OR WALLS:
   A. WHEN ONE OCCURS AT A GRIDLINE, NO DIM. WILL BE SHOWN & WIDTH WILL BE ESTABLISHED BY EITHER CRITERIA OR SCHEDULES.
   B. WHEN ONE JAMB IS LOCATED BY A PARTITION INTERSECTION, THE FOLLOWING DIAGRAM APPLIES:





1 FIRST FLOOR PLAN
1/8" = 1'-0"

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT JAMES RICHMOND WIER, JR. 4946 STATE OF MISSISSIPPI

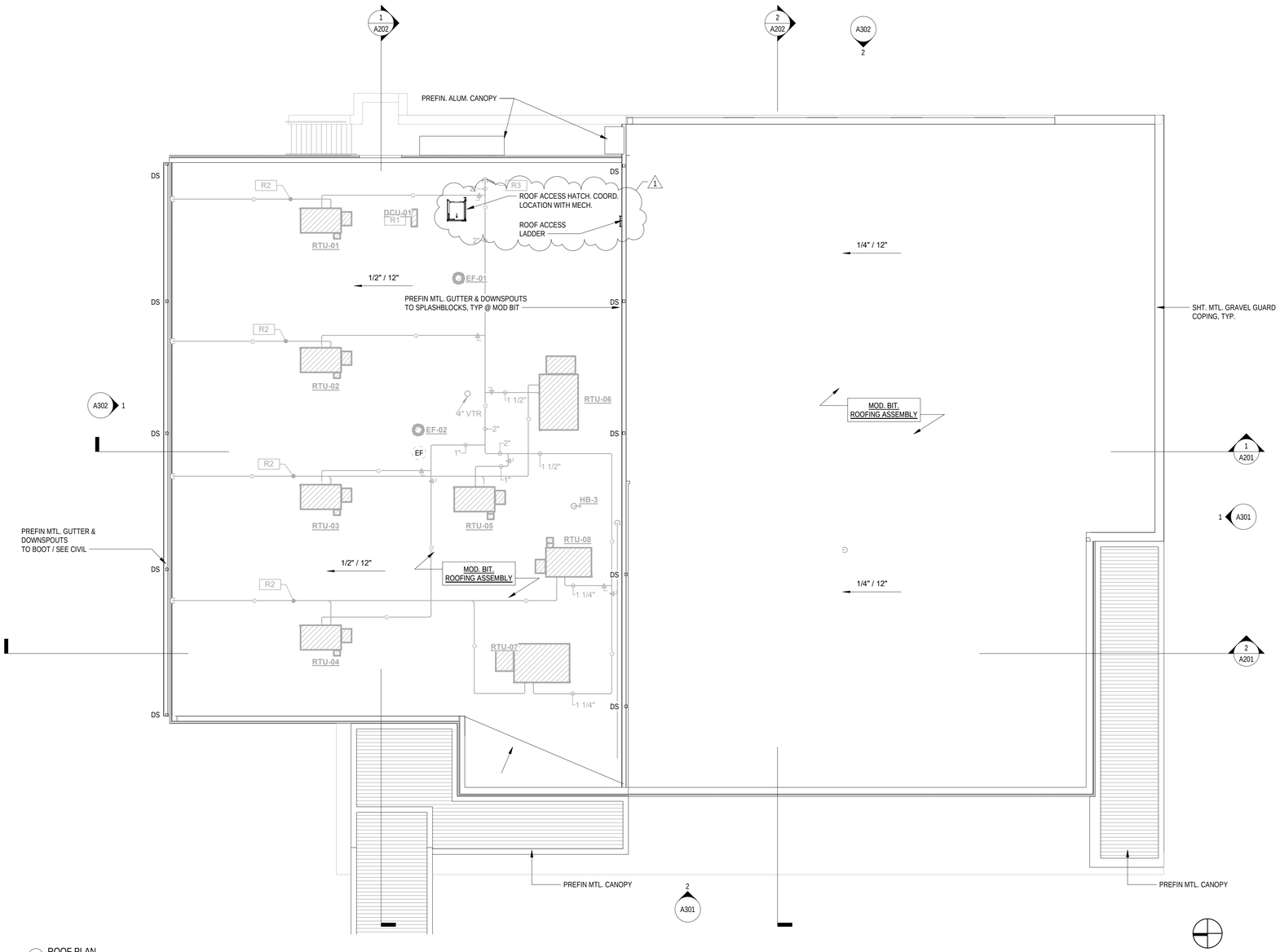
1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

A101
FIRST FLOOR PLAN



1 ROOF PLAN
3/32" = 1'-0"

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT
JAMES RICHMOND WIER, JR.
#4446
STATE OF MISSISSIPPI

1 AUGUST 2019

CONSTRUCTION DOCUMENTS
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REVISIONS

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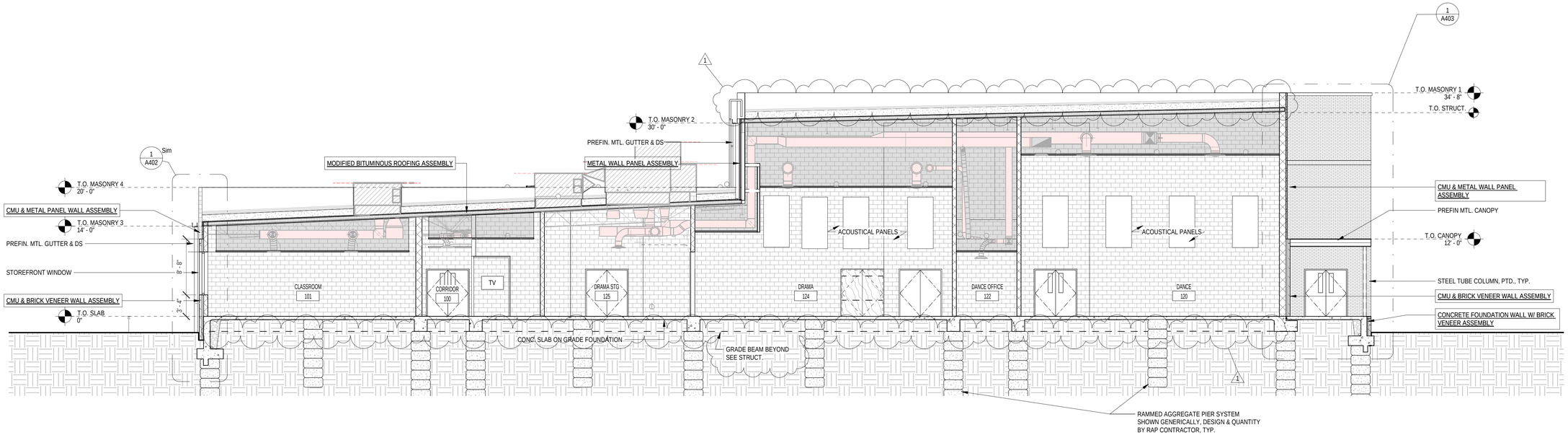
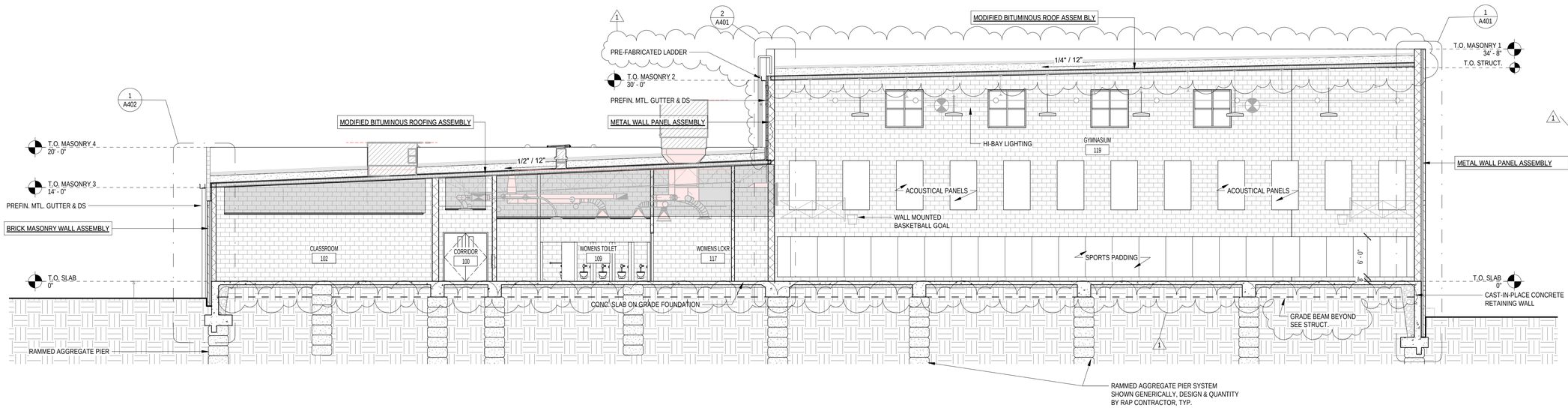
1	ADDENDUM 1	08/19/19

A103
ROOF PLAN



2 BUILDING SECTION B
1/8" = 1'-0"



1 BUILDING SECTION A
1/8" = 1'-0"

ROOF ASSEMBLIES

MODIFIED BITUMINOUS ROOFING ASSEMBLY
-2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
-COVER BOARD
-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
-METAL ROOF DECK (SEE STRUCT.)

PARAPET MEMBRANE FLASHING ASSEMBLY
(SEE SECTION DETAILS)
-FULLY ADHERED CONT. TPO MEMBRANE SYST.
-CONT. SHT. MTL. GRAVEL GUARD @ T.O. WALL
-CONT. SHT. MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERM. BAR

WALL ASSEMBLIES

CMU AND BRICK VENEER WALL ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. RIGID INSULATION (R=10)
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT. FOR ADDN'L REQ'S)

CMU AND METAL PANEL WALL ASSEMBLY
-INSULATED METAL WALL PANEL SYSTEM
-CONT. 7/8" HAT CHANNELS RUN HORIZ. @ 2' O.C. VERT.
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT.)

CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT
JAMES RICHMOND WIER, JR.
4946
STATE OF MISSISSIPPI

1 AUGUST 2019

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WBA # 0419

REVISIONS

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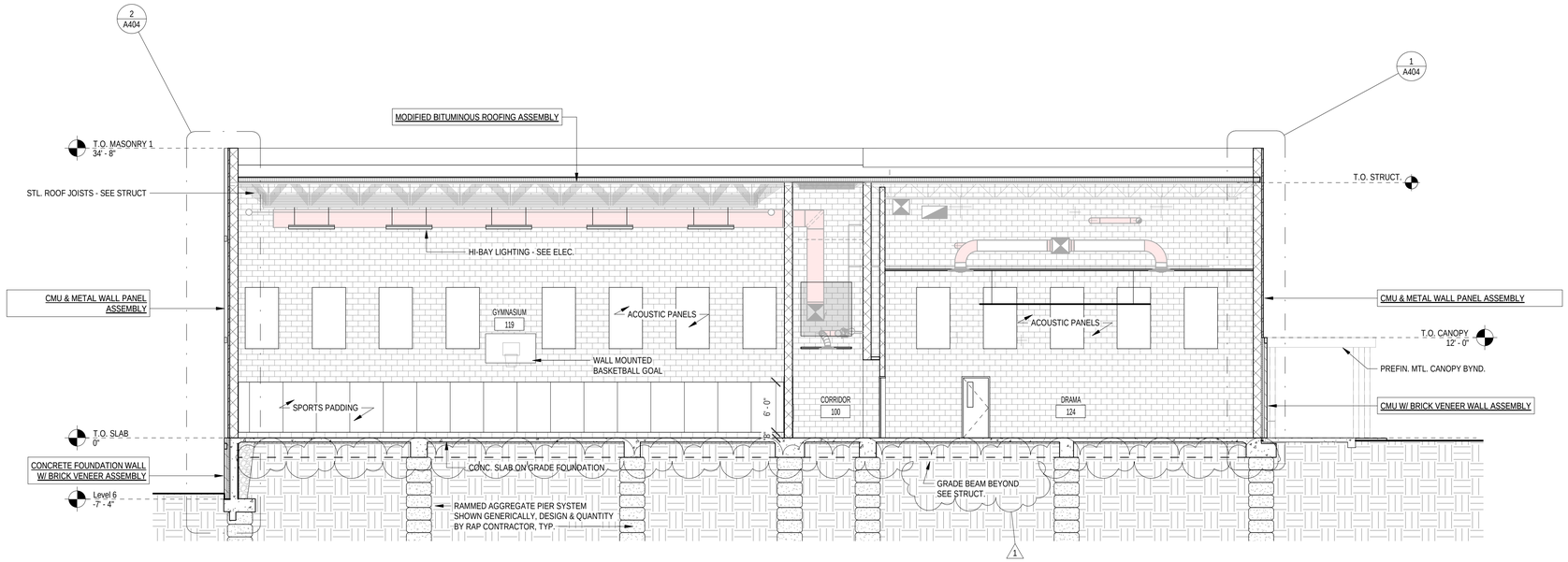
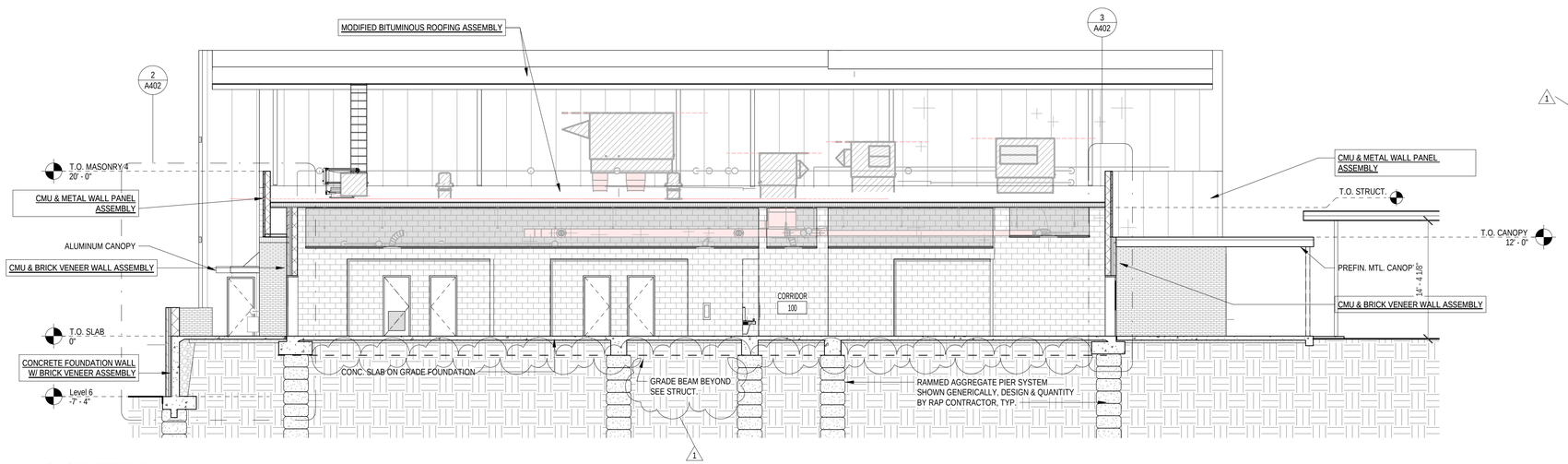
A201
BUILDING SECTIONS



2 BUILDING SECTION D
1/8" = 1'-0"



1 BUILDING SECTION C
1/8" = 1'-0"

ROOF ASSEMBLIES

MODIFIED BITUMINOUS ROOFING ASSEMBLY
-2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
-COVER BOARD
-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
-METAL ROOF DECK (SEE STRUCT.)

PARAPET MEMBRANE FLASHING ASSEMBLY
(SEE SECTION DETAILS)
-FULLY ADHERED CONT. TPO MEMBRANE SYST.
-CONT. SHT. MTL. GRAVEL GUARD @ T.O. WALL
-CONT. SHT. MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERM. BAR

WALL ASSEMBLIES

CMU AND BRICK VENEER WALL ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. RIGID INSULATION (R=10)
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT. FOR ADDN'L REQ'S)

CMU AND METAL PANEL WALL ASSEMBLY
-INSULATED METAL WALL PANEL SYSTEM
-CONT. 7/8" HAT CHANNELS RUN HORIZ. @ 2' O.C. VERT.
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT.)

CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT JAMES RICHMOND WIER, JR. 4946 STATE OF MISSISSIPPI

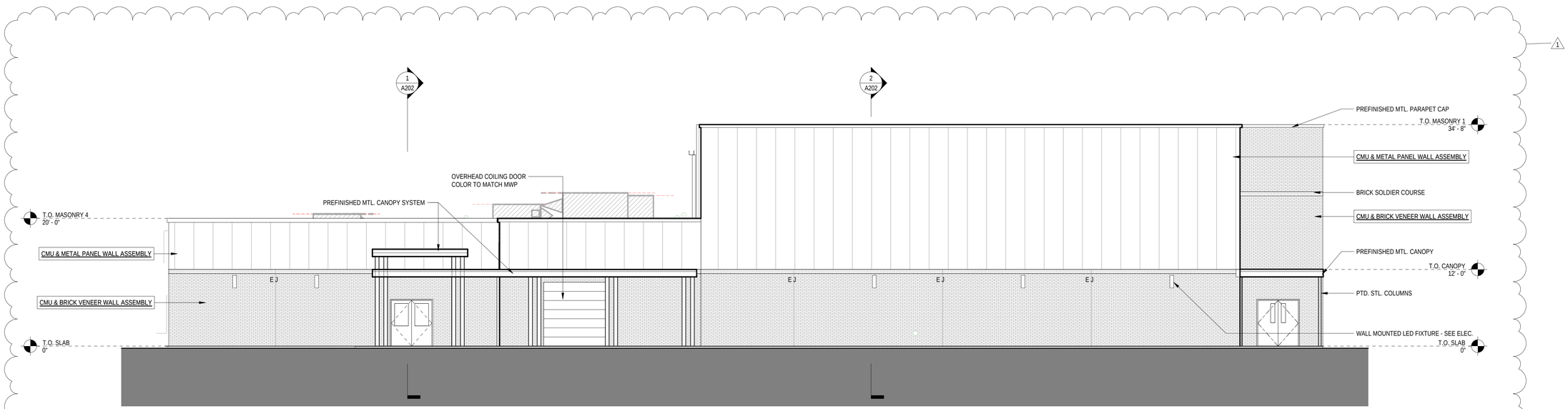
1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

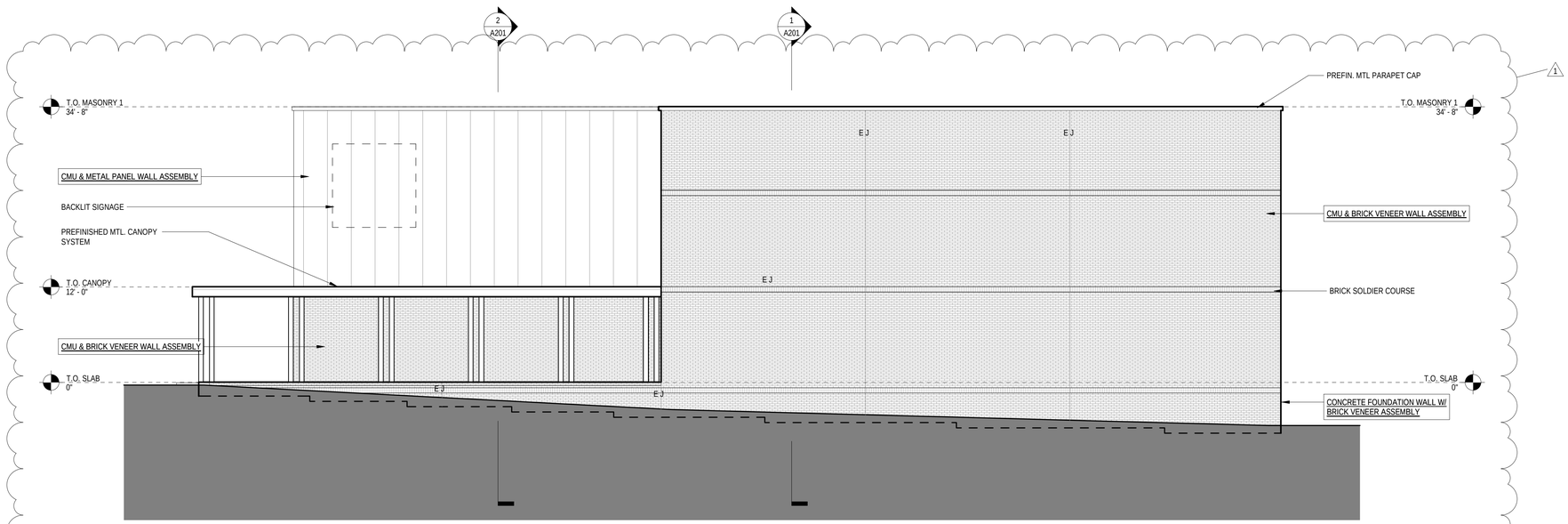
REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

A202
BUILDING SECTIONS



2 West Elevation
1/8" = 1'-0"



1 South Elevation
1/8" = 1'-0"

ROOF ASSEMBLIES

MODIFIED BITUMINOUS ROOFING ASSEMBLY
-2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
-COVER BOARD
-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
-METAL ROOF DECK (SEE STRUCT.)

PARAPET MEMBRANE FLASHING ASSEMBLY
(SEE SECTION DETAILS)
-FULLY ADHERED CONT. TPO MEMBRANE SYST.
-CONT. SHT. MTL. GRAVEL GUARD @ T.O. WALL
-CONT. SHT. MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERM. BAR

WALL ASSEMBLIES

CMU AND BRICK VENEER WALL ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. RIGID INSULATION (R=10)
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT. FOR ADDN'L REQ'S)

CMU AND METAL PANEL WALL ASSEMBLY
-INSULATED METAL WALL PANEL SYSTEM
-CONT. 7/8" HAT CHANNELS RUN HORIZ. @ 2' O.C. VERT.
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT.)

CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT
JAMES RICHMOND WIER, JR.
4946
STATE OF MISSISSIPPI

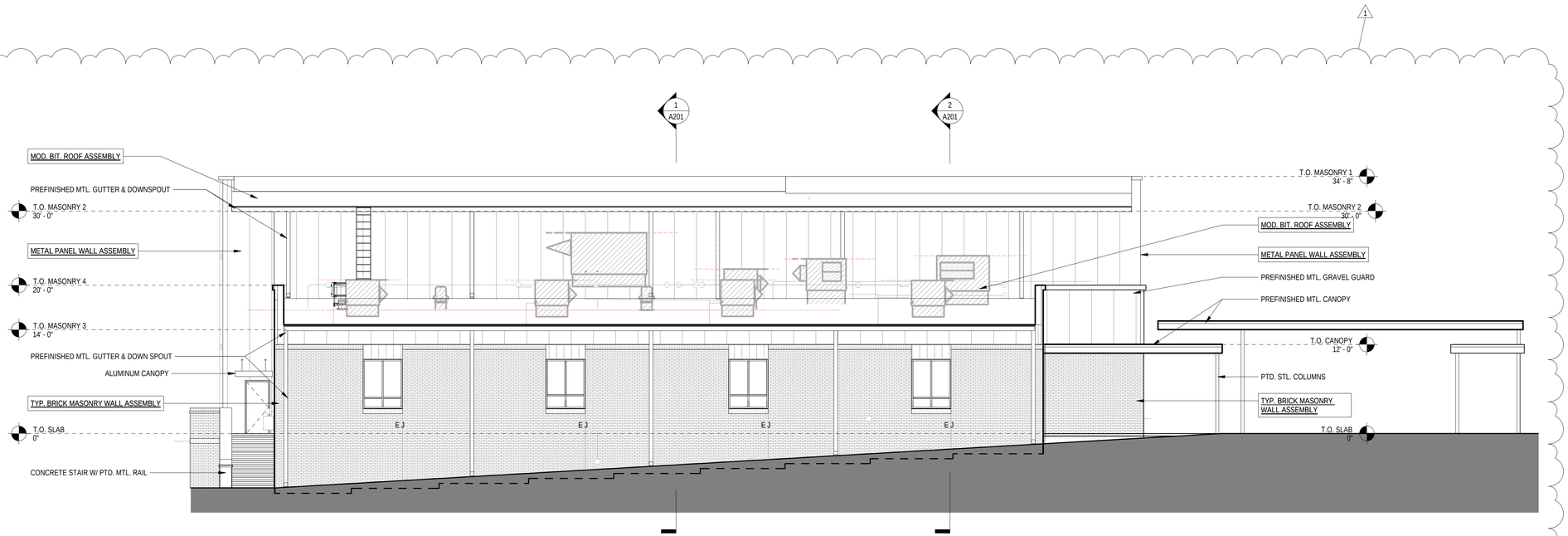
1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

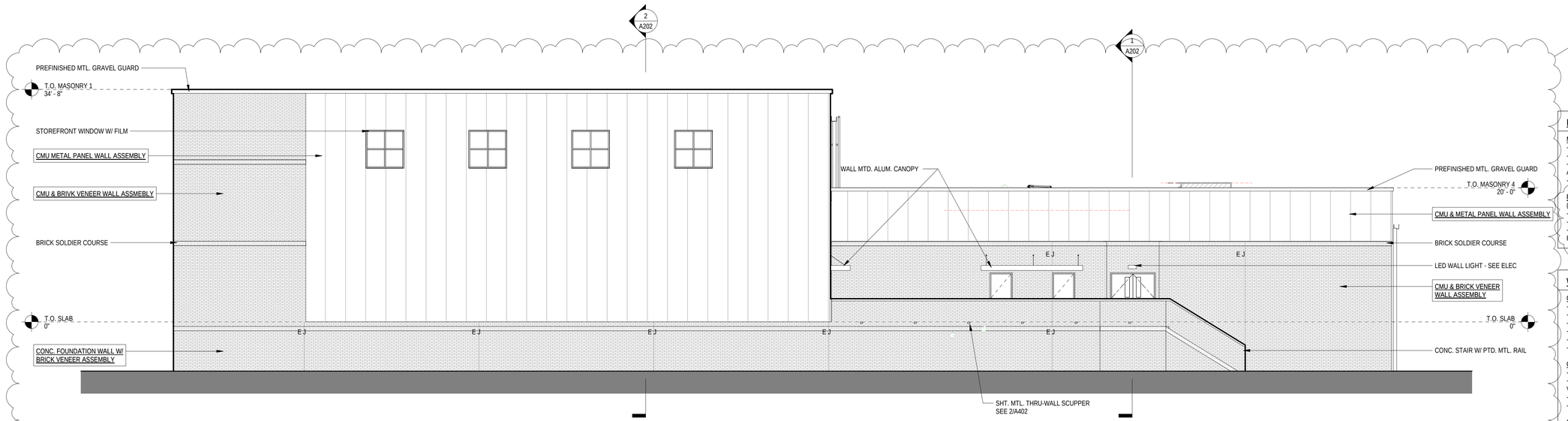
REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

A301
EXTERIOR ELEVATIONS



1 North Elevation
1/8" = 1'-0"



2 East Elevation
1/8" = 1'-0"

ROOF ASSEMBLIES

MODIFIED BITUMINOUS ROOFING ASSEMBLY
-2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
-COVER BOARD
-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
-METAL ROOF DECK (SEE STRUCT.)

PARAPET MEMBRANE FLASHING ASSEMBLY
(SEE SECTION DETAILS)
-FULLY ADHERED CONT. TPO MEMBRANE SYST.
-CONT. SHT. MTL. GRAVEL GUARD @ T.O. WALL
-CONT. SHT. MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERM. BAR

WALL ASSEMBLIES

CMU AND BRICK VENEER WALL ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. RIGID INSULATION (R=10)
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT. FOR ADDN'L REQ'S)

CMU AND METAL PANEL WALL ASSEMBLY
-INSULATED METAL WALL PANEL SYSTEM
-CONT. 7/8" HAT CHANNELS RUN HORIZ. @ 2' O.C. VERT.
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT.)

CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT
JAMES RICHMOND WIER, JR.
19446
STATE OF MISSISSIPPI

1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

A302
EXTERIOR ELEVATIONS

FLOOR ASSEMBLIES

CONCRETE SLAB-ON-GRADE ASSEMBLY
-CONCRETE SLAB (SEE STRUCT. FOR THICKNESS AND REINFORCED STL.)
-CONTINUOUS UNDER-SLAB VAPOR BARRIER
-GRANULAR FILL (SEE STRUCT.)

ROOF ASSEMBLIES

MODIFIED BITUMINOUS ROOFING ASSEMBLY
-2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
-COVER BOARD
-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
-METAL ROOF DECK (SEE STRUCT.)

PARAPET MEMBRANE FLASHING ASSEMBLY
(SEE SECTION DETAILS)
-FULLY ADHERED CONT. TPO MEMBRANE SYST.
-CONT. SHT. MTL. GRAVEL GUARD @ T.O. WALL
-CONT. SHT. MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERM. BAR

WALL ASSEMBLIES

CMU AND BRICK VENEER WALL ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. RIGID INSULATION (R=10)
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT. FOR ADDN'L REQ'S)

CMU AND METAL PANEL WALL ASSEMBLY
-INSULATED METAL WALL PANEL SYSTEM
-CONT. 7/8" HAT CHANNELS RUN HORIZ. @ 2' O.C. VERT.
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT.)

CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

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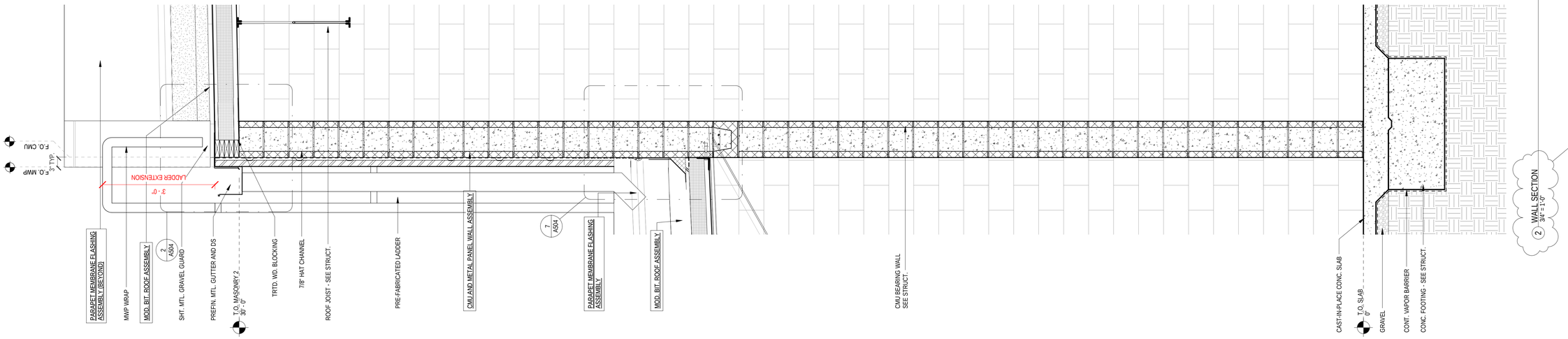
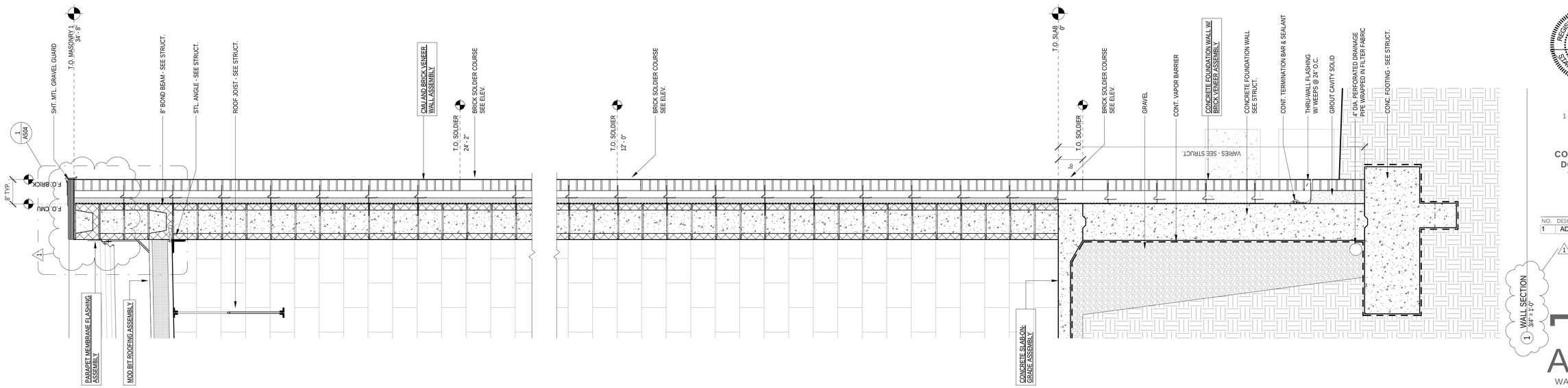
WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110





REGISTERED ARCHITECT
JAMES RICHMOND WIER, JR.
4946
STATE OF MISSISSIPPI

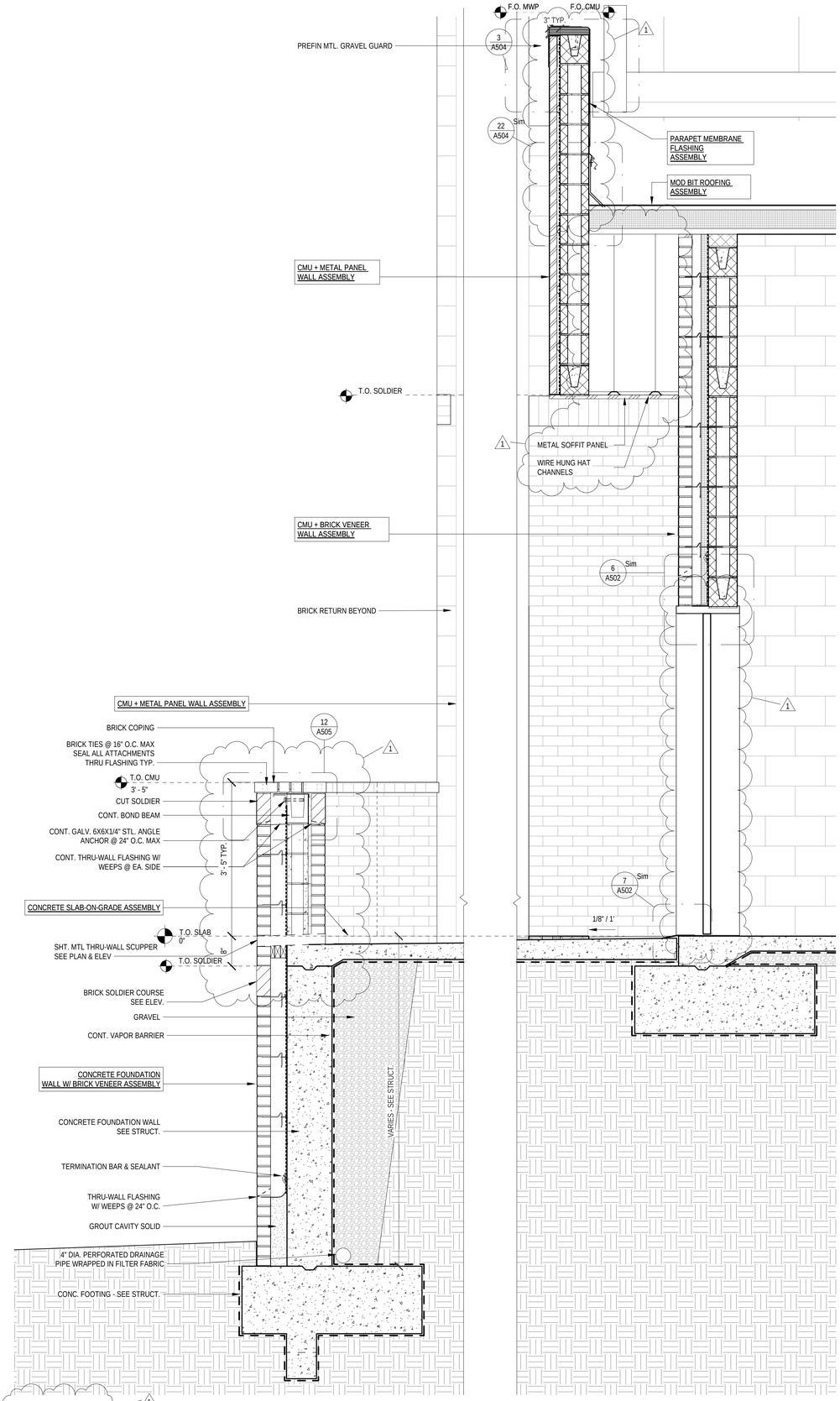
1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

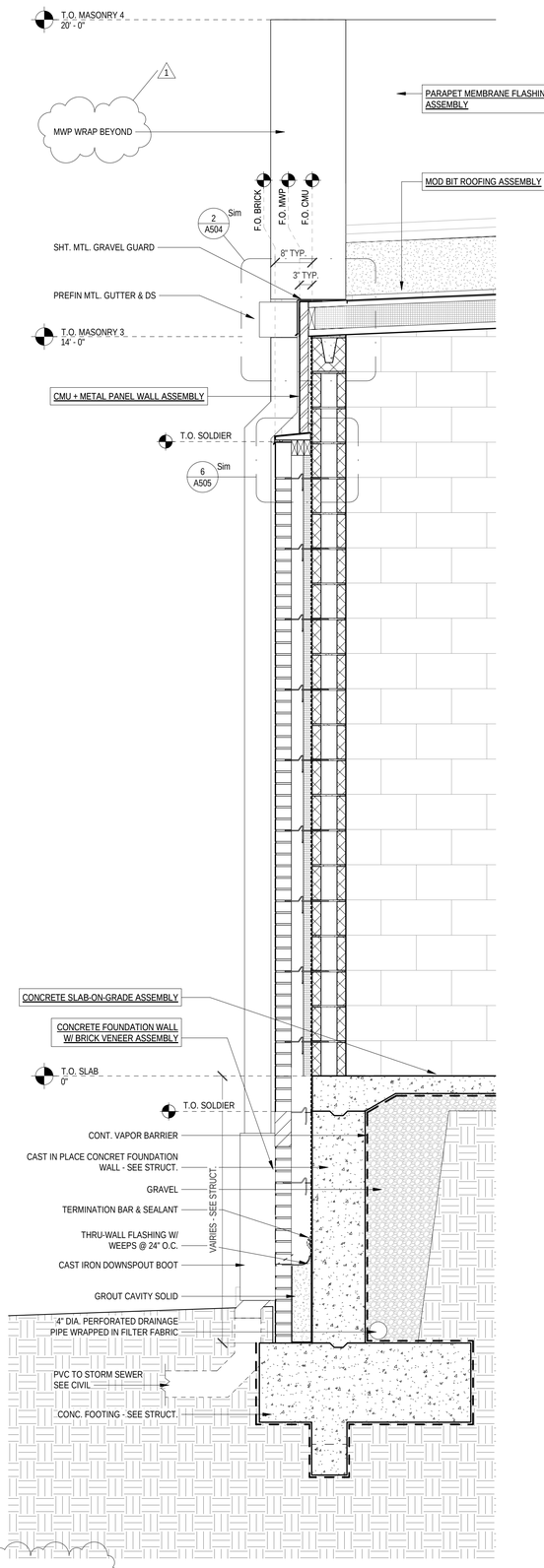
REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

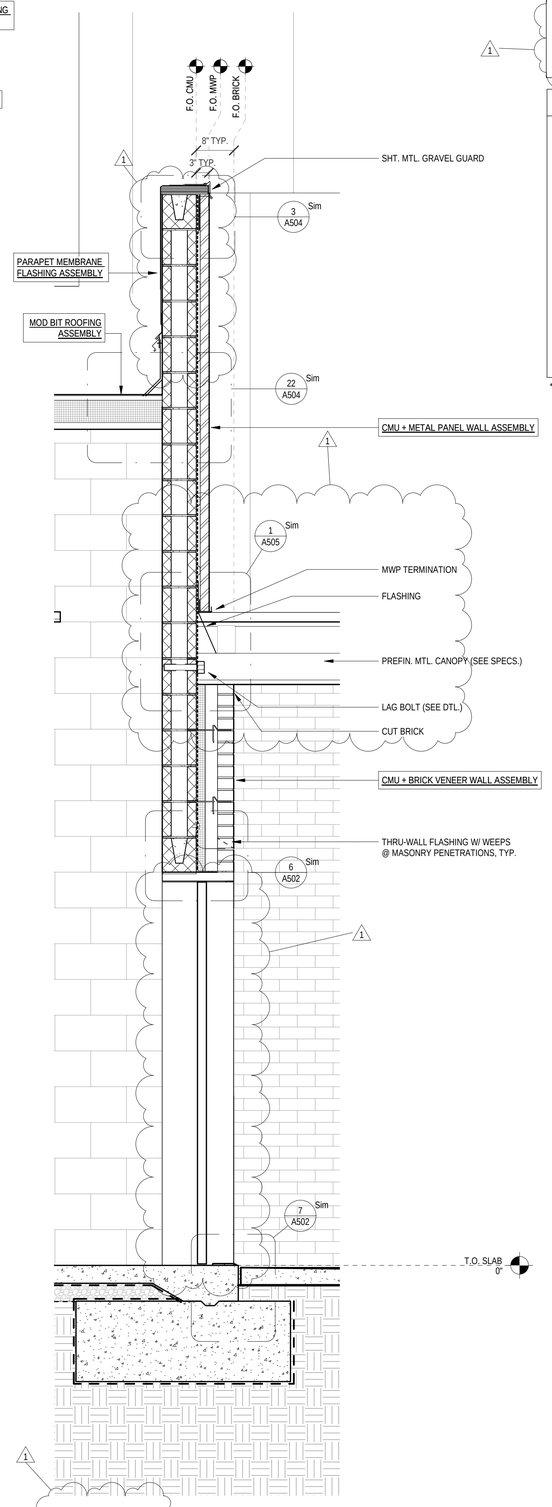
A401
WALL SECTIONS



2 WALL SECTION
3/4" = 1'-0"



1 WALL SECTION
3/4" = 1'-0"



3 WALL SECTION
3/4" = 1'-0"

FLOOR ASSEMBLIES

CONCRETE SLAB-ON-GRADE ASSEMBLY
-CONCRETE SLAB (SEE STRUCT. FOR THICKNESS AND REINFORCED STL.)
-CONTINUOUS UNDER-SLAB VAPOR BARRIER
-GRANULAR FILL (SEE STRUCT.)

ROOF ASSEMBLIES

MODIFIED BITUMINOUS ROOFING ASSEMBLY
-2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
-COVER BOARD
-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
-METAL ROOF DECK (SEE STRUCT.)

PARAPET MEMBRANE FLASHING ASSEMBLY (SEE SECTION DETAILS)
-FULLY ADHERED CONT. TPO MEMBRANE SYST.
-CONT. SHT. MTL. GRAVEL GUARD @ T.O. WALL
-CONT. SHT. MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERM. BAR

WALL ASSEMBLIES

CMU AND BRICK VENEER WALL ASSEMBLY
-BRICK MASONRY VENEER
-AIR SPACE
-MASONRY TIES @ 16" O.C.E.W.
-CONT. RIGID INSULATION (R=10)
-CONT. WEATHER BARRIER
-CMU (SEE STRUCT. FOR ADDN'L REQ'S)

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-INSULATED METAL WALL PANEL SYSTEM
-CONT. 7/8" HAT CHANNELS RUN HORIZ. @ 2' O.C. VERT.
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* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

REGISTERED ARCHITECT
JAMES RICHMOND WIER, JR.
#4946
STATE OF MISSISSIPPI

1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

A402
WALL SECTIONS

FLOOR ASSEMBLIES

CONCRETE SLAB-ON-GRADE ASSEMBLY
-CONCRETE SLAB (SEE STRUCT. FOR THICKNESS AND REINFORCED STL.)
-CONTINUOUS UNDER-SLAB VAPOR BARRIER
-GRANULAR FILL (SEE STRUCT.)

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-5" MIN. RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS/ TAPERED INSUL.)
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(SEE SECTION DETAILS)
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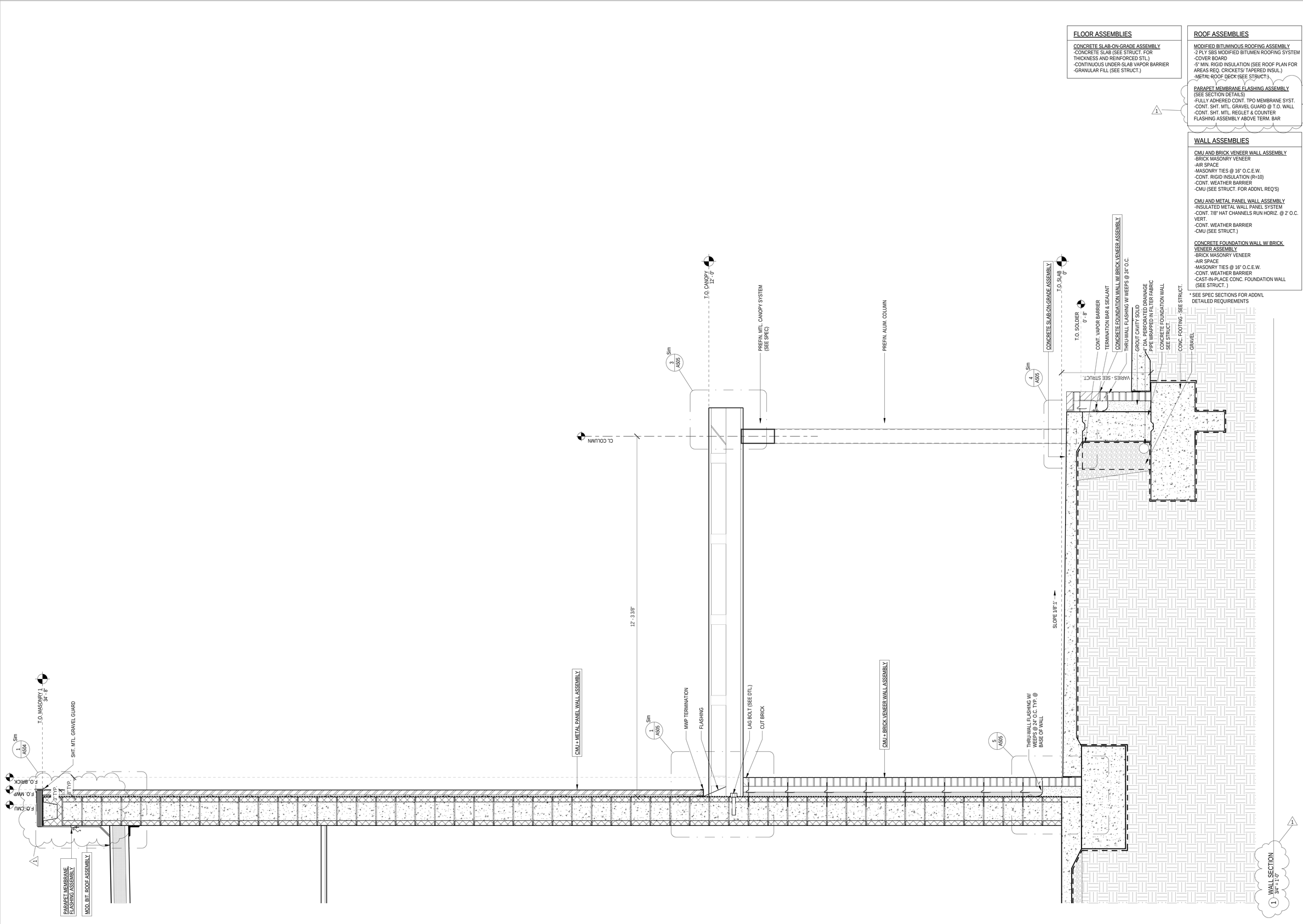
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS



WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

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#9446
STATE OF MISSISSIPPI

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CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

A403
WALL SECTIONS

FLOOR ASSEMBLIES

CONCRETE SLAB-ON-GRADE ASSEMBLY
-CONCRETE SLAB (SEE STRUCT. FOR THICKNESS AND REINFORCED STL.)
-CONTINUOUS UNDER-SLAB VAPOR BARRIER
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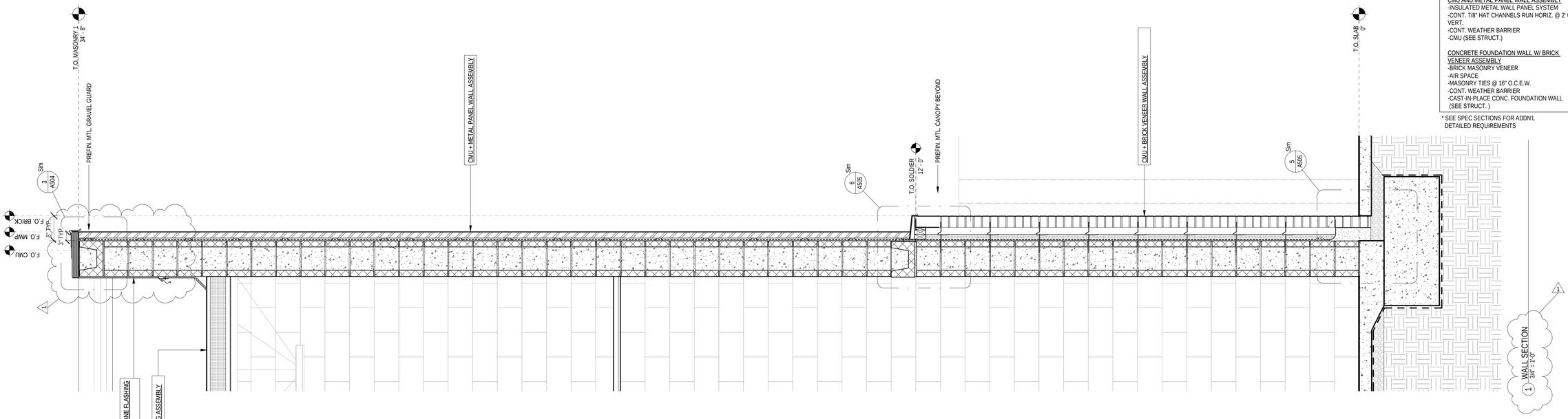
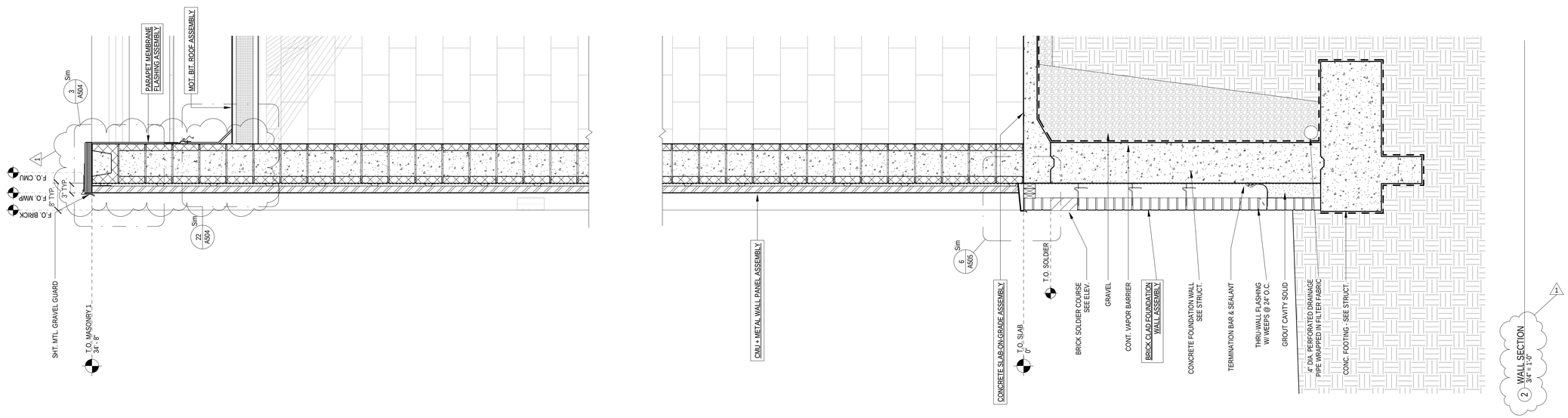
-CONT. WEATHER BARRIER
-CAST-IN-PLACE CONC. FOUNDATION WALL (SEE STRUCT.)

* SEE SPEC SECTIONS FOR ADDN'L DETAILED REQUIREMENTS



1 WALL SECTION
3/4" = 1'-0"



2 WALL SECTION
3/4" = 1'-0"

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

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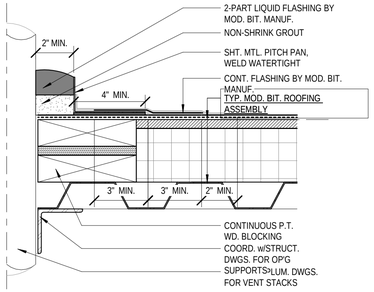
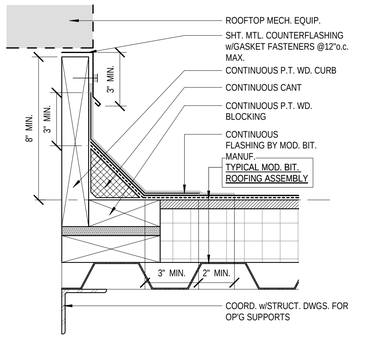
A404
WALL SECTIONS

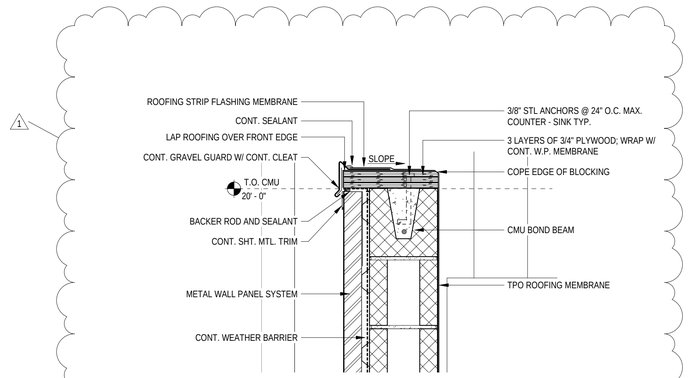


DETAIL PER FIRESTONE STYRENE-BUTADIENE-STYRENE [SBS] MODIFIED BITUMINOUS ROOFING SYSTEM

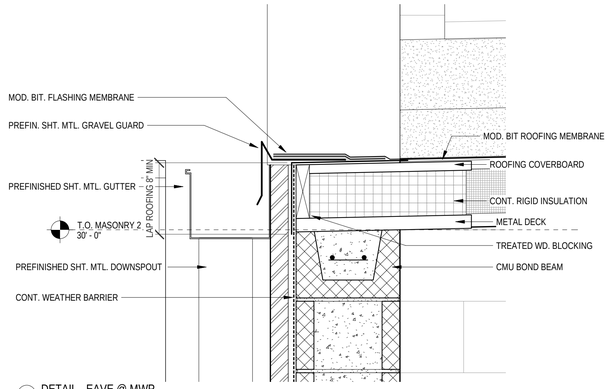
11 TYP. PENETRATION DTL
3" = 1'-0"



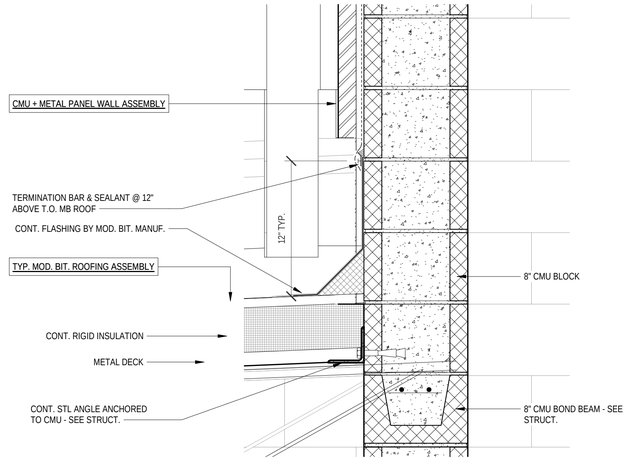
22 MOD BIT ROOF CURB DETAIL
3" = 1'-0"



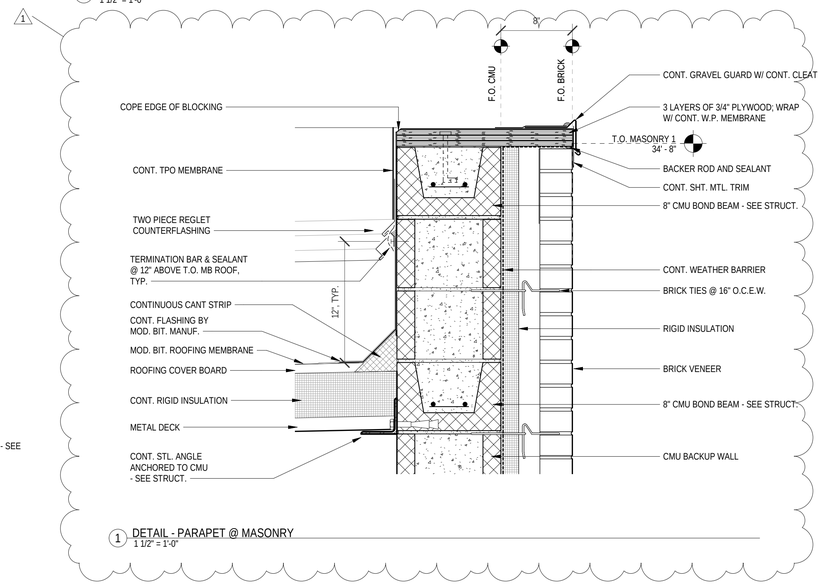
3 DETAIL - PARAPET @ MWP
1 1/2" = 1'-0"



2 DETAIL - EAVE @ MWP
1 1/2" = 1'-0"



7 DETAIL - MOD BIT AT MWP TRANSITION
1 1/2" = 1'-0"



1 DETAIL - PARAPET @ MASONRY
1 1/2" = 1'-0"

WBA ARCHITECTURE

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No. 4946
STATE OF MISSISSIPPI

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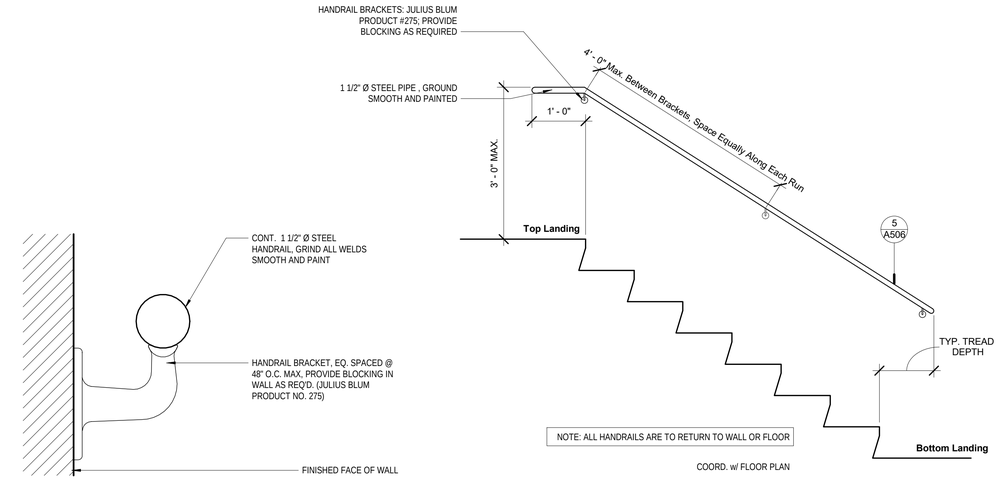
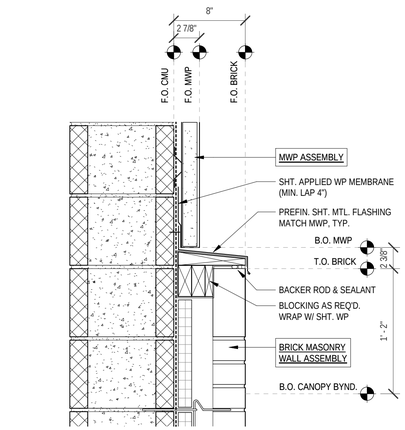
A504
ROOF DETAILS

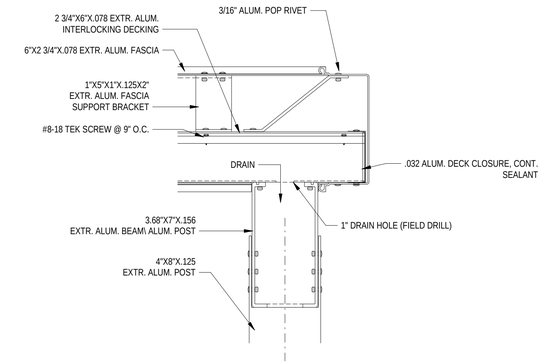


9 TYP. HANDRAIL DTL
6" = 1'-0"

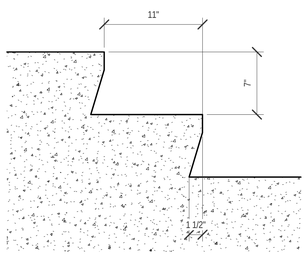
10 TYP. HANDRAIL STAIR DTL
N.T.S.



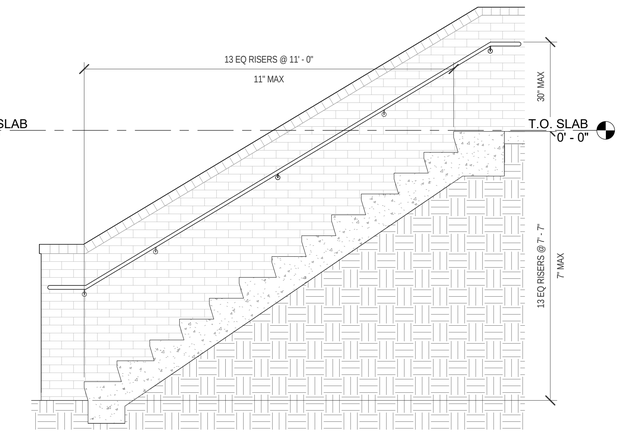
6 DETAIL - BRICK TO MWP VERT. TRANSITION
1 1/2" = 1'-0"



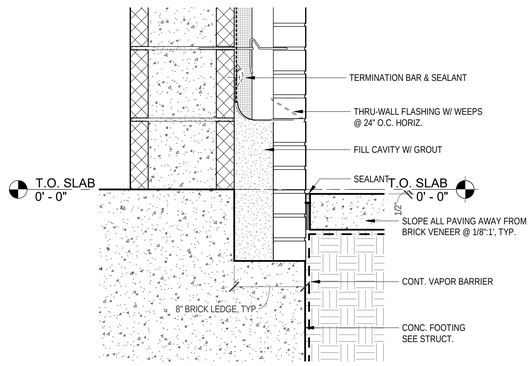
3 DETAIL - CANOPY EDGE 2
1 1/2" = 1'-0"



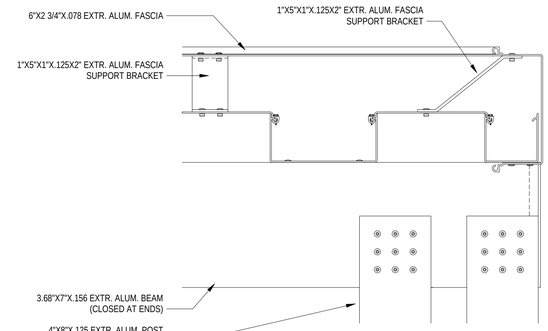
11 NOSING DETAIL
1 1/2" = 1'-0"



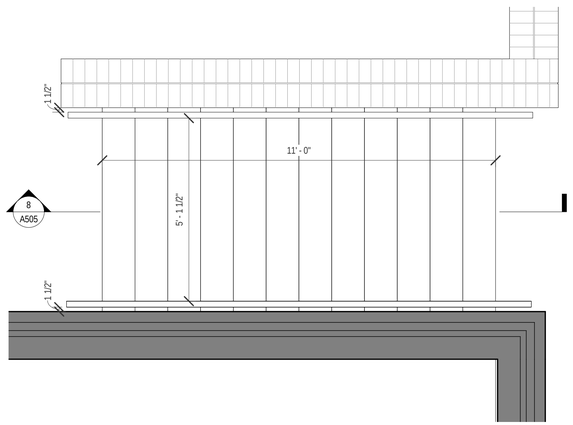
8 STAIR SECTION
1/2" = 1'-0"



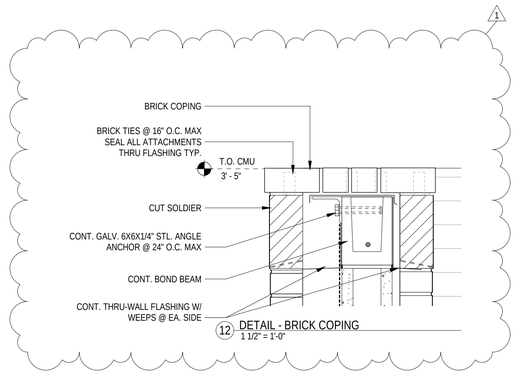
5 DETAIL - BRICK LEDGE @ PAVEMENT
1 1/2" = 1'-0"



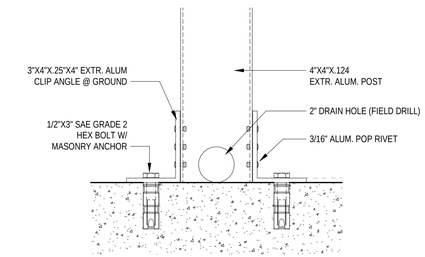
2 DETAIL - CANOPY EDGE 1
1 1/2" = 1'-0"



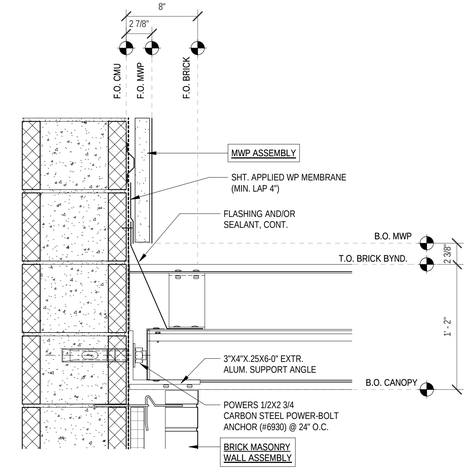
7 ENLARGED STAIR PLAN
1/2" = 1'-0"



12 DETAIL - BRICK COPING
1 1/2" = 1'-0"



4 DETAIL - CANOPY COLUMN BASE
1 1/2" = 1'-0"



1 DETAIL - CANOPY @ BRICK
1 1/2" = 1'-0"

WBA ARCHITECTURE

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CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

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A505
DETAILS

STRUCTURAL NOTES

GENERAL

1. **CODES:** ALL CONSTRUCTION SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE, 2012 EDITION AND STANDARDS REFERENCED THEREIN.
2. **SAFETY:** THE CONTRACTOR IS RESPONSIBLE FOR JOB SAFETY.
3. **COORDINATION:** THE CONTRACTOR SHALL COORDINATE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL WORKS DOCUMENTS WITH THE STRUCTURAL CONTRACT DOCUMENTS. NOTIFICATION SHALL BE MADE, IN WRITING, TO THE ARCHITECT OF ANY CONFLICT, DISCREPANCIES, OMISSIONS, AND/OR ANY VARIATIONS NEEDED IN ORDER TO COMPLY WITH CODES.
4. **SITE CONDITION VERIFICATION:** THE CONTRACTOR SHALL VERIFY EXISTING SITE CONDITIONS, DIMENSIONS, AND ELEVATIONS PRIOR TO STARTING WORK. THE ARCHITECT SHALL BE NOTIFIED, IN WRITING, OF ANY DISCREPANCIES IN EXISTING SITE CONDITIONS, DIMENSIONS, OR ELEVATIONS TO THOSE SHOWN IN THE STRUCTURAL CONSTRUCTION DOCUMENTS.
5. **GENERAL DETAILS:** CONSTRUCTION DETAILS NOT FULLY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS SHOWN FOR SIMILAR CONDITIONS.
6. **DIMENSIONS:** THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL CONSTRUCTION DOCUMENTS, SEE THE ARCHITECTURAL DRAWINGS.
7. **MEANS OF CONSTRUCTION:** THE CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
8. **MATERIALS AND TESTING:** THE CONTRACTOR SHALL BE RESPONSIBLE FOR DOCUMENTATION OF ALL MATERIAL PROPERTIES, GRADES, STRENGTHS, ETC. THE CONTRACTOR IS ALSO RESPONSIBLE FOR ALL TESTING REQUIRED TO VERIFY MATERIAL STRENGTHS. ALL TEST DATA SHALL BE DOCUMENTED AND SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL.
9. **BRACING:** THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, CONSTRUCTION, AND ERECTION OF SAFE AND ADEQUATE BRACING, SHORING, TEMPORARY SUPPORTS, ETC. REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION UNTIL FINAL SUPPORT IS SECURELY ANCHORED AND/OR CURED.
10. **SHOP DRAWINGS:** THE CONTRACTOR SHALL SUPPLY THE STRUCTURAL ENGINEER WITH CHECKED SHOP DRAWINGS BEARING THE CONTRACTOR'S STAMP OF APPROVAL AND SIGNATURE. THE REVIEW OF SHOP DRAWINGS BY THE ENGINEER IS ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THIS REVIEW DOES NOT GUARANTEE IN ANY WAY THAT THE SHOP DRAWINGS ARE CORRECT AND/OR COMPLETE, NOR DOES IT INFER THAT THEY SUPERCEDE THE STRUCTURAL CONSTRUCTION DOCUMENTS.
11. **INSPECTIONS:** ANY INSPECTIONS SPECIAL OR OTHERWISE, WHICH ARE REQUIRED BY THE BUILDING CODES, LOCAL BUILDING DEPARTMENTS, OR THESE PLANS, SHALL BE DONE BY AN INDEPENDENT INSPECTION COMPANY. JOB SITE VISITS BY THE ENGINEER DO NOT CONSTITUTE, OR SUBSTITUTE INSPECTION UNLESS SPECIFICALLY CONTRACTED FOR.
12. **ALTERATIONS:** IN NO CASE SHALL STRUCTURAL ALTERATIONS OR WORK AFFECTING A STRUCTURAL MEMBER BE MADE UNLESS PRIOR APPROVAL IS OBTAINED BY THE STRUCTURAL ENGINEER IN WRITING.

FOUNDATIONS

1. **GEOTECHNICAL REPORT:** THE FOUNDATION AND SLAB-ON-GRADE DESIGN WAS BASED ON RECOMMENDATIONS PROVIDED IN THE GEOTECHNICAL REPORT BY LANDER TESTING, INC. (JULY, 2019). BASED ON THE GEOTECHNICAL REPORT, AN ALLOWABLENET BEARING CAPACITY OF 3,000 POUNDS PER SQUARE FOOT WAS USED FOR THE FOUNDATION DESIGN. SEE THE PROJECT SPECIFICATIONS FOR A COPY OF THE GEOTECHNICAL REPORT. THE CONTRACTOR SHALL FOLLOW ALL RECOMMENDATIONS IN THE GEOTECHNICAL REPORT.
2. **SELECT FILL:** SELECT FILL MATERIAL SHALL CONSIST OF A MATERIAL HAVING A LIQUID LIMIT OF LESS THAN 45 PERCENT, A PLASTICITY INDEX BETWEEN 10 PERCENT AND 25 PERCENT, AND GREATER THAN 10 PERCENT PASSING THE NO. 200 SIEVE. SELECT FILL SHALL BE PLACED IN MAXIMUM LOOSE LIFTS OF 9 INCHES AND COMPACTED TO 98 PERCENT STANDARD PROCTOR DENSITY (ASTM D 698) WITHIN 3 PERCENT OF OPTIMUM MOISTURE CONTENT. DENSITY TESTS SHALL BE CONDUCTED FOR EACH LIFT AT A RATE OF ONE DENSITY TESTS PER 3,000 SQUARE FEET OF SURFACE AREA PER LIFT WITH A MINIMUM OF TWO TESTS PER LIFT PER BUILDING AREA.
3. **QUESTIONABLE SOILS:** SHOULD THE CONTRACTOR ENCOUNTER UNUSUAL SOILS OR QUESTIONABLE SUBSURFACE CONDITIONS, THE STRUCTURAL ENGINEER SHALL BE CONTACTED IMMEDIATELY.
4. **AGGREGATE PIERS:** AGGREGATE PIERS SHALL BE USED TO IMPROVE THE SITE TO ACHIEVE AN ALLOWABLE BEARING CAPACITY OF 3,000 PSF. SEE THE PROJECT SPECIFICATIONS. 1

STRUCTURAL DESIGN DATA

1. **LIVE LOADS:**
   ROOF LIVE LOAD = 20 PSF
2. **SUPERIMPOSED DEAD LOADS:**
   ROOF DEAD LOAD = 20 PSF
3. **SNOW LOADS:**
   GROUND SNOW LOAD = 5 PSF
   SNOW IMPORTANCE FACTOR = 1.10
4. **SEISMIC DESIGN DATA:**

SEISMIC DESIGN DATA

- RISK CATEGORY = III
- IMPORTANCE FACTOR = 1.25
- SITE CLASS = D
- MAPPED SPECTRAL RESPONSE ACCELERATION PARAMETERS:
  S_S = 0.138
  S_1 = 0.082
- DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS:
  S_{DS}= 0.147
  S_{D1}= 0.131
- SEISMIC DESIGN CATEGORY = B 1
- BASIC SEISMIC-FORCE-RESISTING SYSTEMS:
  ORDINARY REINFORCED MASONRY SHEAR WALLS (R=2)
- ANALYSIS PROCEDURE:
  EQUIVALENT LATERAL FORCE PROCEDURE

5. **WIND DESIGN DATA:**

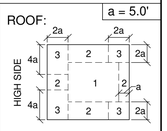
WIND DESIGN DATA

1. BASIC WIND SPEED = 120 MPH (ULTIMATE)
2. RISK CATEGORY = III
3. WIND EXPOSURE CATEGORY = B
4. INTERNAL PRESSURE COEFFICIENT = 0.18 +/-
5. ENCLOSED STRUCTURE

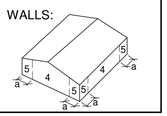
COMPONENTS AND CLADDING LOADS

	AREA (SF)	PRESSURE (PSF) (ASD) MAXIMUM	MINIMUM
ROOF ZONE 1	0-10	9.60	-16.86
	30	9.60	-16.86
	100	9.60	-16.86
ROOF ZONE 2	0-10	9.60	-23.45
	30	9.60	-22.82
	100	9.60	-22.13
ROOF ZONE 3	0-10	9.60	-36.62
	30	9.60	-30.33
	100	9.60	-23.45
WALL ZONE 4	0-10	14.23	-15.41
	50	12.76	-13.95
	200	11.50	-12.69
WALL ZONE 5	0-10	14.23	-18.97
	50	12.76	-16.04
	200	11.50	-13.52

ROOF:



WALLS:



LIGHT GAUGE STEEL

1. **STUDS:** SEE PLANS FOR STUD SIZES.
2. **CONNECTORS:** LIGHT GAUGE STEEL CONNECTORS SHALL BE BY THE STEEL NETWORK, CLARKDIETRICH, OR SIMPSON STRONG TIE. SUBSTITUTES MUST BE APPROVED PRIOR TO CONSTRUCTION BY THE STRUCTURAL ENGINEER.
3. **FASTENERS:** ALL SCREWS SHALL BE IN CONFORMANCE WITH THE REQUIREMENTS OF THE AMERICAN IRON AND STEEL INSTITUTE, NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, 2007 EDITION.
4. **FRAMING MEMBERS:** ALL LIGHT GAUGE FRAMING MEMBERS SHALL HAVE SECTION PROPERTIES IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE, NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS, 2007 EDITION.

CONCRETE MASONRY

1. **CODE:** MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES (ACI 530.1 / ASCE6 / TMS 602)," PUBLISHED BY THE MASONRY SOCIETY, BOULDER, COLORADO; THE AMERICAN CONCRETE INSTITUTE, FARMINGTON HILLS, MICHIGAN; AND THE AMERICAN SOCIETY OF CIVIL ENGINEERS, RESTON, VIRGINIA.
2. **CONCRETE MASONRY UNITS:** CONCRETE MASONRY UNITS SHALL BE NORMAL WEIGHT OR LIGHTWEIGHT HOLLOW-CORE (ASTM C 331), GRADE N, TYPE II, AND COMPLY WITH ASTM C 90. CONCRETE MASONRY UNITS SHALL BE CONSTRUCTED IN RUNNING BOND.
3. **STRENGTH:** CONCRETE MASONRY UNITS SHALL SHAVE A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1,900 PSI.
4. **GROUT:** GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI AT 28 DAYS AND CONFORM TO ASTM C 476.
5. **CONTROL JOINTS:** VERTICAL CONTROL JOINTS SHALL BE PROVIDED IN CMU WALLS AS SHOWN ON THE CMU WALL PLAN.

CONCRETE & REINFORCING STEEL

1. **STRENGTH:** CONCRETE SHALL HAVE A NORMAL WEIGHT OF 145 PCF AND HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF:

FOUNDATIONS & SLAB-ON-GRADE	3,500 PSI
CONCRETE FOUNDATION WALLS	4,000 PSI
CONCRETE BEAMS	4,000 PSI

2. **SLUMP:** THE CONCRETE SLUMP SHALL BE 4" AT THE POINT OF PLACEMENT. THE USE OF WATER-REDUCING ADMIXTURES MAY BE USED IF AN INCREASED SLUMP IS DESIRED FOR WORKABILITY. THE SLUMP SHALL NOT EXCEED 5" WITH THE USE OF NORMAL WATER-REDUCERS & SHALL NOT EXCEED 7" WITH THE USE OF MID-RANGE OR HIGH-RANGE WATER-REDUCERS.
3. **DOCUMENTATION:** THE CONCRETE MIX DESIGN, STRENGTH, AND ALL SPECIFIED TESTING SHALL BE FULLY DOCUMENTED AND SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL.
4. **CURING:** CONCRETE SLABS SHALL BE CURED WITH A SPRAY-ON CURING COMPOUND APPLIED IMMEDIATLY AFTER FINISHING IS COMPLETED, OR BY FULLY COVERING WITH PLASTIC SHEETING FOR 7 DAYS. THE PLASTIC SHEETING SEAMS SHALL BE TAPED. THE CONTRACTOR IS RESPONSIBLE FOR VERIFYING THE CURING COMPOUND IS COMPATIBLE WITH ALL FLOOR FINISHES.
5. **REINFORCEMENT PLACEMENT:** ALL REINFORCEMENT SHALL BE BENT COLD. REINFORCEMENT SHALL NOT BE WELDED. REINFORCEMENT SHALL NOT BE WET SET IN CONCRETE.
6. **INSERTS:** ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCING, DOWELS, BOLTS, ANCHORS, PIPES, SLEEVES, ETC. SHALL BE SECURELY POSITIONED IN THE FORMS BEFORE PLACING CONCRETE.
7. **SECURING REINFORCEMENT:** ALL REINFORCING STEEL & EMBEDMENTS SHALL BE SECURELY TIED AND SUFFICIENTLY SUPPORTED TO MAINTAIN THE POSITION WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES.
8. **WEATHER:** WEATHER CONDITIONS SHALL NOT BE ACCEPTABLE AS A VALID REASON FOR INCORRECT OR OTHERWISE POOR QUALITY OF CONCRETE OR CONCRETE SURFACES.
9. **REINFORCEMENT COVER:** MINIMUM REINFORCEMENT COVER SHALL BE AS SHOWN IN THIS TABLE:

CONCRETE REINFORCEMENT COVER

EXPOSURE CONDITION	MINIMUM COVER	TOLERANCE (-)
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"	3/8"
EXPOSED TO EARTH OR WEATHER:		
No. 5 AND SMALLER BARS	1½"	1/4"
No. 6 AND LARGER BARS	2"	1/4"
NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND:		
STRUCTURAL SLABS & WALLS	3/4"	1/8"
BEAMS AND COLUMNS	1½"	1/4"
SLABS ON GRADE:	2"	1/4"

10. **SPLICING REINFORCEMENT:** CONTINUOUS REINFORCEMENT SHALL BE PROVIDED WHEREVER POSSIBLE. REINFORCEMENT SHALL BE LAP SPLICED WHEN REQUIRED AS FOLLOWS:

REINFORCEMENT LAP SLICE LENGTHS

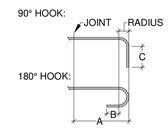
BAR SIZE	LAP SPLICE LENGTH
#3	22"
#4	29"
#5	36"
#6	43"
#7	63"
#8	72"
#9	81"

11. **REINFORCEMENT STANDARD HOOKS:** REINFORCEMENT SHALL BE HOOKED AS SHOWN IN THE TABLE BELOW WHEN SPECIFIED IN THE DRAWINGS:

REINFORCEMENT STANDARD HOOKS

BAR SIZE	RADIUS	A	B	C
#3	1.5"	8.5"	2.5"	4.5"
#4	2"	11"	2.5"	6"
#5	2.5"	14"	2.5"	7.5"
#6	3"	16.5"	3"	9"
#7	3.5"	19.5"	3.5"	10.5"
#8	4"	22"	4"	12"
#9	5.625"	25"	4.5"	13.5"

90° HOOK: 180° HOOK:



STRUCTURAL STEEL

1. **CODE:** STEEL CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE TOLERANCES, QUALITY, FABRICATION, AND ERECTION AS SET FORTH IN CHAPTER 22 OF THE INTERNATIONAL BUILDING CODE, 2012 AND THE LATEST AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) SPECIFICATION.
2. **MATERIAL:** STRUCTURAL STEEL PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36. WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B (Fy=46KSI). STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A53, GRADE B (Fy=35KSI).
3. **SHOP DRAWINGS:** THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING ALL MEMBERS, SIZES, LOCATIONS, AND CONNECTIONS TO THE ARCHITECT/STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION.
4. **FOUNDATION ANCHOR RODS:** ALL FOUNDATION ANCHOR RODS SHALL BE ASTM F1554, GRADE 55, AND SHALL BE WELDABLE. ALL ANCHOR RODS SHALL HAVE TWO HEAVY HEX HEAD NUTS OR HAVE A SINGLE HEAVY HEX NUT TACK WELDED TO THE SHAFT ON THE END THAT IS ANCHORED IN CONCRETE. ALTERNATIVELY, THE ANCHOR RODS MAY HAVE A HEAVY HEX HEAD FACTORY FORGED TO THE SHAFT.
5. **FOUNDATION ANCHOR ROD HOLES:** ANCHOR ROD HOLES IN BASE PLATES MAY BE OF THE MAXIMUM SIZE AS SPECIFIED IN TABLE 14-2 OF THE AISC MANUAL. PLATE WASHERS OF THE SPECIFIED MINIMUM SIZE SHALL BE USED WITH OVERSIZED HOLES.
6. **GROUTING:** ALL BASE PLATES SHALL BE FULLY GROUTED WITH A NON-SHRINK GROUT THAT MEETS OF EXCEEDS ASTM C 1107, GRADES B AND C.
7. **WELDING:** ALL WELDING SHALL BE IN ACCORDANCE WITH THE AISC AND EXECUTED BY WELDERS QUALIFIED IN ACCORDANCE WITH THE REQUIREMENTS OF AWS D1.1. ALL WELDS SHALL BE MADE WITH E70XX ELECTRODES. ALL WELDED JOINTS SHALL BE PREQUALIFIED BY AISC.

STEEL JOISTS

1. **STANDARD:** STEEL JOISTS, BRIDGING, CONNECTIONS, AND ALL ACCESSORIES SHALL CONFORM TO THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI).
2. **DESIGN:** THE STEEL JOIST MANUFACTURER SHALL PROVIDE THE DESIGN OF THE STEEL JOISTS AND ALL NECESSARY ACCESSORIES BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MISSISSIPPI. SEE THE PROJECT SPECIFICATIONS.
3. **WIND UPLIFT LOADS:** STEEL JOISTS SHALL BE DESIGNED FOR A NET UPLIFT OF 15 PSF (ASD).

STEEL DECK

1. **STANDARD:** STEEL DECK AND ACCESSORIES SHALL CONFORM TO THE SPECIFICATIONS OF THE STEEL DECK INSTITUTE (SDI).
2. **FINISH:** ALL STEEL DECK SHALL BE GALVANIZED (G90). SEE SPECIFICATIONS.
3. **FASTENING:** STEEL ROOF DECK SHALL BE FASTENED TO THE SUPPORTING STRUCTURE AS FOLLOWS:

   1.5B, 20 GAUGE
   PERIMETER FASTENERS:
   #12 SELF-DRILLING SCREWS OR 5/8" PUDDLE WELDS AT 6" O.C.
   SUPPORT FASTENERS:
   #12 SELF-DRILLING SCREWS OR 5/8" PUDDLE WELDS IN A 36/7 PATTERN
   SIDELAP FASTENERS:
   #10 SELF-DRILLING SCREWS - 7 PER SPAN

BRICK VENEER

1. **BRICK TIES:** BRICK VENEER SHALL BE ANCHORED TO THE WALL WITH BRICK TIES AT A MAXIMUM SPACING OF 16" VERTICAL AND HORIZONTAL. SEE SPECIFICATIONS FOR BRICK TIES.
2. **LOOSE LINTELS:** L6x6x3/8 LOOSE LINTELS SHALL BE INSTALLED ABOVE ALL OPENINGS NOT EXCEEDING A CLEAR OPENING WIDTH OF 7' - 0" (SEE ARCHITECTURAL). ANCHOR LINTEL TO CMU WITH 5/8"x5" KWIK BOLT 3 WEDGE ANCHORS AT 32" O.C. FOR OPENINGS 7 FEET OR MORE.
3. **CONTROL JOINTS:** INSTALL CONTROL JOINTS IN BRICK VENEER AS SHOWN IN THE ARCHITECTURAL DRAWINGS.

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

JASON M. MCQUEEN
LICENSED PROFESSIONAL ENGINEER
18476
STATE OF MISSISSIPPI

1 AUGUST 2019

CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

MSE McQueen Structural Engineering
965 Highway 51, Suite 3 - Madison, MS 39110
(601) 898-1295

S1.0
STRUCTURAL NOTES

STRUCTURAL QUALITY ASSURANCE REQUIREMENTS

GENERAL

1. **SPECIAL INSPECTIONS:** SPECIAL INSPECTIONS SHALL BE COMPLETED PER CHAPTER 17 OF IBC 2012. THE CONTRACTOR WILL EMPLOY THE SERVICES OF A SPECIAL INSPECTOR TO PROVIDE INSPECTIONS FOR THE ITEMS LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS. THE SPECIAL INSPECTOR SHALL BE APPROVED BY THE ARCHITECT.

2. **SPECIAL INSPECTOR:** THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO INSPECTION OF ITEMS LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS.

3. **SCOPE:** THE SPECIAL INSPECTION REQUIREMENTS NOTED IN THE STRUCTURAL DOCUMENTS ARE FOR BUILDING COMPONENTS THAT ARE WITHIN THE SCOPE OF THE STRUCTURAL ENGINEER. REFER TO OTHER PORTIONS OF THE CONSTRUCTION DOCUMENTS FOR OTHER BUILDING COMPONENTS.

CONTRACTOR RESPONSIBILITY

1. **STATEMENT OF RESPONSIBILITY:** THE CONTRACTOR SHALL SUBMIT A STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON ANY SYSTEM LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS. THE STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:

   a. ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS

   b. ACKNOWLEDGMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL

   c. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS

   d. IDENTIFICATION AND QUALIFICATIONS OF THE PERSON EXERCISING SUCH CONTROL AND THEIR POSITION IN THE ORGANIZATION

SPECIAL INSPECTOR RESPONSIBILITY

1. **RECORD KEEPING:** SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, OWNER, ARCHITECT, AND CONTRACTOR. REPORTS SHALL INDICATE WHETHER OR NOT WORK INSPECTED WAS DONE IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS.

2. **FINAL REPORT:** A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON BY THE PERMIT APPLICANT AND THE BUILDING OFFICIAL PRIOR TO THE START OF WORK.

3. **DISCREPANCIES:** DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, OWNER, AND ARCHITECT PRIOR TO THE COMPLETION OF THAT PHASE OF WORK.

STATEMENT OF SPECIAL INSPECTIONS

1. **LATERAL FORCE RESISTING SYSTEM:**

   - SEISMIC SYSTEM:

     THE FOLLOWING ARE PART OF THE SEISMIC FORCE-RESISTING SYSTEM AND REQUIRE SPECIAL INSPECTION:
     a. CMU LOAD BEARING WALLS/SHEAR WALLS AND THEIR FOUNDATIONS
     b. STEEL ROOF DECK & CONNECTIONS
   - WIND SYSTEM: SPECIAL INSPECTION IS NOT REQUIRED

2. **SOILS:**

   CONTRACTOR DUTIES:

   A. PROVIDE THE SPECIAL INSPECTOR A COPY OF THE GEOTECHNICAL REPORT.

   SPECIAL INSPECTOR DUTIES:

   A. PERFORM TESTING AND MONITORING AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER AND GEOTECHNICAL REPORT AND AS INDICATED IN THE DRAWINGS AND SPECIFICATIONS. (PERIODIC INSPECTION)

   B. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL. (PERIODIC INSPECTION)

   C. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS. (PERIODIC INSPECTION)

   D. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL. (CONTINUOUS INSPECTION)

   E. PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THE SITE HAS BEEN PREPARED PROPERLY. (PERIODIC INSPECTION)

3. **STRUCTURAL STEEL:**

   CONTRACTOR DUTIES:

   A. SUBMIT THE MANUFACTURER'S CERTIFICATION THAT THE STEEL COMPLIES WITH THE STRUCTURAL CONSTRUCTION

      DOCUMENTS:
      - STRUCTURAL BOLTS, NUTS, AND WASHERS
      - STRUCTURAL STEEL MEMBERS

   B. SUBMIT STRUCTURAL STEEL SHOP DRAWINGS.

   SPECIAL INSPECTOR DUTIES:

   A. PERFORM PERIODIC INSPECTION OF THE STEEL FRAMING TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACING, STIFFENING, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.

   B. PERFORM PERIODIC INSPECTIONS TO VERIFY COMPLIANCE FOR THE FOLLOWING:

      a. MATERIAL VERIFICATION OF STRUCTURAL STEEL, HIGH-STRENGTH BOLTS, NUTS, AND WASHERS.

      b. SINGLE PASS FILLET WELDS ≤ 5/16"

   C. VISUALLY INSPECT ALL FIELD WELDS IN ACCORDANCE WITH AWS D1.1.

   D. VERIFY WELDING PROCEDURES ARE BEING ADHERED TO DURING FIELD WELDING.

4. **CONCRETE:**

   CONTRACTOR DUTIES:

   A. SUBMIT CONCRETE MIX DESIGN.

   B. SUBMIT MANUFACTURER'S CERTIFICATION FOR REINFORCING STEEL.

   C. SUBMIT REINFORCEMENT SHOP DRAWINGS.

   D. SUBMIT A PLAN FOR CURING PROCEDURES.

   SPECIAL INSPECTOR DUTIES:

   A. MATERIAL TESTING:

      a. PERFORM ALL TESTING IN SECTION 03.3000 OF THE PROJECT SPECIFICATIONS

   B. PERFORM PERIODIC INSPECTIONS TO VERIFY COMPLIANCE FOR THE FOLLOWING:

      a. FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF CONCRETE MEMBER BEING FORMED.

      b. CONCRETE REINFORCEMENT FOR LOCATION, SIZE, GRADE,
      AND SPLICE LENGTH.

      c. LOCATION OF WALL DOWELS AND OTHER EMBEDMENTS.

      d. VERIFY USE OF REQUIRED DESIGN MIX.

      e. VERIFY CURING PROCEDURES, TEMPERATURE, AND TECHNIQUES ARE IN COMPLIANCE WITH THE APPROVED CURING PLAN AND THE SPECIFICATIONS.

5. **POST-INSTALLED MECHANICAL AND EPOXY ANCHORS:**

   CONTRACTOR DUTIES:

   A. SUBMIT THE MANUFACTURER'S PRODUCT DATA:

      a. GENERAL PRODUCT DATA

      b. INSTALLATION INSTRUCTIONS

      c. ICC ES REPORT

   B. ENSURE PERSONNEL INSTALLING THE ANCHORS HAVE BEEN PROPERLY TRAINED TO INSTALL THE ANCHORS PER THE MANUFACTURER'S SPECIFICATIONS.

   SPECIAL INSPECTOR DUTIES:

   A. REVIEW PRODUCT DATA, INSTALLATION INSTRUCTIONS, AND ICC ES REPORT AND VERIFY THE ANCHORS ARE IN COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.

   B. PROVIDE CONTINUOUS INSPECTION FOR THE INSTALLATION OF THE FIRST 5 ANCHORS OF EACH TYPE AND VERIFY COMPLIANCE WITH THE MANUFACTURER'S INSTALLATION SPECIFICATIONS. PROVIDE PERIODIC INSPECTION OF THE REMAINING POST-INSTALLED ANCHORS.

6. **STEEL DECK:**

   CONTRACTOR DUTIES:

   A. SUBMIT SHOP DRAWINGS.

   SPECIAL INSPECTOR DUTIES:

   A. VERIFY STEEL DECK MATERIAL.

   B. VERIFY DECK INSTALLATION IS IN COMPLIANCE WITH THE SHOP DRAWINGS.

   C. VERIFY DECK CONNECTIONS.

7. **STEEL JOIST:**

   CONTRACTOR DUTIES:

   A. SUBMIT SHOP DRAWINGS.

   SPECIAL INSPECTOR DUTIES:

   A. PERFORM PERIODIC INSPECTIONS TO VERIFY COMPLIANCE FOR THE FOLLOWING:

      a. VERIFY JOIST PLACEMENT IS IN COMPLIANCE WITH THE SHOP DRAWINGS.

      b. VERIFY JOIST BOLTED AND WELDED CONNECTIONS.

      c. VERIFY JOIST BRIDGING IS IN COMPLIANCE WITH THE SHOP DRAWINGS.

      d. VERIFY JOIST REINFORCEMENT AT CONCENTRATED LOADS.

      e. VERIFY EMBEDMENT PLATES AT JOIST BEARINGS ARE LOCATED CORRECTLY.

8. **MASONRY:**

   CONTRACTOR DUTIES:

   A. SUBMIT MANUFACTURER'S CERTIFICATES FOR MATERIALS SHOWING COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS FOR EACH OF THE FOLLOWING:

      - CONCRETE MASONRY UNITS
      - MORTAR
      - GROUT
      - JOINT REINFORCEMENT
      - REINFORCING STEEL

   B. SUBMIT MIXING PROPORTIONS FOR GROUT.

   SPECIAL INSPECTOR DUTIES:

   A. REVIEW MATERIAL CERTIFICATES AND VERIFY COMPLIANCE.

   B. AS MASONRY CONSTRUCTION BEGINS AND PERIODICALLY DURING CONSTRUCTION, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:

      a. PROPORTIONS OF SITE-PREPARED MORTAR

      b. CONSTRUCTION OF MORTAR JOINTS

      c. LOCATION AND PROPER SUPPORT OF REINFORCEMENT

   C. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:

      a. GROUT SPACE IS CLEAN

      b. PLACEMENT OF REINFORCEMENT

      c. PROPORTIONS OF SITE-PREPARED GROUT

      d. CONSTRUCTION OF MORTAR JOINTS

   D. PERFORM CONTINUOUS INSPECTION TO ENSURE THE COMPLIANCE OF THE FOLLOWING:

      a. PLACEMENT OF GROUT IN CELLS

      b. PREPARATION OF REQUIRED GROUT AND MORTAR SPECIMENS

   E. VERIFY THE COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS PRIOR TO CONSTRUCTION BY TESTING THREE CONCRETE MASONRY UNITS PER ASTM C140. THE MANUFACTURER'S TESTING REPORTS ARE ACCEPTABLE FOR THIS REQUIREMENT.

9. **AGGREGATE PIERS:**

   CONTRACTOR DUTIES:

   A. SUBMIT SHOP DRAWINGS.

   SPECIAL INSPECTOR DUTIES:

   A. REVIEW MATERIAL CERTIFICATES AND VERIFY COMPLIANCE.

   B. AS PIER CONSTRUCTION BEGINS AND CONTINUOUSLY DURING CONSTRUCTION, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:

      a. FOOTING AND AGGREGATE PIER LOCATION.

      b. PIER LENGTH AND DRILLED DIAMETER.

      c. PLANNED AND ACTUAL PIER ELEVATIONS AT THE TOP AND BOTTOM OF THE ELEMENT.

      d. AVERAGE LIFT THICKNESS FOR EACH AGGREGATE PIER.

      e. SOILD TYPES ENCOUNTERED AT THE BOTTOM OF THE PIER AND ALONG THE LENGTH OF THE ELEMENT.

      f. DEPTH TO GROUNDWATER, IF ENCOUNTERED.

      g. DOCUMENTATION OF ANY UNUSUAL CONDITIONS ENCOUNTERED.

      h. TYPE AND SIZE OF DENSIFICATION EQUIPMENT USED.

1

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

JASON M. MCQUEEN
LICENSED PROFESSIONAL ENGINEER
18476
STATE OF MISSISSIPPI

1 AUGUST 2019

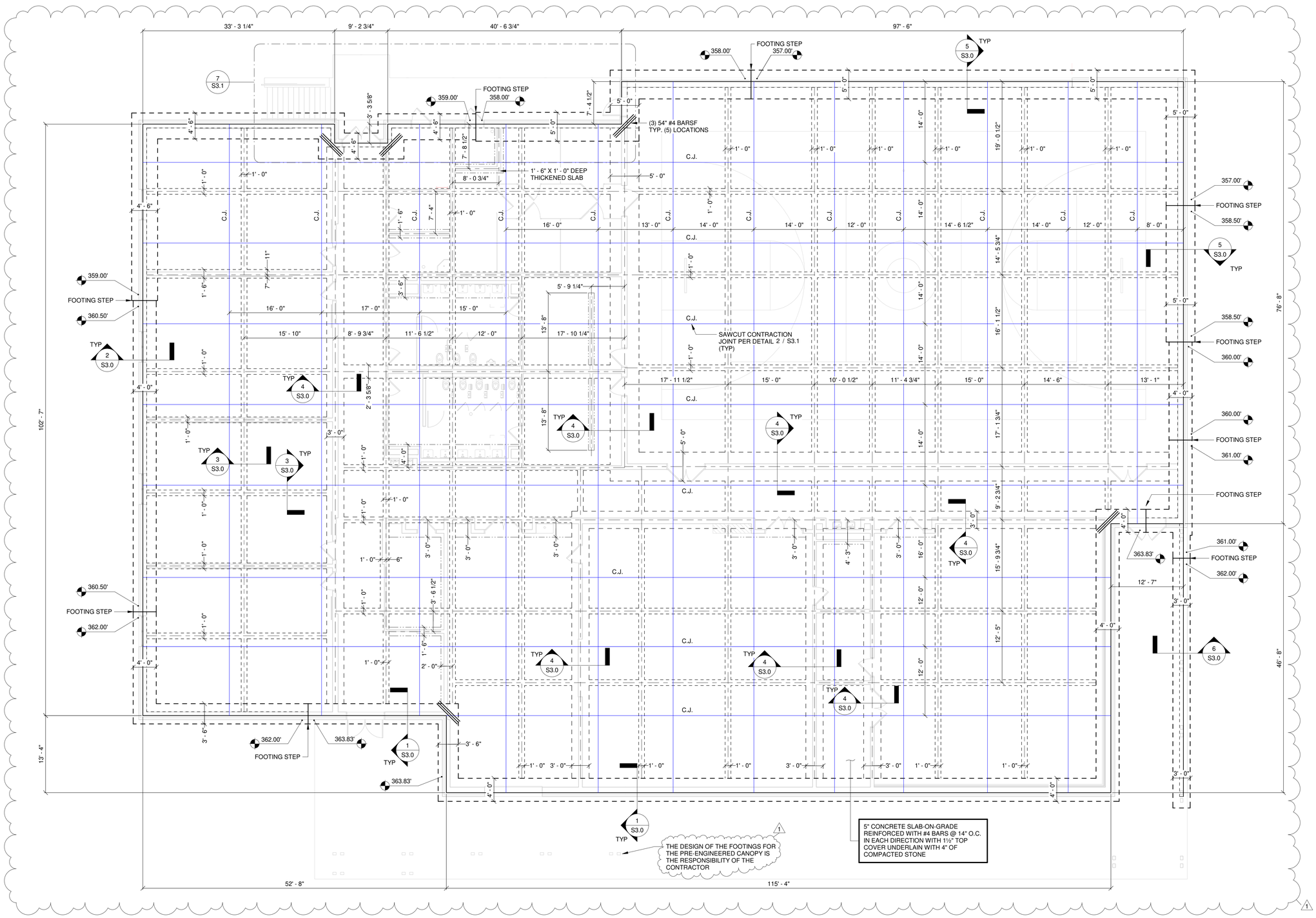
CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

MSE
McQueen Structural Engineering
965 Highway 51, Suite 3 - Madison, MS 39110
(601) 898-1295

S1.1
STRUCTURAL QUALITY ASSURANCE



FOUNDATION PLAN

1/8" = 1'-0"

TOP OF SLAB = 0' - 0" = 364.50'

NORTH

WBA ARCHITECTURE

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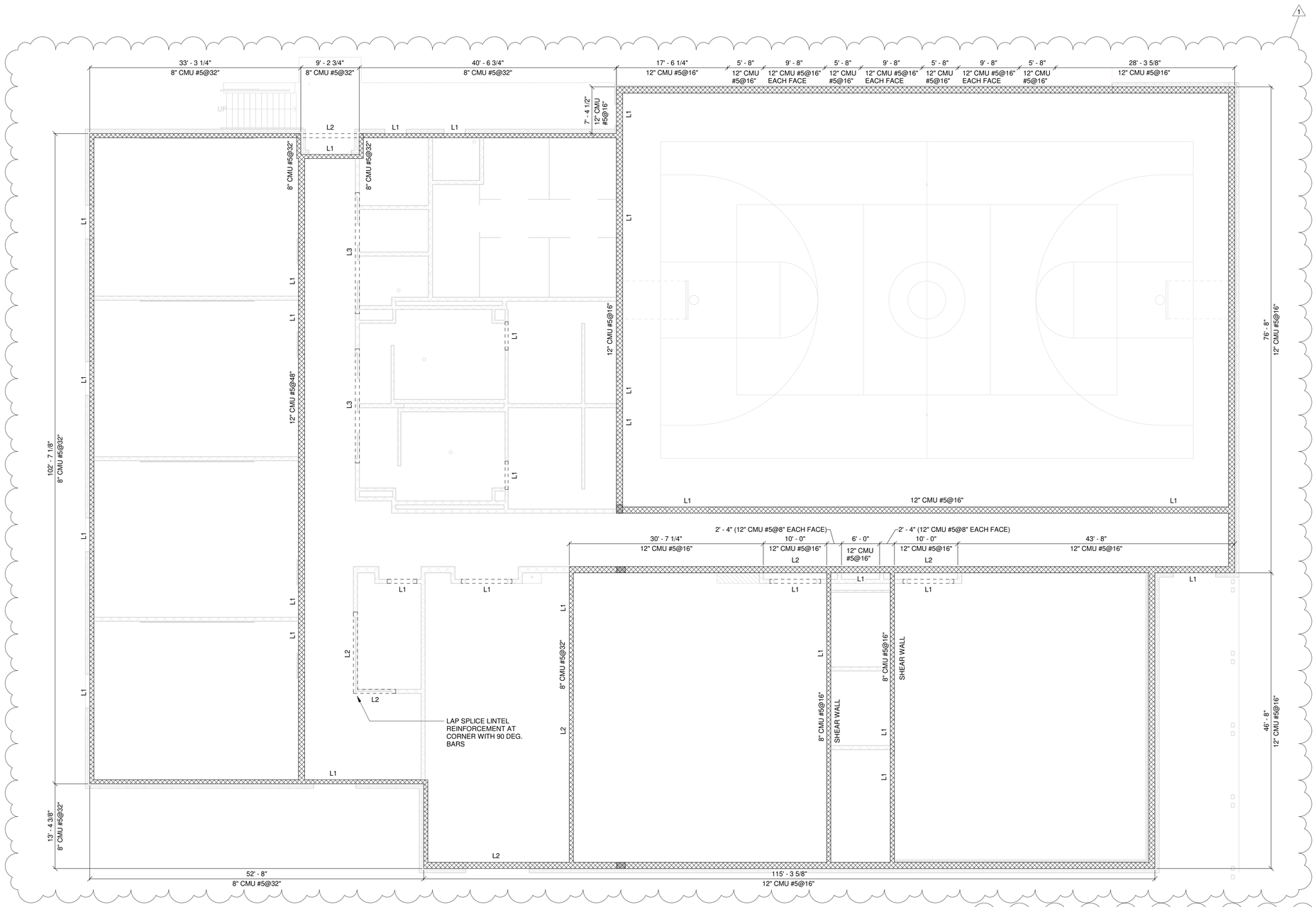
S2.0
FOUNDATION PLAN



CMU WALL PLAN

1/8" = 1'-0"

NORTH

CMU WALL NOTES

1. **REINFORCEMENT:**
   ALL INTERIOR AND EXTERIOR CMU WALLS SHALL BE REINFORCED WITH HORIZONTAL 9 GAUGE LADDER TYPE JOINT REINFORCEMENT @ 16" O.C.

   INTERIOR 8" WALLS -
   VERTICAL #5 BARS @ 48" O.C. (EXCEPT AS NOTED IN THE DETAILS)

   INTERIOR 6" WALLS -
   VERTICAL #5 BARS @ 16" O.C.
2. **PARTIAL GROUT:** GROUT SOLID REINFORCED CELLS AND BOND BEAMS AS INDICATED IN THE DRAWINGS. UNREINFORCED CELLS THAT ARE NOT A BOND BEAM DO NOT REQUIRE GROUTING EXCEPT WHERE SPECIFIED IN THE DETAILS.
3. **CONTROL JOINTS:** VERTICAL CONTROL JOINTS SHALL BE INSTALLED IN CMU WALLS AT A MAXIMUM SPACING OF 25' O.C. COORDINATE WITH THE STRUCTURAL ENGINEER FOR EXACT LOCATIONS.
4. **LINTELS:** SEE DETAIL 4 / S3.4 FOR CMU LINTELS.
5. **CMU DETAILS:** SEE S3.4 FOR CMU DETAILS
6. **NON-LOAD BEARING WALLS:** BRACE THE TOP OF NON-LOAD BEARING WALLS TO THE STEEL ROOF DECK PER DETAIL
7. **SHEAR WALLS:** SEE DETAIL 12 / S3.4 FOR THE CONNECTION OF INTERIOR SHEAR WALLS TO THE STEEL ROOF DECK.

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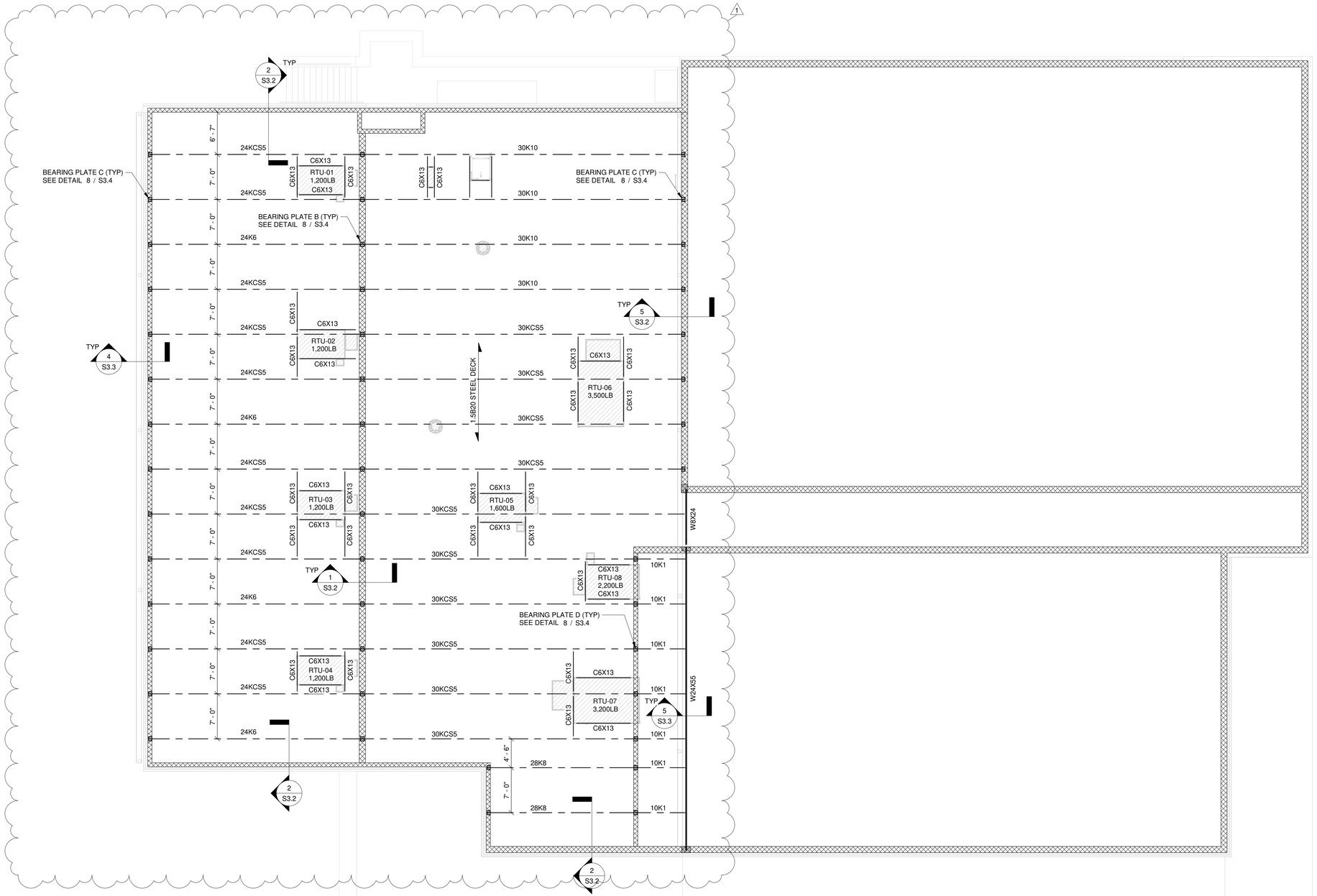
S2.1
CMU WALL PLAN



LOWER ROOF FRAMING PLAN

1/8" = 1'-0"

NORTH

ROOF FRAMING NOTES

1. THE STEEL JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT LOAD OF 15 PSF (ALLOWABLE STRESS DESIGN)
2. SEE THE STEEL JOIST SHOP DRAWINGS FOR ALL JOIST BRACING REQUIREMENTS.
3. THE STEEL DECK SHALL BE GALVANIZED (G90). SEE SPECIFICATIONS
4. SEE S1.0 FOR ROOF DECK FASTENING
5. THE LOCATIONS, SIZES, AND WEIGHTS OF THE MECHANICAL EQUIPMENT SHOWN ON THIS PLAN IS FOR REFERENCE ONLY. THE FINAL LOCATION, SIZE, AND WEIGHT OF ALL MECHANICAL EQUIPMENT MUST BE APPROVED BY THE STRUCTURAL ENGINEER.

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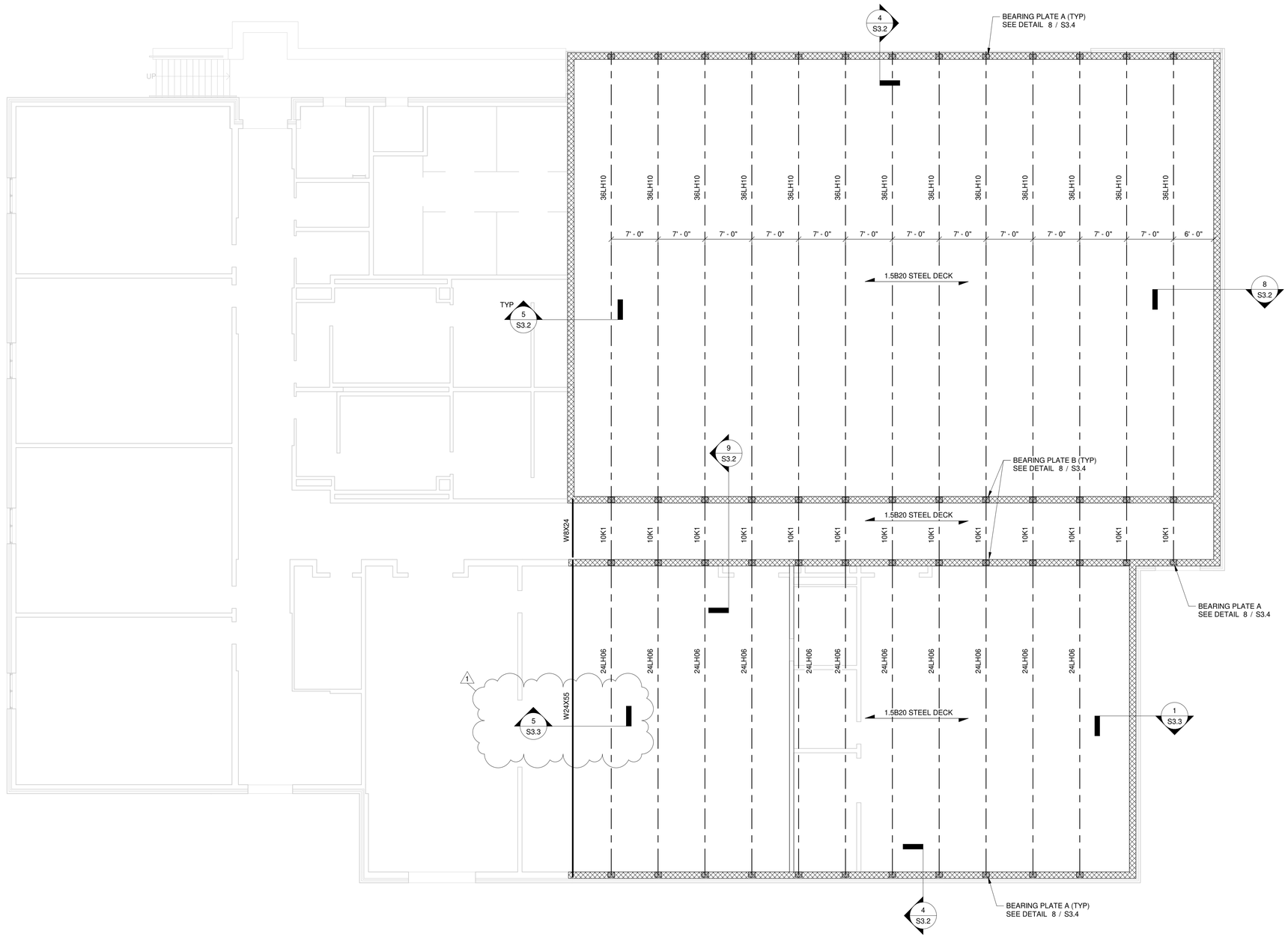
S2.2
LOWER ROOF FRAMING PLAN



UPPER ROOF FRAMING PLAN

1/8" = 1'-0"

NORTH

ROOF FRAMING NOTES

1. THE STEEL JOISTS SHALL BE DESIGNED FOR A NET WIND UPLIFT LOAD OF 15 PSF (ALLOWABLE STRESS DESIGN)
2. SEE THE STEEL JOIST SHOP DRAWINGS FOR ALL JOIST BRACING REQUIREMENTS
3. THE STEEL DECK SHALL BE GALVANIZED (G90). SEE SPECIFICATIONS
4. SEE S1.0 FOR ROOF DECK FASTENING
5. THE LOCATIONS, SIZES, AND WEIGHTS OF THE MECHANICAL EQUIPMENT SHOWN ON THIS PLAN IS FOR REFERENCE ONLY. THE FINAL LOCATION, SIZE, AND WEIGHT OF ALL MECHANICAL EQUIPMENT MUST BE APPROVED BY THE STRUCTURAL ENGINEER.

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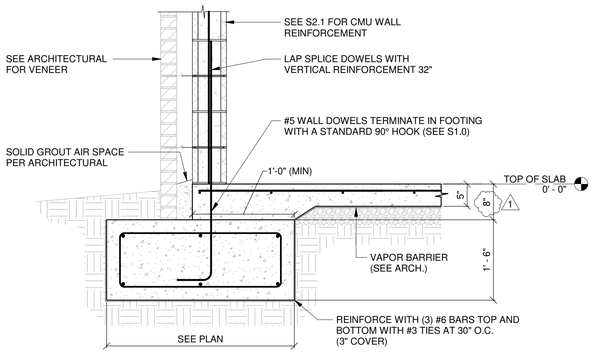
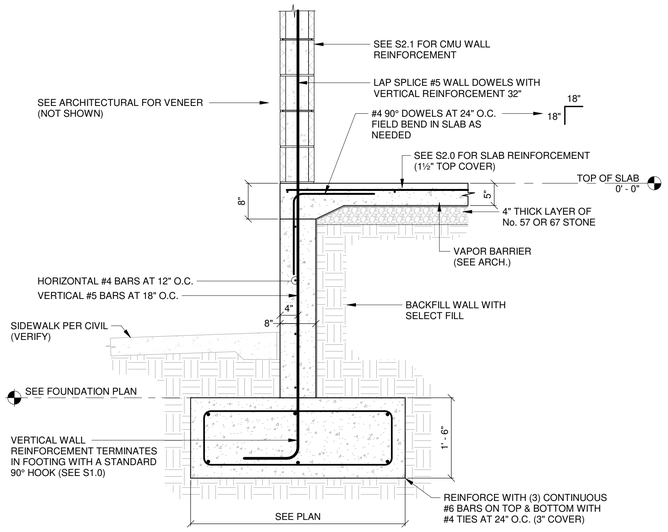
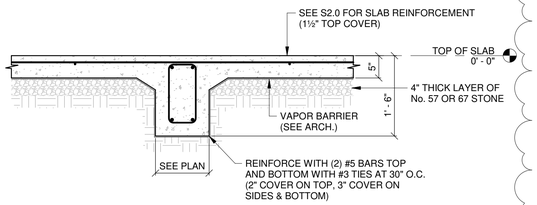
S2.3
UPPER ROOF FRAMING PLAN



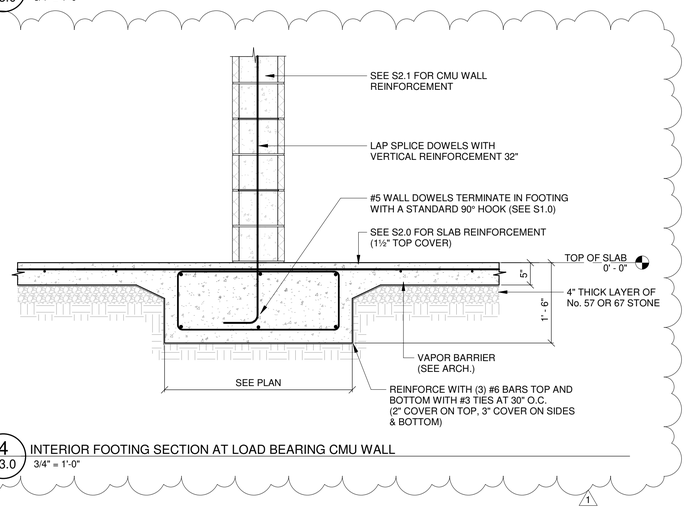
1 S3.0 EXTERIOR FOOTING SECTION
3/4" = 1'-0"



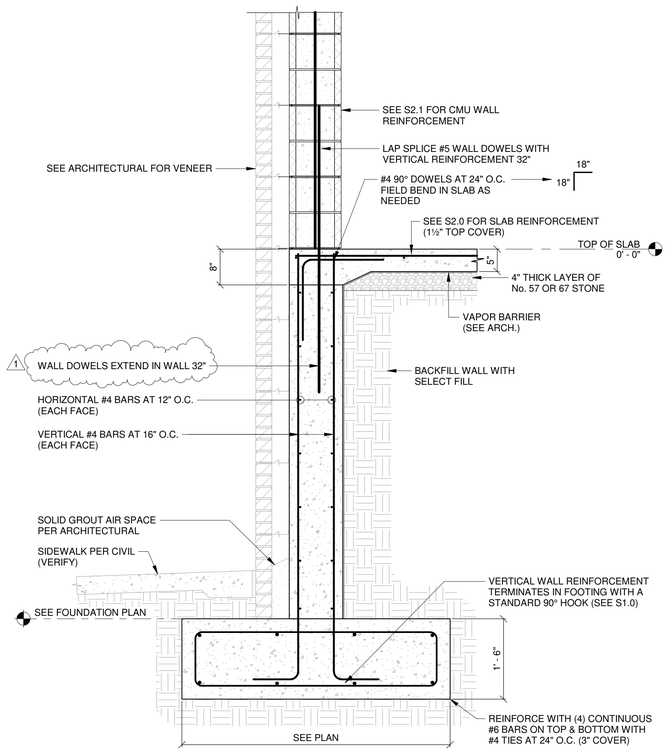
2 S3.0 FOUNDATION SECTION AT EXTERIOR WALL(8" WALL)
3/4" = 1'-0"



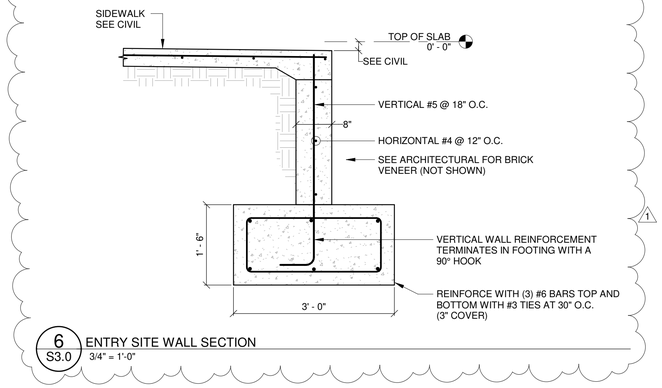
3 S3.0 INTERIOR GREADE BEAM SECTION
3/4" = 1'-0"



4 S3.0 INTERIOR FOOTING SECTION AT LOAD BEARING CMU WALL
3/4" = 1'-0"



5 S3.0 FOUNDATION SECTION AT EXTERIOR WALL(12" WALL)
3/4" = 1'-0"



6 S3.0 ENTRY SITE WALL SECTION
3/4" = 1'-0"

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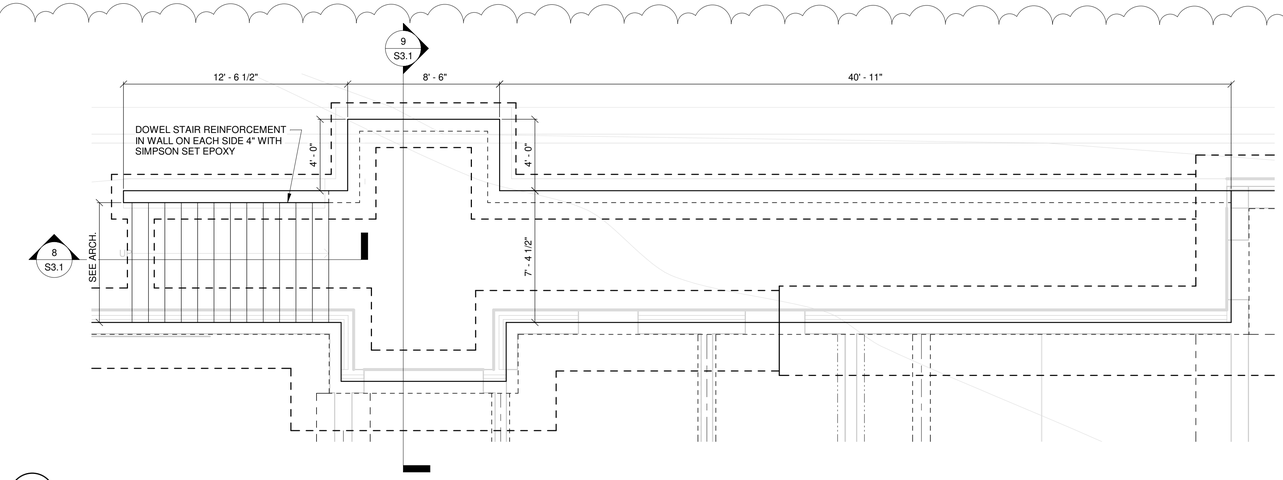
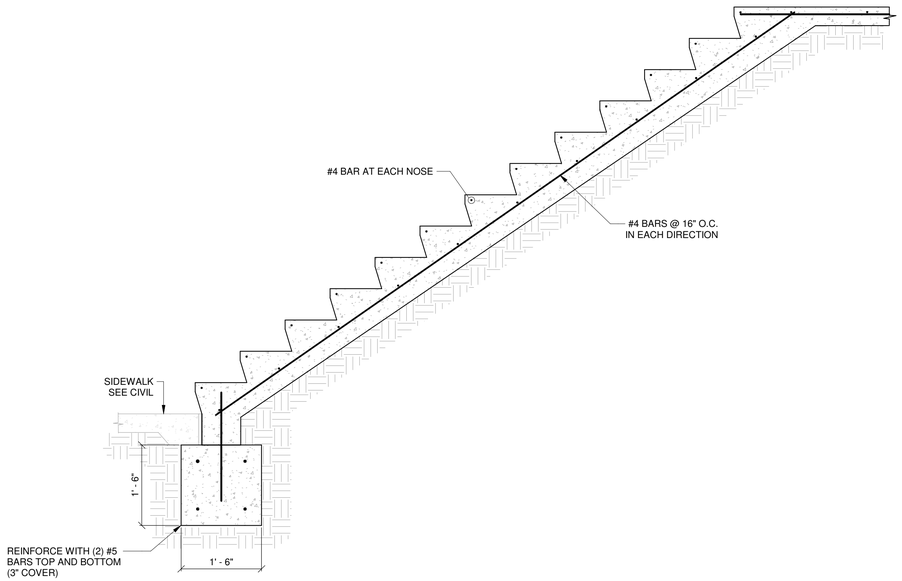
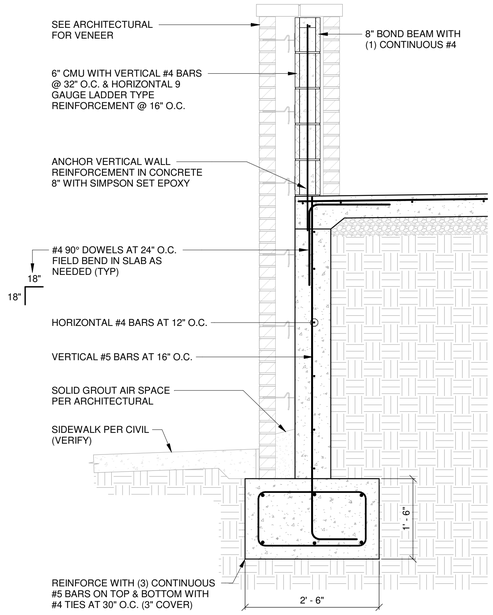
S3.0
DETAILS



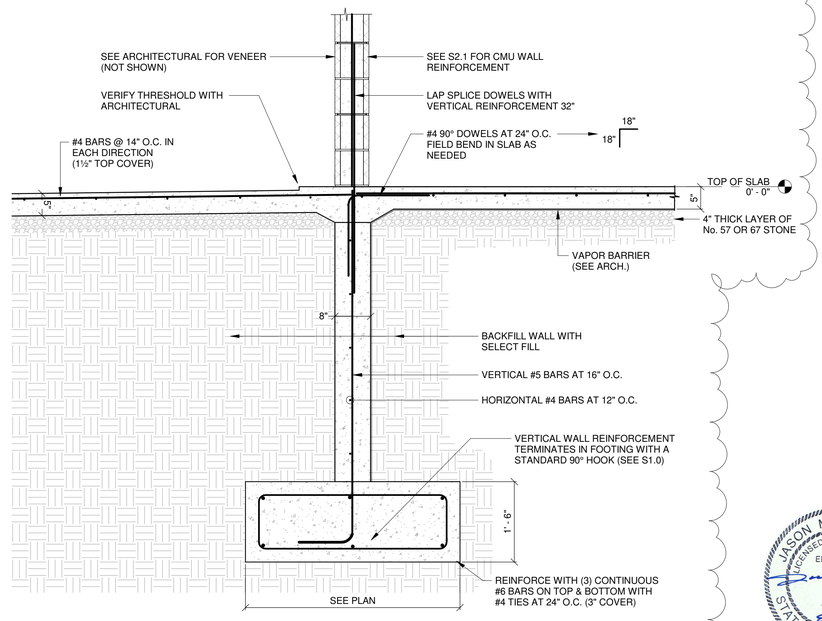
7 S3.1 ENLARGED PLAN AT SIDE EXIT
1/4" = 1'-0"

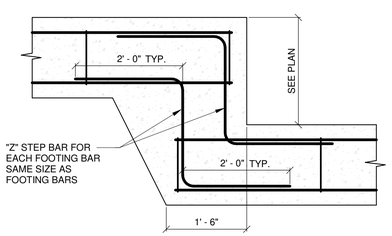


8 S3.1 CONCRETE STEPS DETAIL
3/4" = 1'-0"

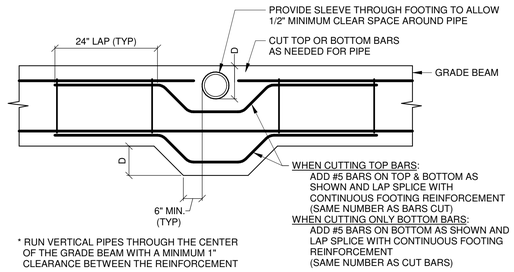


9 S3.1 SIDE EXIT FOUNDATION SECTION
3/4" = 1'-0"



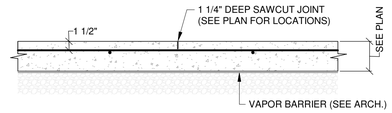


6 S3.1 FOOTING STEP DETAIL
3/4" = 1'-0"

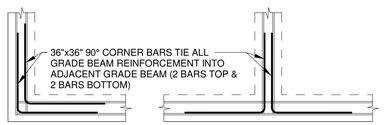


* RUN VERTICAL PIPES THROUGH THE CENTER OF THE GRADE BEAM WITH A MINIMUM 1" CLEARANCE BETWEEN THE REINFORCEMENT

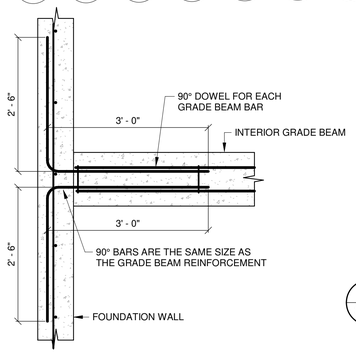
5 S3.1 PLUMBING PENETRATION IN FOUNDATION
3/4" = 1'-0"



2 S3.1 CONTRACTION JOINT
1" = 1'-0"

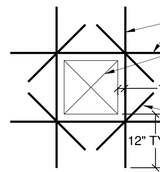


3 S3.1 GRADE BEAM INTERSECTION
3/8" = 1'-0"



TOP VIEW OF GRADE BEAM AND FOUNDATION WALL INTERSECTION

1 S3.1 WALL/GRADE BEAM INTERSECTION DETAIL
3/4" = 1'-0"



4 S3.1 SLAB PENETRATION DETAIL
3/4" = 1'-0"

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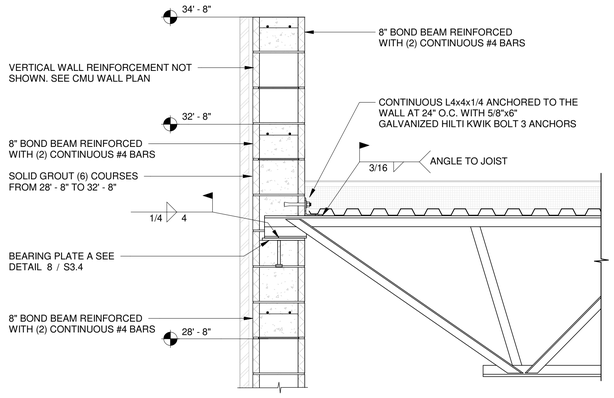
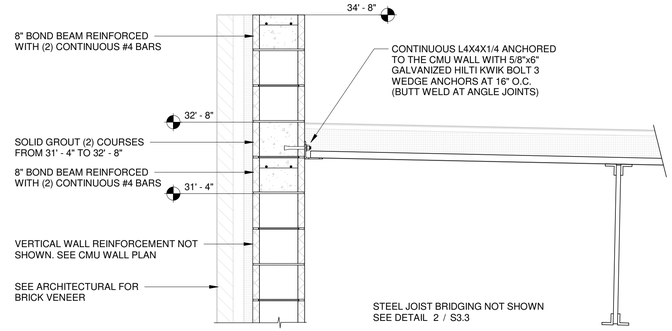
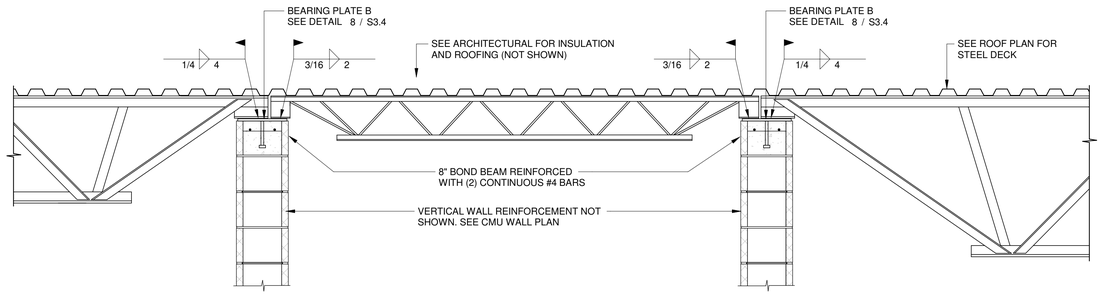


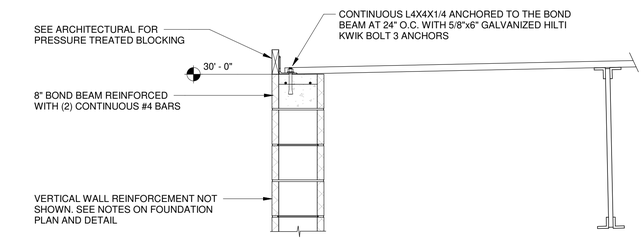
4 S3.2 ROOF JOIST BEARING AT EXTERIOR WALL - GYMNASIUM
3/4" = 1'-0"

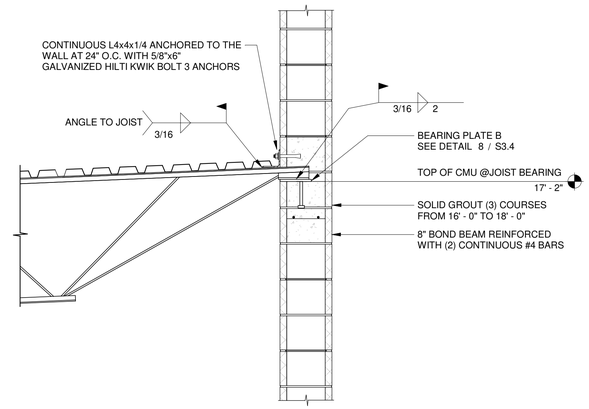


8 S3.2 ROOF DECK BEARING AT CMU WALL - GYMNASIUM
3/4" = 1'-0"

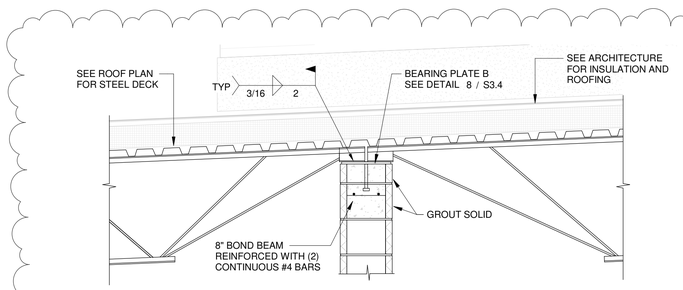


9 S3.2 UPPER ROOF SECTION AT CORRIDOR WALLS
3/4" = 1'-0"

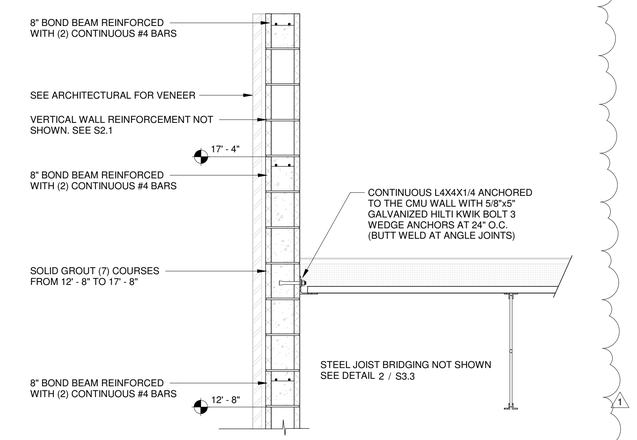




5 S3.2 ROOF JOIST AND DECK BEARING AT GYMNASIUM WALL
3/4" = 1'-0"



1 S3.2 ROOF JOIST BEARING AT INTERIOR WALL
3/4" = 1'-0"



2 S3.2 TYPICAL ROOF DECK BEARING AT EXTERIOR WALL
3/4" = 1'-0"

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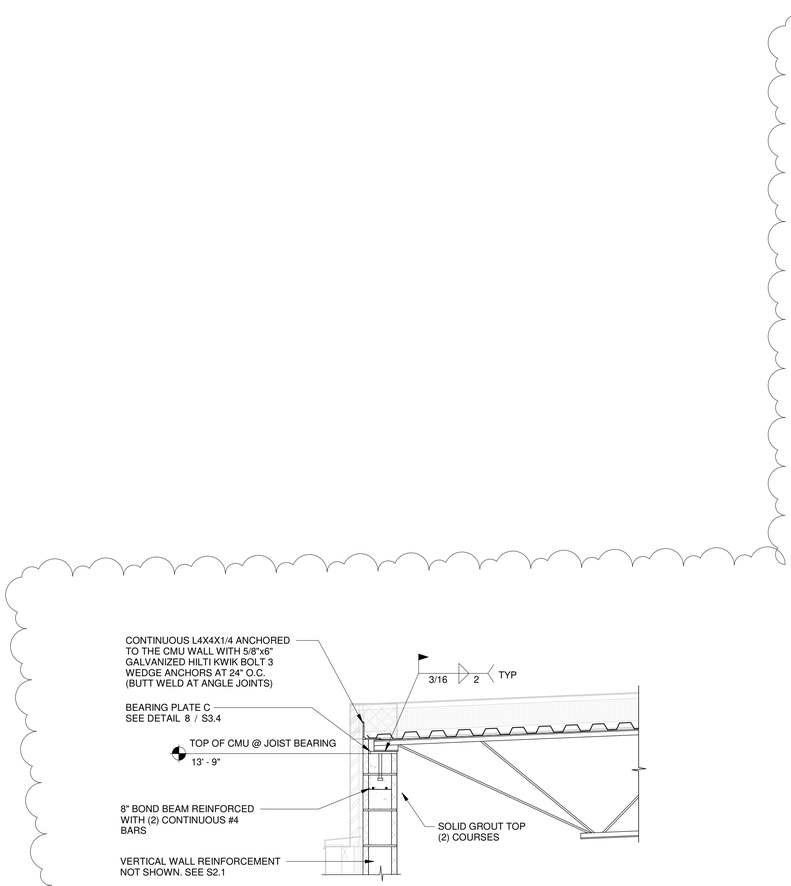
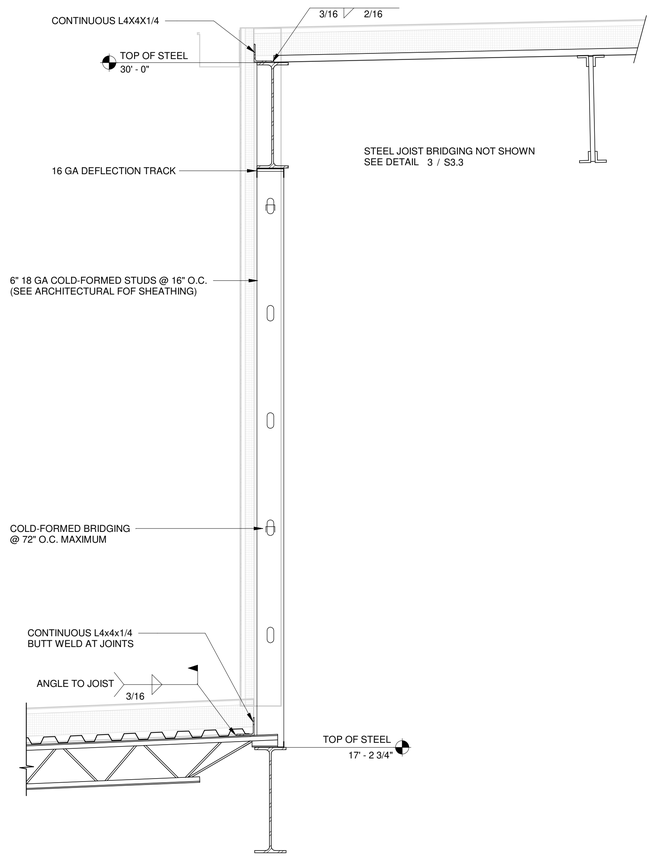
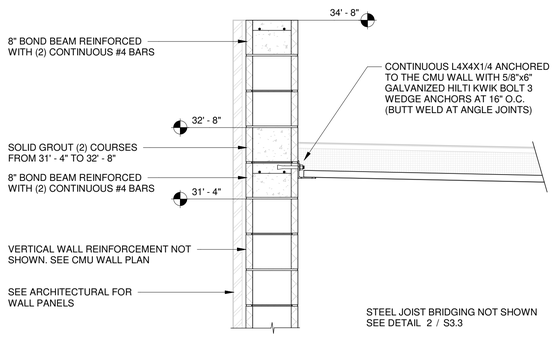


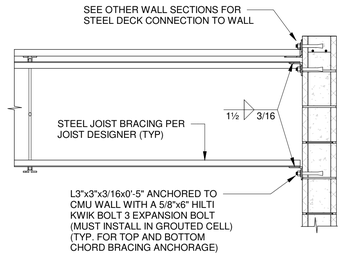
4 S3.3 ROOF JOIST BEARING AT EXTERIOR WALL - LOW ROOF
3/4" = 1'-0"



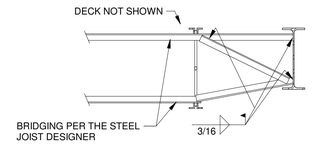
5 S3.3 ROOF SECTION AT STEEL BEAMS
3/4" = 1'-0"



1 S3.3 STEEL DECK BEARING AT CMU - DANCE ROOM
3/4" = 1'-0"



2 S3.3 STEEL JOIST BRIDGING TERMINUS POINT AT CMU WALL
3/4" = 1'-0"



3 S3.3 STEEL JOIST BRIDGING TERMINUS POINT AT STEEL BEAM
3/4" = 1'-0"

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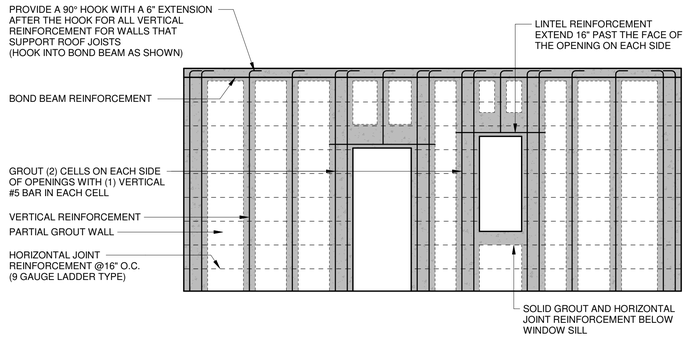
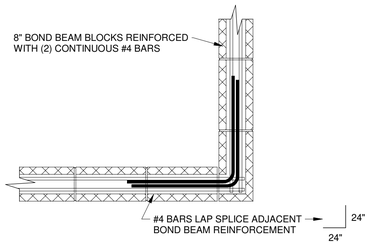
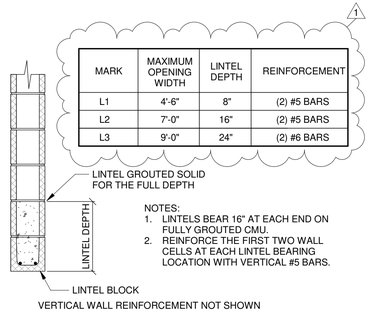


1 S3.4 CMU WALL REINFORCEMENT ELEVATION
1/4" = 1'-0"





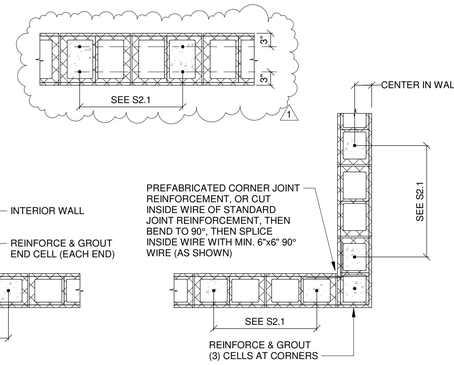
MARK	MAXIMUM OPENING WIDTH	LINTEL DEPTH	REINFORCEMENT
L1	4'-6"	8"	(2) #5 BARS
L2	7'-0"	16"	(2) #5 BARS
L3	9'-0"	24"	(2) #6 BARS

NOTES:
1. LINTELS BEAR 16" AT EACH END ON FULLY GROUTED CMU.
2. REINFORCE THE FIRST TWO WALL CELLS AT EACH LINTEL BEARING LOCATION WITH VERTICAL #5 BARS.

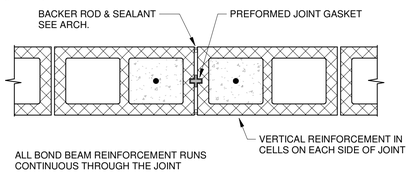
VERTICAL WALL REINFORCEMENT NOT SHOWN

3 S3.4 CMU WALL BOND BEAM REINFORCEMENT DETAIL
3/4" = 1'-0"

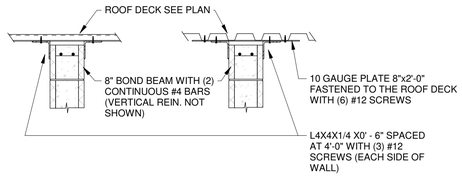
4 S3.4 CMU LINTEL SCHEDULE
3/4" = 1'-0"



2 S3.4 CMU WALL REINFORCEMENT DETAIL
3/4" = 1'-0"

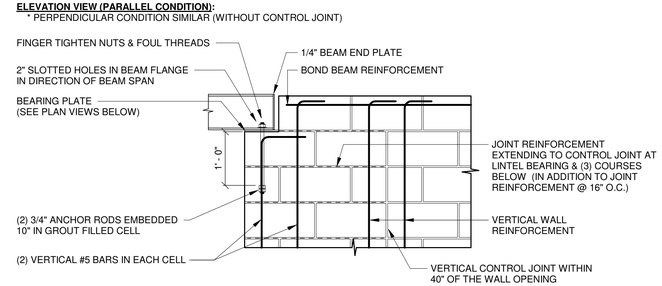


5 S3.4 CMU CONTROL JOINT DETAIL
1 1/2" = 1'-0"



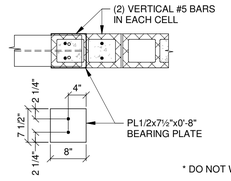
6 S3.4 NON-LOAD BEARING CMU WALL BRACING TO ROOF DECK
3/4" = 1'-0"

ELEVATION VIEW (PARALLEL CONDITION):
* PERPENDICULAR CONDITION SIMILAR (WITHOUT CONTROL JOINT)

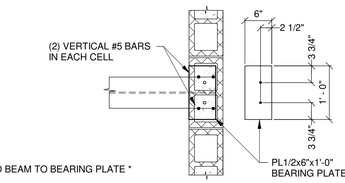


PLAN VIEWS:

PARALLEL CONDITION:

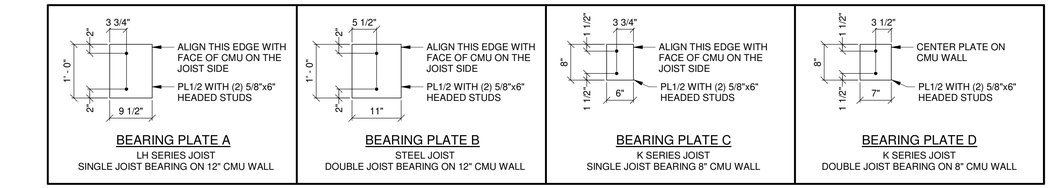


PERPENDICULAR CONDITION:

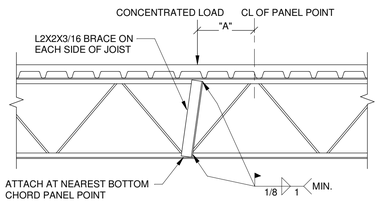


* DO NOT WELD BEAM TO BEARING PLATE *

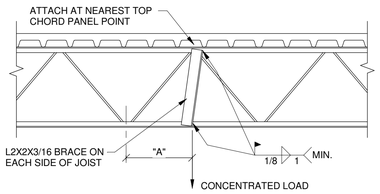
7 S3.4 STEEL BEAM BEARING ON CMU
3/4" = 1'-0"



8 S3.4 ROOF JOIST BEARING PLATE DETAILS
3/4" = 1'-0"



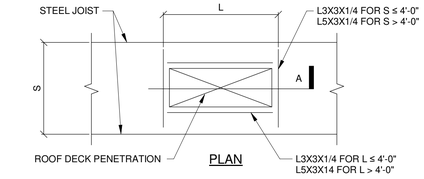
LOAD ON TOP CHORD



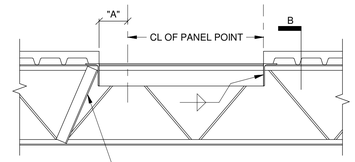
LOAD ON BOTTOM CHORD

NOTES:
1. DIAGONAL BRACE IS NOT REQUIRED FOR "A" LESS THAN THREE INCHES.
2. PROVIDE DIAGONAL BRACE AT LOCATION OF CONCENTRATED LOADS SUCH AS HEAVY PIPES, MECHANICAL UNITS, HEAVY LIGHTS, AND ANY OTHER CONCENTRATED LOADS AND AS NOTED ELSEWHERE IN THE STRUCTURAL DRAWINGS.
3. CONTRACTOR SHALL COORDINATE MAXIMUM ALLOWABLE CONCENTRATED LOAD WITH THE JOIST MANUFACTURER'S ENGINEER.

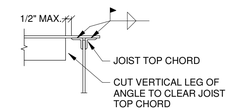
9 S3.4 ROOF DECK PENETRATIONS
3/4" = 1'-0"



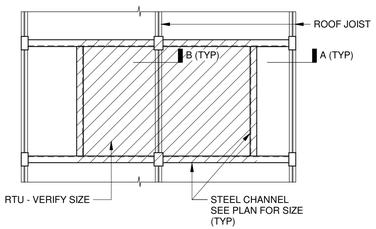
PLAN



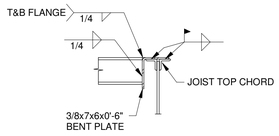
SECTION A



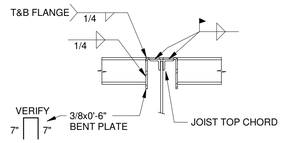
SECTION B



10 S3.4 STEEL JOIST REINFORCEMENT
3/4" = 1'-0"

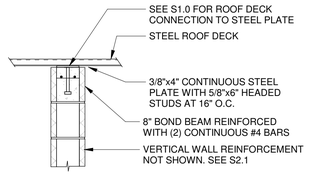


SECTION A



SECTION B

11 S3.4 RTU SUPPORT BEARING DETAIL
3/4" = 1'-0"



12 S3.4 INTERIOR CMU SHEAR WALL - TOP OF WALL CONNECTION
3/4" = 1'-0"

WBA ARCHITECTURE

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

JASON M. MCQUEEN
LICENSED PROFESSIONAL ENGINEER
18476
STATE OF MISSISSIPPI

1 AUGUST 2019

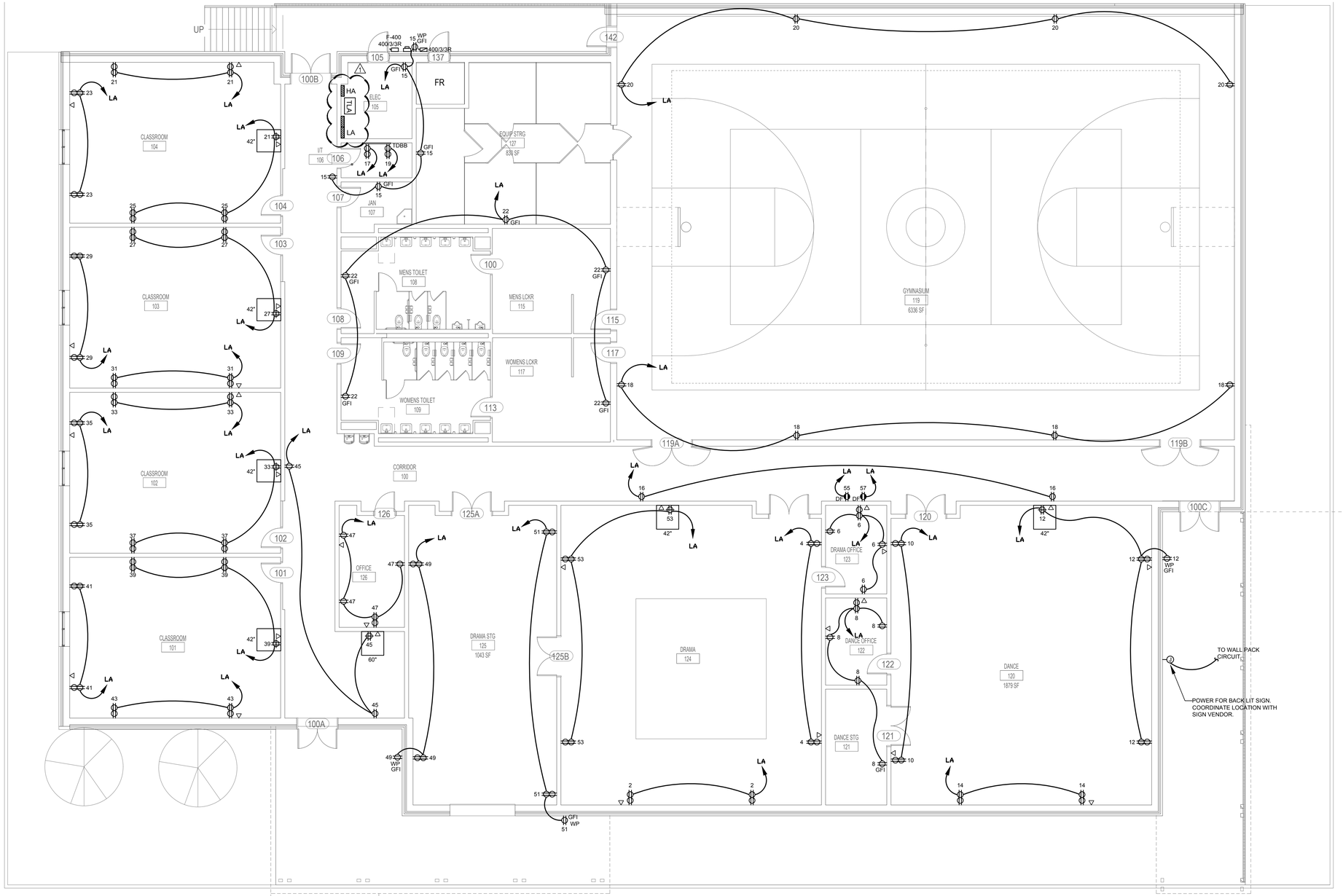
CONSTRUCTION DOCUMENTS
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM 1	08/19/19

MSE
McQueen Structural Engineering
965 Highway 51, Suite 3 - Madison, MS 39110
(601) 898-1295

S3.4
DETAILS



1 / E-200 POWER PLAN
Scale: 1/8" = 1'- 0"

WIER BOERNER ALLIN
ARCHITECTURE

2906 N.STATE STREET, SUITE 106
JACKSON, MISSISSIPPI 39216

P:601.321.9107 F:601.321.9108

PEARL HIGH SCHOOL MULTIPURPOSE BUILDING

PEARL PUBLIC SCHOOL DISTRICT

500 Pirates Cove
Pearl, MS 39110

THE POWER SOURCE PLLC
945 Madison Ave.
Madison, MS 39110
Voice (601) 605-4820
Fax (601) 605-4875
TPS Proj. # 19151

CHRISTOPHER GREEN — REGISTERED PROFESSIONAL ENGINEER — 7/24/19 — 16726 — STATE OF MISSISSIPPI

24 JULY 2019

DESIGN DEVELOPMENT
WBA # 0419

REVISIONS

NO.	DESCRIPTION	DATE
1	ADDENDUM #1	8/19/19

E-200

POWER PLAN