

March 30, 2023

Project Name: PEARL JUNIOR HIGH MULTIPURPOSE  
Project No: 22-056

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## ADDENDUM NO. 04

### NOTICE TO ALL DOCUMENT HOLDERS:

The following additions, deletions, changes and clarifications to the drawings and specifications are to be included as part of the Contract Documents.

### DRAWINGS

#### ITEM NO. 01

**REPLACE** the following sheets with attached revised sheets.

- |      |                            |   |
|------|----------------------------|---|
| R200 | LIFE SAFETY COMPLIANCE     | <ul style="list-style-type: none"><li>• Located fire extinguishers.</li></ul>   |
| C201 | SITE PLAN                  | <ul style="list-style-type: none"><li>• Clarification of retaining wall and sidewalk scope of work on the north side of the existing building</li></ul> |
| C301 | GRADING PLAN               | <ul style="list-style-type: none"><li>• Clarification of retaining wall and sidewalk scope of work on the north side of the existing building</li></ul> |
| C302 | STORM DRAIN LAYOUT         | <ul style="list-style-type: none"><li>• Clarification of new French drain around new building.</li></ul>  |
| C400 | GENERAL CONSTRUCTION NOTES | <ul style="list-style-type: none"><li>• Added details for the retaining wall and steps on the north side of the existing building.</li></ul>            |
| A002 | ENLARGED SITE PLAN         | <ul style="list-style-type: none"><li>• Clarification of site stair on North side of the existing building</li></ul>                                    |
| A301 | WALL SECTIONS – BASE BID   | <ul style="list-style-type: none"><li>• Clarification of the size and elevation of the French drain around the new building.</li></ul>                  |
| A302 | WALL SECTIONS – BASE BID   | <ul style="list-style-type: none"><li>• Clarification of the size and elevation of the French drain around the new building.</li></ul>                  |

## SPECIFICATIONS

**ITEM NO. 02**      **REPLACE** the following project manual sheets with the attached revised sheets.

- 00.0110      TABLE OF CONTENTS
  - Added 08.9200 “Louvered Equipment Enclosures”
  - Added 10.2113.19 “Plastic Toilet Compartment”.
  - Added 02.3000 “SUBSURFACE INVESTIGATION”.
- 02.3000      SUBSURFACE INVESTIGATION
  - Combined Geotechnical Report

**ITEM NO. 03**      **ADD** the following project manual sheets with the attached revised sheets.

08.9200      LOUVERED EQUIPMENT ENCLOSURES

**ITEM NO. 04**      **01.2000      PRICE AND PAYMENT PROCEDURES**

**Para. 3A1 : Add sub-item “b” to read as follows:**

“b. Project Wage rates shall be as indicated in the enclosed wage rate determination sheet dated 01/27/2023 – 5 pages.”

**ITEM NO. 05**      **00.2113      INSTRUCTIONS TO BIDDERS**

**Part 1 para. 11 j “Proposal Submittal” add sentence to read as follows:**

“Bid proposals shall be delivered to Pearl Public School District Central Office at the address listed herein. Bids shall be opened publicly by the Owner at this location.”

**ITEM NO. 06**      **01.2100      “ALLOWANCES”**

**Part 1 para. 5 B “ENERGY UTILITY ALLOWANCE” revise \$40,000.00 to read \$100,000.00**

## GENERAL CLARIFICATIONS

**ITEM NO. 06**      **CLARIFICATION NO 1.**

On sheet **A600 “FINISH LEGEND, SCHEDULE, AND DETAILS”** Acoustical Wall Panel (AP-1) is a premium custom match color. It is not a standard color from their chart.

**ITEM NO. 07**      **CLARIFICATION NO 2.**

Toilet partitions can be found under specification section **10.2113.19 “PLASTIC TOILET COMPARTMENTS”**.

**ITEM NO. 08**      **CLARIFICATION NO 3.**

Add general note to sheet **C200 “SITE DEMOLITION PLAN”** the states “Remove the 9 existing trees to allow for new parking lot and drive under alternate 3”.

**ITEM NO. 09**      **09.8430      SOUND-ABSORBING WALL UNITS**

Revise wall panel size to be 2 feet x 2 feet and 2” thick.

**ITEM NO. 10**      **C200      SITE DEMOLITION PLAN**

Revise per the enclosed sketch

Encl:      Drawings (8 Sheets - 24x36):  
            SK Drawings (1 Sheet – 8.5 x 11):  
            Wage Rates Determinations (5 Sheets – 8.5 x 11):  
            Project Manual Sheets: (27 sheets – 8.5 x 11):

cc:      All Document Holders

# LIFE SAFETY PLANS

### LIFE SAFETY LEGEND

- 1 HR RATED WALL
- 2 HR RATED WALL
- EXIT ACCESS TRAVEL DISTANCE
- MAX. OCCUPANCY NOTIFICATION SIGN
- TYPE ABC FIRE EXTINGUISHER CABINET
- TYPE ABC FIRE EXTINGUISHER w/WALL HOOK
- TYPE ABC FIRE EXTINGUISHER w/WALL HOOK

**1 OVERALL LIFE SAFETY PLAN - EXISTING BASEMENT**  
1/16" = 1'-0"

### OCCUPANT LOAD CALCULATIONS (BASEMENT)

Number	Name	Area	Load Factor	Total Occupant Load
E001	7TH FOOTBALL	1036 SF	50 SF	50
E004	LAUNDRY	94 SF	300 SF	2
E005	STORAGE	65 SF	300 SF	2
E006	FOOTBALL OFFICE	153 SF	150 SF	2
E007	8TH FOOTBALL	1810 SF	50 SF	112
E011	EQ STORAGE	65 SF	300 SF	1
E010	TR STORAGE	86 SF	300 SF	1
E012	CORRIDOR	628 SF	300 SF	3
016	WEIGHT ROOM	2641 SF	50 SF	54
015	LOCKER ROOM	1559 SF	50 SF	32
017	MEETING ROOM	1113 SF	70 SF	70
Grand total: 11		9249 SF		329 OCCUPANTS

### OCCUPANT LOAD CALCULATIONS (FIRST FLOOR)

Number	Name	Area	Load Factor	Total Occupant Load
123	STORAGE	72 SF	300 SF	2
E101	STORAGE	75 SF	300 SF	2
E102	OFFICE	75 SF	150 SF	2
E104	CART STORAGE	84 SF	300 SF	2
E105	CONCESSION	153 SF	150 SF	2
E108	GYMNASIUM	9320 SF	15 SF	622
E110	ISS	381 SF	20 SF	20
E111	CLASSROOM	727 SF	20 SF	37
E112	CLASSROOM	701 SF	20 SF	36
E113	CLASSROOM	721 SF	20 SF	36
E114	STORAGE	173 SF	30 SF	6
E119	BB OFFICE 2	126 SF	150 SF	2
E120	BB OFFICE 1	120 SF	150 SF	2
Grand total: 13		12729 SF		771 OCCUPANTS

### OCCUPANT LOAD CALCULATIONS (NEW ADDITION)

Number	Name	Area	Load Factor	Total Occupant Load
200	CLASSROOM	772 SF	20 SF	40
201	CLASSROOM	783 SF	20 SF	40
203	CLASSROOM	783 SF	20 SF	40
205	CLASSROOM	772 SF	20 SF	40
209	OFFICE	122 SF	150 SF	2
210	MULTIPURPOSE	4505 SF	5 SF	897
211	STORAGE	76 SF	300 SF	1
213	STORAGE	53 SF	300 SF	1
214	STORAGE	72 SF	300 SF	1
215	STORAGE	72 SF	300 SF	1
216	STORAGE	98 SF	300 SF	1
217	IT/DATA	43 SF	300 SF	1
218	RISER ROOM	59 SF	300 SF	1
219	ELECTRICAL	81 SF	300 SF	1
Grand total: 14		8291 SF		1,067 OCCUPANTS

# MEANS OF EGRESS

OCCUPANT LOAD	EGRESS SIZING								
	CORRIDOR WIDTH			EGRESS DOOR WIDTH			STAIR WIDTH		
SEE LIFE SAFETY PLANS FOR DETAILED OCCUPANCY LOAD INFORMATION	MIN.	CALC.	ACTUAL	MIN.	CALC.	ACTUAL	MIN.	CALC.	ACTUAL
BASEMENT FLOOR	329	329	329	32"	32"	32"	44"	44"	44"
1ST FLOOR	771	771	771	32"	32"	32"	44"	44"	44"
NEW ADDITION	1,067	1,067	1,067	32"	32"	32"	44"	44"	44"
<b>TOTAL</b>	<b>2,167</b>	<b>2,167</b>	<b>2,167</b>						

NUMBER OF EXITS	EGRESS PATH	
	REQUIRED	PROVIDED
BASEMENT FLOOR	4	4
1ST FLOOR	4	5
<b>TOTAL</b>	<b>8</b>	<b>9</b>

# FIRE AND SMOKE PROTECTION

**FLOOR ASSEMBLY : UL# XXXXX**      **EXTERIOR WALL ASSEMBLY : UL# XXXXX**

**ROOF ASSEMBLY : UL# XXXXX**      **FIRE BARRIER ASSEMBLY : UL# XXXXX**

A. PARTITION TYPES & FIRE RESISTIVE RATINGS INDICATED ON A WALL ARE TO BE CONTINUOUS FOR THE LENGTH & HEIGHT OF A PARTITION.

B. OPENINGS IN RATED WALL, FLOOR, CEILING & ROOF ASSEMBLIES SHALL BE SEALED WITH PENETRATION SEALANT SYSTEMS MEETING OR EXCEEDING THE REQUIRED FIRE RESISTIVE RATINGS.

C. MAINTAIN THE FIRE RATING OF CONSTRUCTION AROUND CABINETS, PANELS, & BOXES RECESSED IN FIRE RATED WALL, FLOOR, & CEILING ASSEMBLIES.

D. PROVIDE CONTINUOUS PERIMETER FIRE SAFING BETWEEN FLOORS & COORDINATE THE INSTALLATION WITH THE EXTERIOR WALL. FIRE RATING OF SAFING SHALL MATCH FIRE RATING OF FLOOR CONSTRUCTION.

AUTOMATIC SPRINKLER SYSTEM REQUIREMENTS	STANDPIPE SYSTEM REQUIREMENTS
AUTOMATIC SPRINKLER SYSTEM NOT REQUIRED. THIS PROJECT WILL STILL PROVIDE ONE.	

### PORTABLE FIRE EXTINGUISHER REQUIREMENTS

**F.E.C. FIRE EXTINGUISHER CABINET**  
TYPE "ABC" FIRE EXTINGUISHER IN SEMI-RECESSED WALL CABINET. CONFIRM PLACEMENT WITH LOCAL INSPECTOR / FIRE MARSHAL PRIOR TO INSTALLATION.

**F.E.-K. FIRE EXTINGUISHER - KITCHEN**  
TYPE "K" FIRE EXTINGUISHER ON SURFACE-MOUNTED WALL HOOK. CONFIRM PLACEMENT WITH LOCAL INSPECTOR / FIRE MARSHAL PRIOR TO INSTALLATION.

**F.E. FIRE EXTINGUISHER**  
TYPE "ABC" FIRE EXTINGUISHER ON SURFACE-MOUNTED WALL HOOK. CONFIRM PLACEMENT WITH LOCAL INSPECTOR / FIRE MARSHAL PRIOR TO INSTALLATION.

SEE LIFE SAFETY PLANS AND FLOOR PLANS FOR LOCATIONS OF EACH TYPE OF PORTABLE FIRE EXTINGUISHER.

- MAX FLOOR AREA PER F.E.: 3500SQFT
- MAX DISTANCE BETWEEN F.E.: 75FT

# PLUMBING FIXTURES - NEW BUILDING

TOTAL OCCUPANT LOAD PER OCCUPANCY CLASSIFICATION	WATER CLOSETS AND/OR URINALS		LAVATORIES		DRINKING FOUNTAINS	SERVICE SINKS
	MALE	FEMALE	MALE	FEMALE		
BASEMENT FLOOR 168 TOTAL (84 M/84 F)	1 PER 50	1 PER 50	1 PER 50	1 PER 50	2 REQUIRED	1
	2 REQUIRED	2 REQUIRED	2 REQUIRED	2 REQUIRED	2 PROVIDED	1 PROVIDED
	<b>5 PROVIDED</b>	<b>5 PROVIDED</b>	<b>4 PROVIDED</b>	<b>4 PROVIDED</b>		

# PLUMBING FIXTURES - EXISTING BUILDING

TOTAL OCCUPANT LOAD PER OCCUPANCY CLASSIFICATION	WATER CLOSETS AND/OR URINALS		LAVATORIES		DRINKING FOUNTAINS	SERVICE SINKS
	MALE	FEMALE	MALE	FEMALE		
492 TOTAL (246 M / 246 F)	1 PER 50	1 PER 50	1 PER 50	1 PER 50	2 REQUIRED	1
	10 REQUIRED	10 REQUIRED	10 REQUIRED	10 REQUIRED	2 PROVIDED	1 PROVIDED
	<b>16 PROVIDED</b>	<b>16 PROVIDED</b>	<b>11 PROVIDED</b>	<b>11 PROVIDED</b>		

# GENERAL INFO

### PROJECT CONTACTS

OWNER	PEARL PUBLIC SCHOOL DISTRICT	ARCHITECT	WIER BOERNER ALLIN, PLLC
CONTACT	CHRIS CHISM	CONTACT	JAMIE WIER
PHONE	601.932.7921 (P)	PHONE	601.321.9107
ADDRESS	3375 HIGHWAY 80 EAST PEARL, MS 39208	ADDRESS	2727 OLD CANTON ROAD, SUITE 200 JACKSON MS 39216
EMAIL	chism@pearl12.com	EMAIL	jwier@wbaarchitecture.com

### AUTHORITIES HAVING JURISDICTION

PRIMARY AHJ	CONTACT PERSON(S)	PRIMARY AHJ	CONTACT PERSON(S)
ADDITIONAL AHJ	CONTACT PERSON(S)	ADDITIONAL AHJ	CONTACT PERSON(S)

### APPLICABLE CODES

SCOPE	YEAR / CODE	AUTHORITY
BUILDING	2018 INTERNATIONAL BUILDING CODE	INTERNATIONAL CODE COUNCIL
ACCESSIBILITY	2010 ADA STANDARDS FOR ACCESSIBLE DESIGN	UNITED STATES DEPARTMENT OF JUSTICE
LIFE SAFETY	2018 INTERNATIONAL LIFE SAFETY CODE	INTERNATIONAL CODE COUNCIL
FIRE PREVENTION	2018 INTERNATIONAL FIRE CODE	INTERNATIONAL CODE COUNCIL
ENERGY	2018 INTERNATIONAL BUILDING CODE	INTERNATIONAL CODE COUNCIL
ELECTRIC	2018 INTERNATIONAL ELECTRICAL CODE   NFPA 70	NATIONAL FIRE PREVENTION ASSOCIATION
PLUMBING	2018 INTERNATIONAL PLUMBING CODE	INTERNATIONAL CODE COUNCIL
MECHANICAL	2018 INTERNATIONAL MECHANICAL CODE	INTERNATIONAL CODE COUNCIL
GAS	2018 INTERNATIONAL FUEL GAS CODE	INTERNATIONAL CODE COUNCIL
ZONING	XXXXXX	XXXXXX

### USE GROUP AND OCCUPANCY CLASSIFICATION

USE GROUP(S): \_\_\_\_\_

MIXED USE CLASSIFICATION (IF APPLICABLE): \_\_\_\_\_

### CONSTRUCTION TYPE & FIRE RESISTANCE RATINGS

FIRE-RESISTANCE RATINGS PER TABLE 601	FIRE-RESISTANCE RATINGS FOR NONBEARING EXTERIOR WALLS BASED ON FIRE SEPARATION DISTANCE PER TABLE 602	
	FIRE SEPARATION DISTANCE	CONSTRUCTION TYPE IIB
STRUCTURAL FRAME	0 HR.	
FLOOR CONSTRUCTION	0 HR.	
ROOF CONSTRUCTION	0 HR.	X ≤ 5'
EXTERIOR BEARING WALLS	0 HR.	5' ≤ X ≤ 10'
INTERIOR BEARING WALLS	0 HR.	10' ≤ X ≤ 30'
INTERIOR NONBEARING WALLS	0 HR.	X > 30'

BUILDING FULLY SPRINKLERED: **Y** **N**

# HEIGHT & AREA LIMITATIONS

HEIGHT	AREA			
	ALLOWABLE	ACTUAL	ALLOWABLE (CALCULATE FRONTAGE INCREASE BELOW)	ACTUAL BUILDING AREA
PER IBC TABLE 504.3 (IN FEET)	75	30		
PER IBC TABLE 504.4 (IN STO.)	3	1	38,000 SF	20,754 SF

### BUILDING DIAGRAMS

SECTION 506.3 FRONTAGE INCREASE  
EVERY BUILDING SHALL ALLOW OR HAVE ACCESS TO A PUBLIC WAY TO RECEIVE AN AREA FACTOR INCREASE BASED ON FRONTAGE. SECTION 506.3.3 AMOUNT OF INCREASE. THE AREA FACTOR INCREASE BASED ON FRONTAGE SHALL BE DETERMINED IN ACCORDANCE WITH EQUATION 5-5:

DERIVE "Y" VALUE USING EQUATION 5-4:  
 $W = [L_1 \times (1 - 0.25N_1) + (L_2 \times 0.3) + \dots] \times P$   
 WHERE:  
 L<sub>1</sub> = LENGTH OF A PORTION OF THE EXTERIOR PERIMETER WALL.  
 W = WIDTH OF A PUBLIC WAY OR OPEN SPACE ASSOCIATED WITH THAT PORTION OF THE EXTERIOR PERIMETER WALL.  
 P = PERIMETER THAT FRONTS ON A PUBLIC WAY OR OPEN SPACE HAVING A WIDTH OF 20 FT OR MORE.  
 USE VALUE "X" FOR "Y" IN EQUATION 5-5.  
 IF P ≤ 0.25W/30  
 WHERE:  
 W = WIDTH OF PUBLIC WAY OR OPEN SPACE IN ACCORDANCE WITH SECTION 506.3.2

ALLOWABLE BUILDING AREA  
SECTION 506.2.1 SINGLE-OCCUPANCY, ONE-STORY BUILDINGS  
EQUATION 5-1:  
 $A_n = A_s \times (1 + N_1/10)$   
 WHERE:  
 A<sub>n</sub> = ALLOWABLE BUILDING AREA  
 A<sub>s</sub> = ALLOWABLE AREA FACTOR PER TABLE 506.2  
 N<sub>1</sub> = ALLOWABLE AREA FACTOR IN ACCORDANCE WITH TABLE 506.2 FOR A NONSPRINKLERED BUILDING  
 IF = AREA FACTOR INCREASE DUE TO FRONTAGE (EQUATION 5-5)



PPSD JH  
MULTIPURPOSE

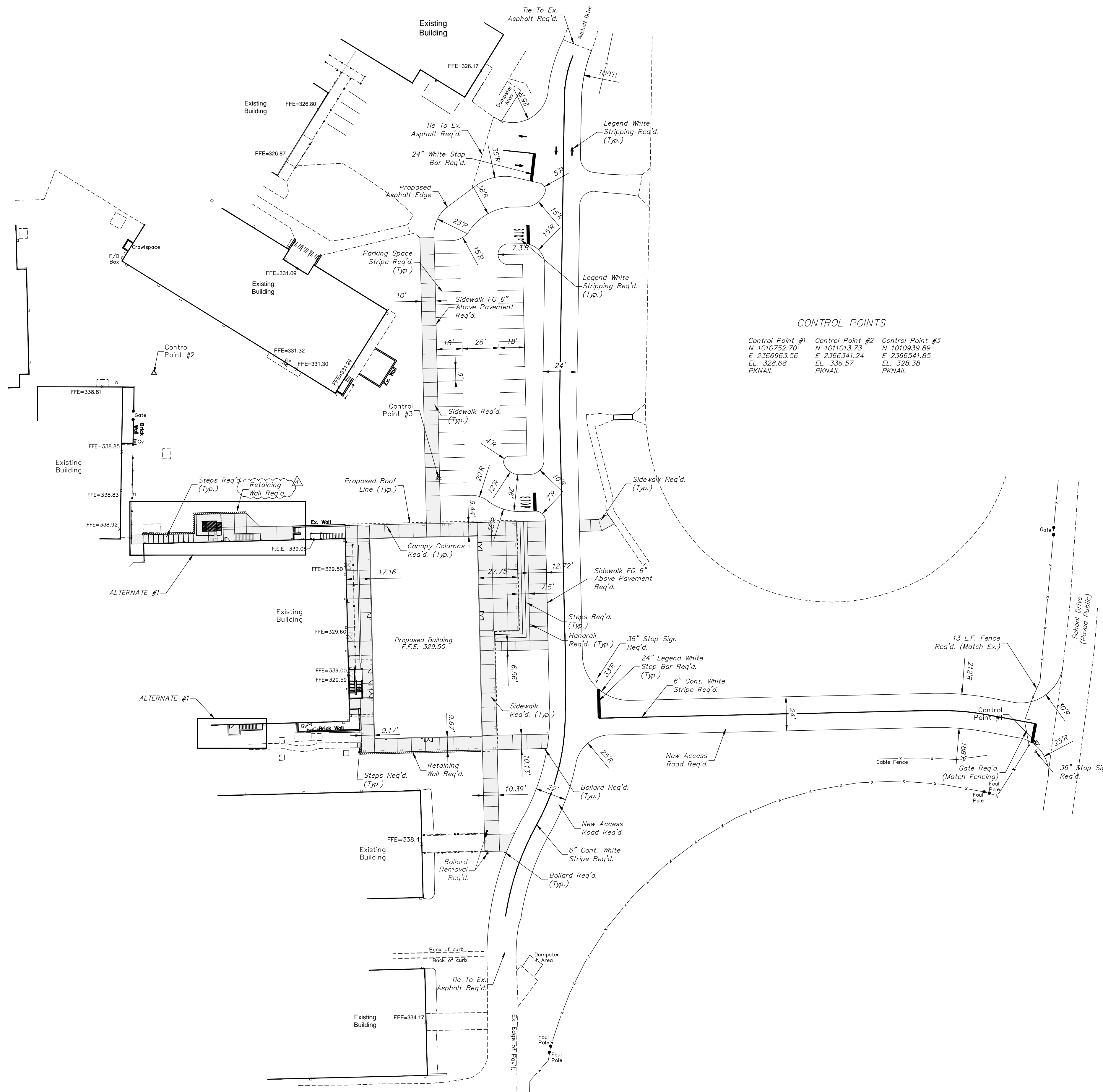
200 Mary Ann Drive  
Pearl, MS 39208

02/21/23

CONSTRUCTION DOCUMENTS  
WBA # 22-056

NO.	DESCRIPTION	DATE
4	Revision 4	3.30.2023





**CONTROL POINTS**

Control Point #1	Control Point #2	Control Point #3
N 1010752.70	N 1011013.73	N 1010939.89
E 2366963.56	E 2366541.24	E 2366541.85
EL. 328.68	EL. 336.57	EL. 328.38
PKNAIL	PKNAIL	PKNAIL

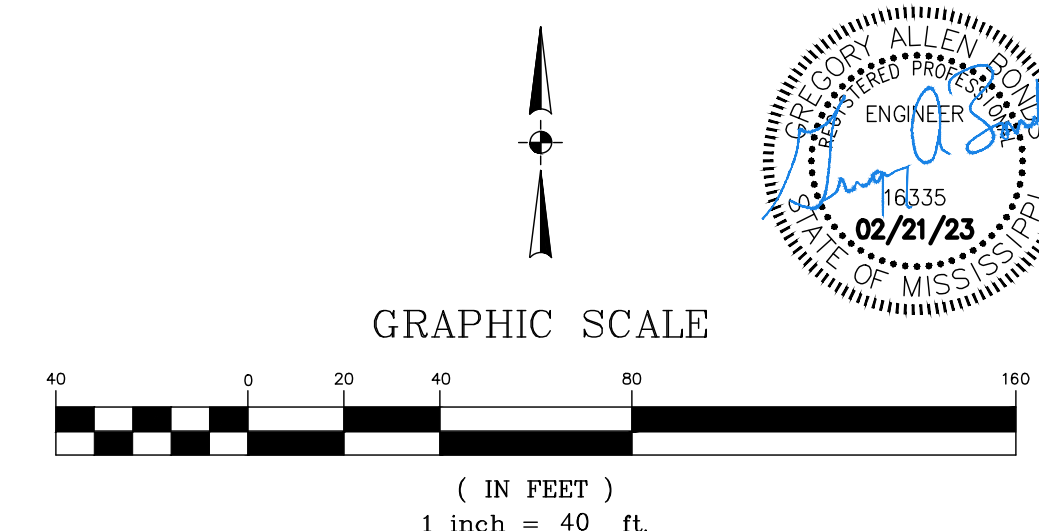
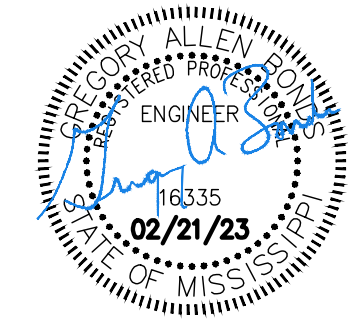
- NOTE:
- CONTRACTOR SHALL SEE ARCHITECTURAL PLANS OR PLANS FOR MORE INFORMATION ON STEPS, HANDRAILS, DOWNSPOUTS, BOLLARDS, SIDEWALK JOINT PATTERNS, ETC.
  - INFORMATION SHOWN ON THIS SHEET INSIDE THE BUILDING (IF ANY) ARE FOR INFORMATIONAL PURPOSES AND ARE NOT INTENDED TO BE USED FOR BUILDING CONSTRUCTION.
  - SEE SHEET C100 FOR ADDITIONAL CONSTRUCTION NOTES AND INFORMATION.
  - SEE TYPICAL SECTION ON SHEET 400 FOR MORE DETAILS ON PAVING REQUIREMENTS.
  - RADIAL DIMENSIONS ARE MEASURED TO BACK OF CURB OR EDGE OF PAVEMENT IF NO CURB IS PROPOSED.
  - PARKING LOT DIMENSION ARE TO THE EDGE OF PAVEMENT OR TO THE FACE OF SIDEWALK CURB.
  - PARKING SPACE STRIPING & ACCESS ROAD CENTERLINE STRIPING SHALL BE PAINTED WITH FAST DRYING SOLVENT BASED TRAFFIC PAINT FOR USE ON BITUMINOUS AND PORTLAND CEMENT CONCRETE PAVEMENT. PAINT SHALL MEET THE REQUIREMENTS OF SECTIONS 625 & 710 OF THE LATEST EDITION OF THE MISSISSIPPI STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION UNLESS OTHERWISE NOTED AS THERMOPLASTIC.
  - PARKING SPACE STRIPES SHALL BE 4" IN WIDTH.
  - PAINT SHALL BE APPLIED WITH MECHANICAL EQUIPMENT TO PRODUCE PAVEMENT MARKINGS OF DIMENSIONS AND COLORS INDICATED, WITH UNIFORM STRAIGHT EDGES. APPLY AT MANUFACTURER'S RECOMMENDED RATES TO PROVIDE A MINIMUM WET FILM THICKNESS OF 15 MILS AND ONLY AT MANUFACTURER'S RECOMMENDED AMBIENT AND SURFACE TEMPERATURES.
  - STOP BARS, WORDING, DIRECTIONAL ARROWS SHALL BE THERMOPLASTIC.
  - NO STRIPING (PAINT OR THERMOPLASTIC) SHALL BE INSTALLED UNTIL AT LEAST 7 DAYS AFTER THE FINAL ASPHALT SURFACE HAS BEEN INSTALLED.

02/21/23

CONSTRUCTION DOCUMENTS  
 WBA # 22-056

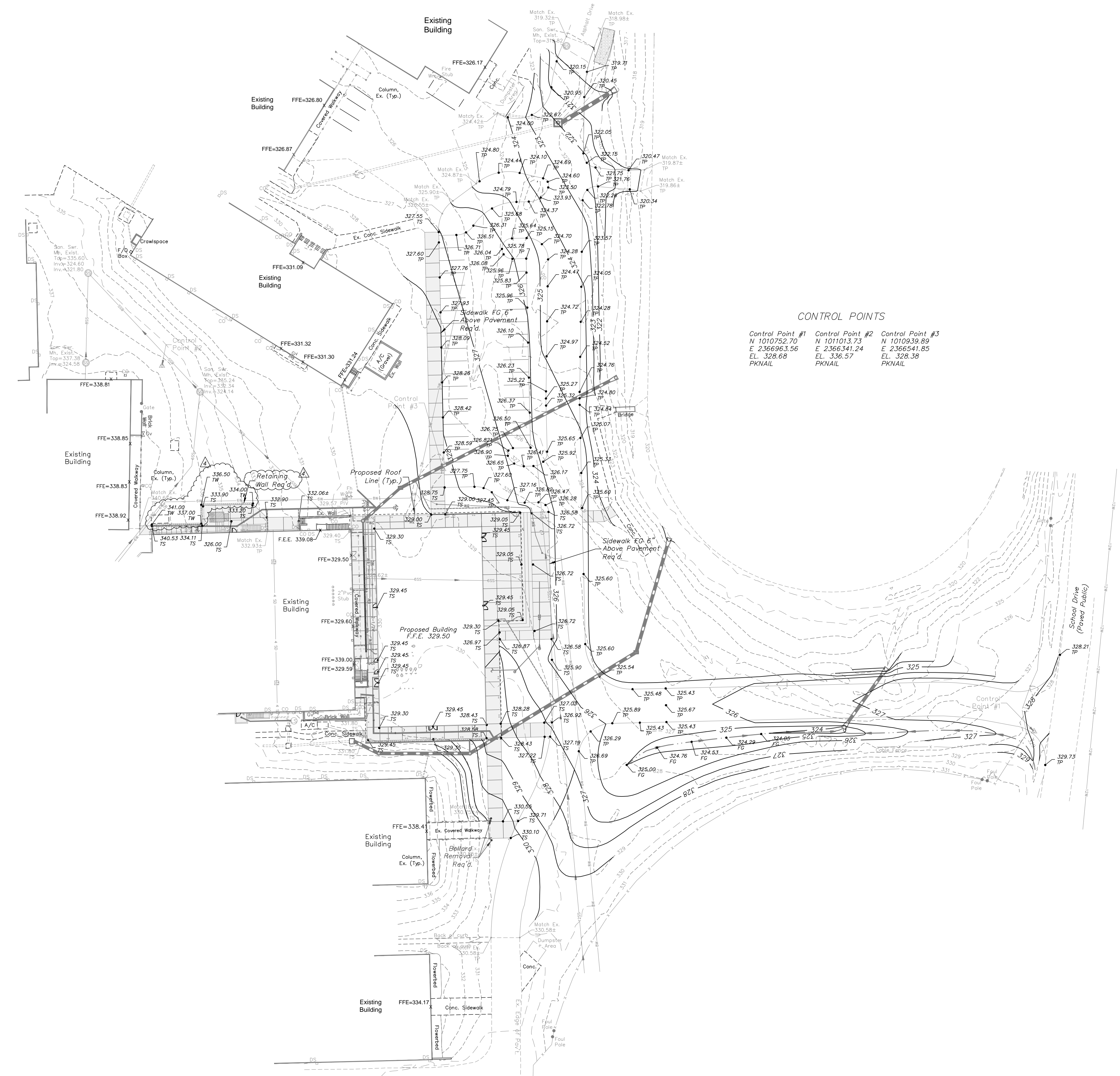
REVISIONS

NO.	DESCRIPTION	DATE
4	RET. WALL	03/30/23
	SW HATCH	



1 SITE PLAN  
 1" = 40'-0"

**C201**  
 SITE PLAN

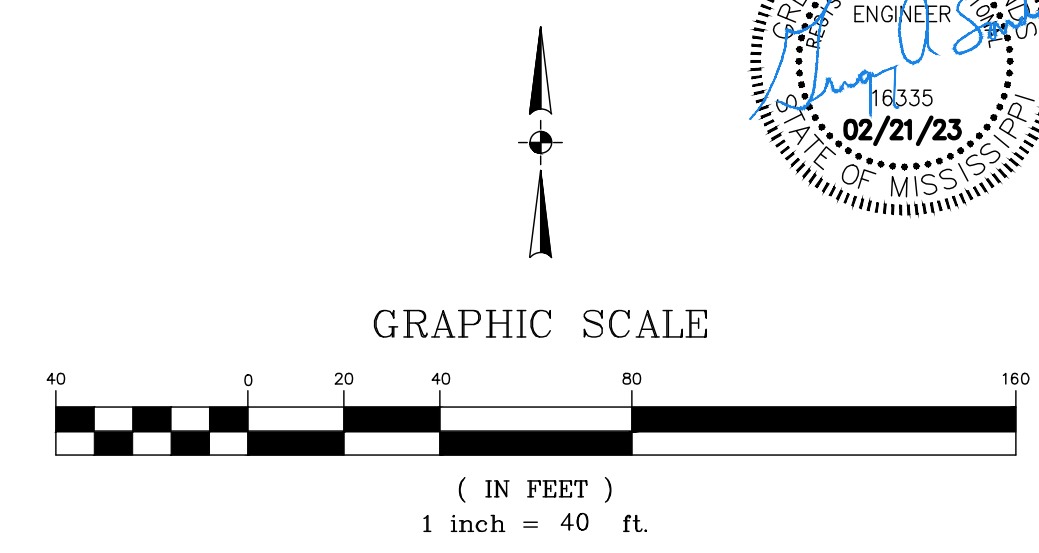
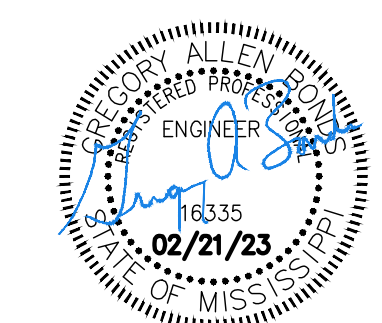


- NOTES:
- SEE CONSTRUCTION NOTES ON SHEET C100 & PROJECT SPECIFICATIONS FOR MORE INFORMATION.
  - STRUCTURAL OR SELECT FILL MATERIALS SHALL CONSIST OF APPROVED MATERIALS WITH LESS THAN 2% ORGANIC MATTER, FREE OF DEBRIS, WITH ROCKS NO GREATER THAN 6" AND A LIQUID LIMIT LESS THAN 40 AND PLASTICITY INDEX BETWEEN 10 & 20.
  - EXISTING UTILITIES SHOWN SHALL BE CONSIDERED APPROXIMATE. ADDITIONAL UTILITY LINES THAT ARE NOT SHOWN ON THE DRAWING MAY LIE WITHIN THE PROJECT AREA. CONTRACTOR TO CONTACT MISSISSIPPI ONE CALL AT 811 FOR A UTILITY LOCATE PRIOR TO BEGINNING CONSTRUCTION.
  - CONTRACTOR TO VERIFY EXISTING LOCATION AND ELEVATION OF ALL UTILITY INFRASTRUCTURE REQUIRED FOR COMPLETION OF THIS PROJECT IN FULL PRIOR TO BEGINNING ANY ASPECT OF CONSTRUCTION. THIS INCLUDES ALL ON-SITE AND OFF-SITE UTILITIES AS REQUIRED. SHOULD ANY DISCREPANCIES BE FOUND THEY SHALL BE IMMEDIATELY BROUGHT TO THE ENGINEER'S ATTENTION IN WRITING TO RECEIVE FURTHER INSTRUCTION.
  - CONTRACTOR SHALL COORDINATE ALL WORK DIRECTLY INVOLVING CROSSING OR IN THE VICINITY OF AN EXISTING UTILITY LINE WITH UTILITY PROVIDER/OWNER.
  - SEE ARCHITECTURAL PLANS & NOTES ON SHEET C400 FOR MORE DETAILS ON SIDEWALKS AND HANDICAPPED RAMPS.
  - SLOPES THAT ARE GREATER THAN 3:1 SHALL RECEIVE SOLID SOO.
  - ALL DISTURBED AREAS OUTSIDE THE SCOPE OF WORK ARE TO BE REPAIRED TO AS GOOD AS THE ORIGINAL CONDITION OR BETTER AT NO CHARGE TO THE OWNER. PICTURE DOCUMENTATION OF THESE AREAS SHALL BE PROVIDED BY THE CONTRACTOR PRIOR TO DISTURBING.
  - THIS PROJECT IS LOCATED FLOOD ZONE X ACCORDING TO FEMA FLOOD INSURANCE MAP 28121C0188F, EFFECTIVE DATE 06/09/2014.

02/21/23

CONSTRUCTION DOCUMENTS  
 WBA # 22-056

NO.	DESCRIPTION	DATE
4	ADDED SPOT ELEV.	03/30/23



1 GRADING PLAN  
 1" = 40'-0"

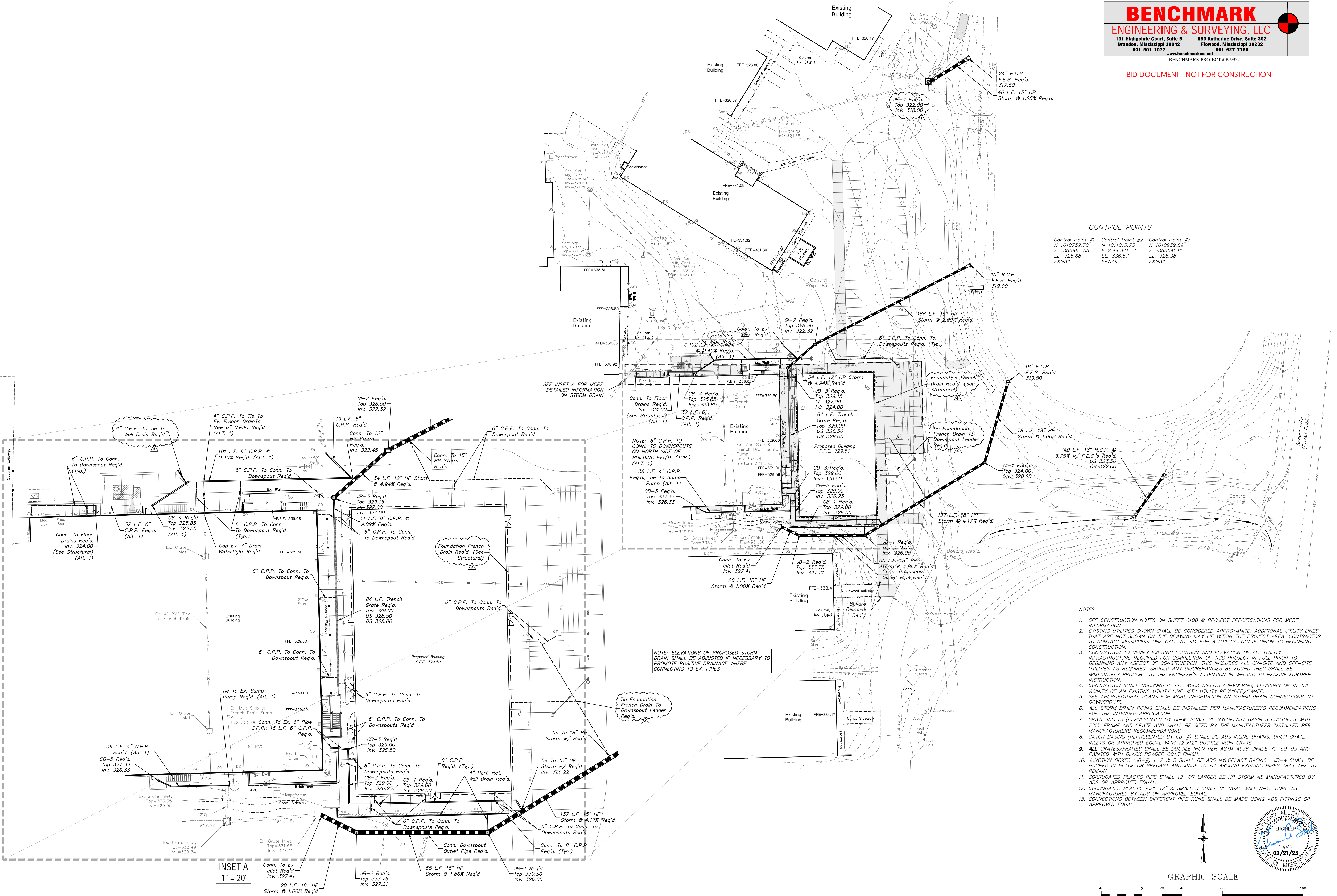
BID DOCUMENT - NOT FOR CONSTRUCTION

PPSD JH  
 MULTIPURPOSE

200 Mary Ann Drive  
 Pearl, MS 39208

**CONTROL POINTS**

Control Point #1	Control Point #2	Control Point #3
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E 2366963.56	E 2366341.24	E 2366541.85
EL. 328.68	EL. 336.57	EL. 328.38
PKNAIL	PKNAIL	PKNAIL



SEE INSET A FOR MORE DETAILED INFORMATION ON STORM DRAIN

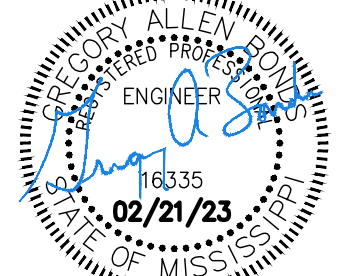
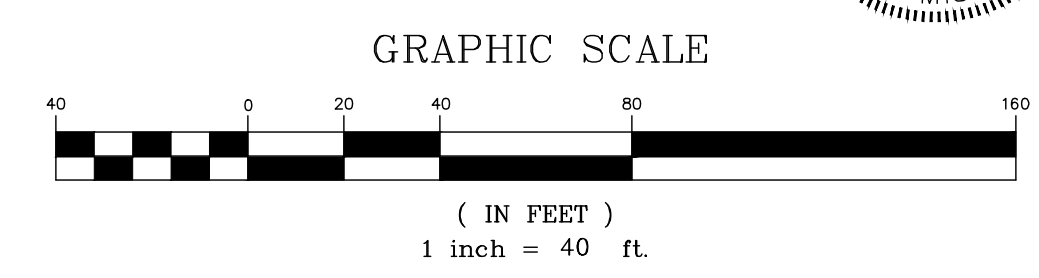
NOTE: 6" C.P.P. TO CONN. TO DOWNSPOUTS ON NORTH SIDE OF BUILDING REQ'D. (TYP.) (ALT. 1)

NOTE: ELEVATIONS OF PROPOSED STORM DRAIN SHALL BE ADJUSTED IF NECESSARY TO PROMOTE POSITIVE DRAINAGE WHERE CONNECTING TO EX. PIPES

- NOTES:**
- SEE CONSTRUCTION NOTES ON SHEET C100 & PROJECT SPECIFICATIONS FOR MORE INFORMATION.
  - EXISTING UTILITIES SHOWN SHALL BE CONSIDERED APPROXIMATE. ADDITIONAL UTILITY LINES THAT ARE NOT SHOWN ON THE DRAWING MAY LIE WITHIN THE PROJECT AREA. CONTRACTOR TO CONTACT MISSISSIPPI ONE CALL AT 811 FOR A UTILITY LOCATE PRIOR TO BEGINNING CONSTRUCTION.
  - CONTRACTOR TO VERIFY EXISTING LOCATION AND ELEVATION OF ALL UTILITY INFRASTRUCTURE REQUIRED FOR COMPLETION OF THIS PROJECT IN FULL PRIOR TO BEGINNING ANY ASPECT OF CONSTRUCTION. THIS INCLUDES ALL ON-SITE AND OFF-SITE UTILITIES AS REQUIRED. SHOULD ANY DISCREPANCIES BE FOUND, THEY SHALL BE IMMEDIATELY BROUGHT TO THE ENGINEER'S ATTENTION IN WRITING TO RECEIVE FURTHER INSTRUCTION.
  - CONTRACTOR SHALL COORDINATE ALL WORK DIRECTLY INVOLVING CROSSING OR IN THE VICINITY OF AN EXISTING UTILITY LINE WITH UTILITY PROVIDER/OWNER.
  - SEE ARCHITECTURAL PLANS FOR MORE INFORMATION ON STORM DRAIN CONNECTIONS TO DOWNSPOUTS.
  - ALL STORM DRAIN PIPING SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS FOR THE INTENDED APPLICATION.
  - GRATE INLETS (REPRESENTED BY GI-#) SHALL BE NYLOPLAST BASIN STRUCTURES WITH 2'X3' FRAME AND GRATE AND SHALL BE SIZED BY THE MANUFACTURER INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.
  - CATCH BASINS (REPRESENTED BY CB-#) SHALL BE ADS INLINE DRAINS, DROP GRATE INLETS OR APPROVED EQUAL WITH 12"x12" DUCTILE IRON GRATE.
  - ALL GRATES/FRAMES SHALL BE DUCTILE IRON PER ASTM A536 GRADE 70-50-05 AND PAINTED WITH BLACK POWDER COAT FINISH.
  - JUNCTION BOXES (JB-#) 1, 2 & 3 SHALL BE ADS NYLOPLAST BASINS. JB-4 SHALL BE POURED IN PLACE OR PRECAST AND MADE TO FIT AROUND EXISTING PIPES THAT ARE TO REMAIN.
  - CORRUGATED PLASTIC PIPE SHALL 12" OR LARGER BE HP STORM AS MANUFACTURED BY ADS OR APPROVED EQUAL.
  - CORRUGATED PLASTIC PIPE 12" & SMALLER SHALL BE DUAL WALL N-12 HDPE AS MANUFACTURED BY ADS OR APPROVED EQUAL.
  - CONNECTIONS BETWEEN DIFFERENT PIPE RUNS SHALL BE MADE USING ADS FITTINGS OR APPROVED EQUAL.

INSET A  
 1" = 20'

1 STORM DRAIN LAYOUT  
 1" = 40'-0"



02/21/23

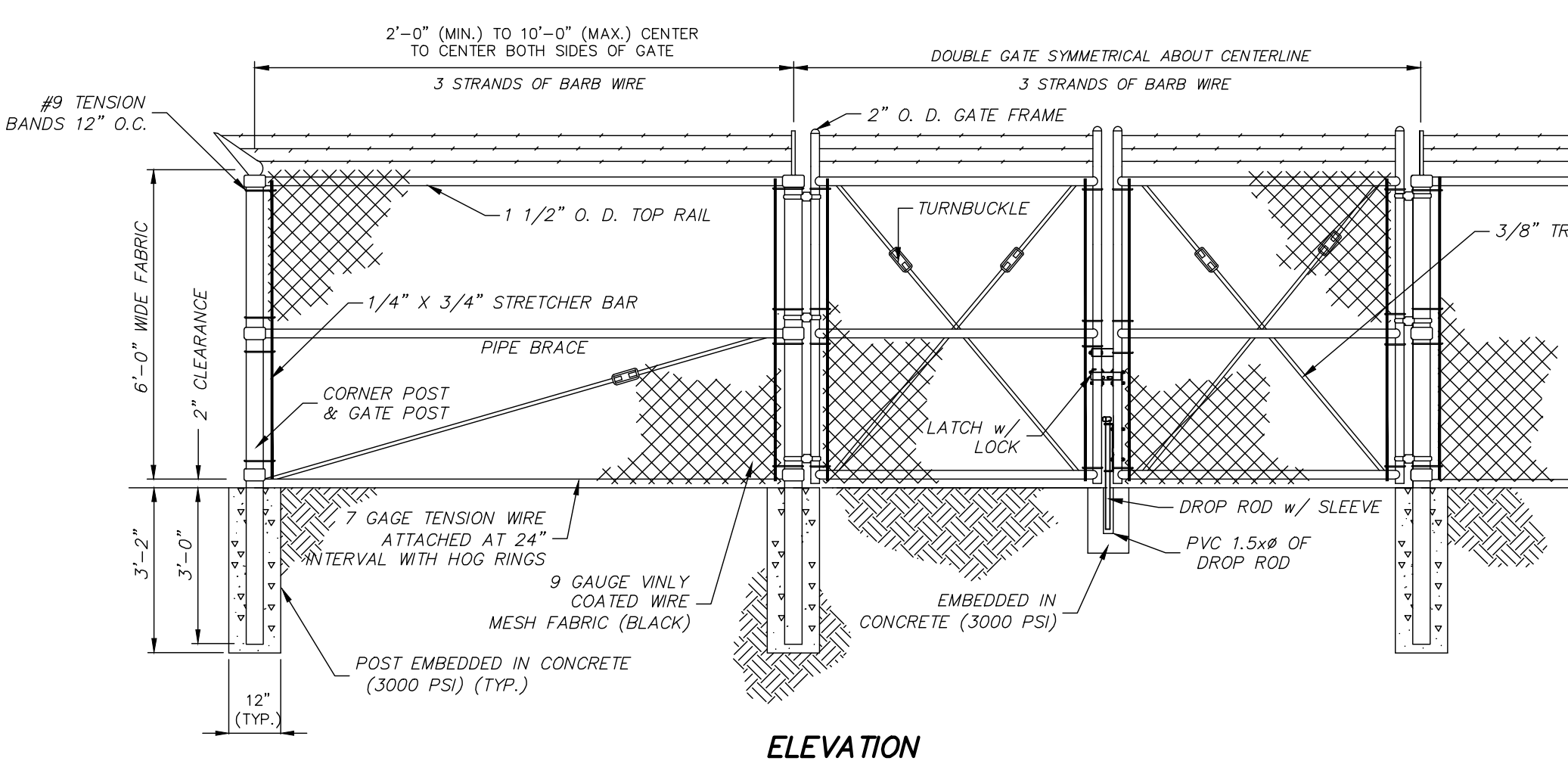
CONSTRUCTION DOCUMENTS  
 WBA # 22-056

REVISIONS

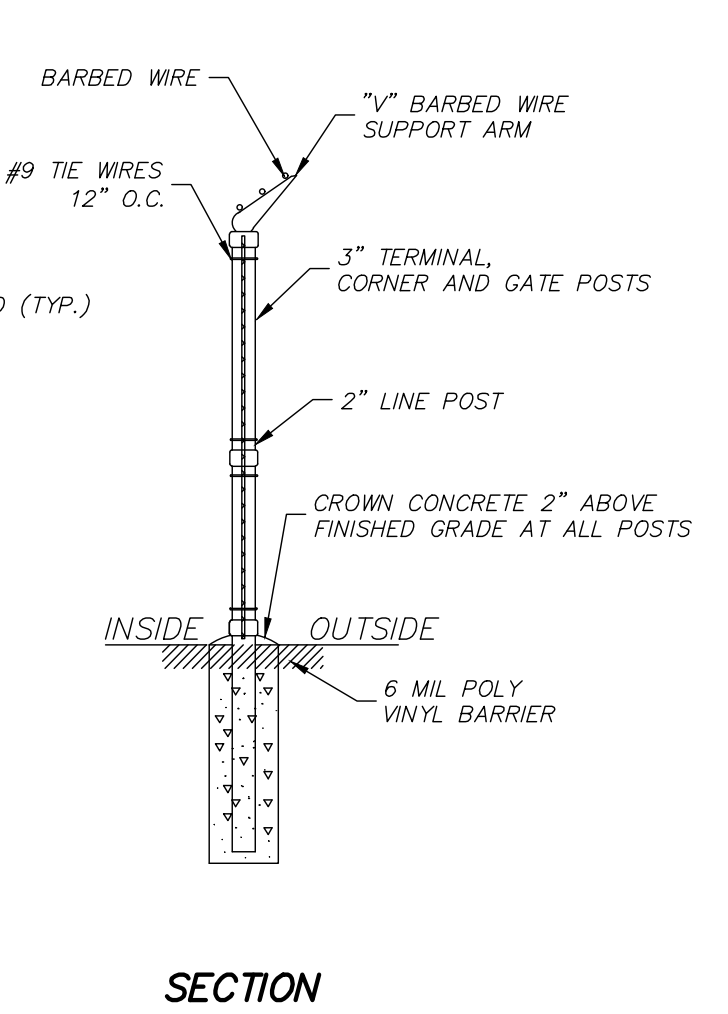
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4	MISC.	03/30/23

**C302**  
 STORM DRAIN LAYOUT

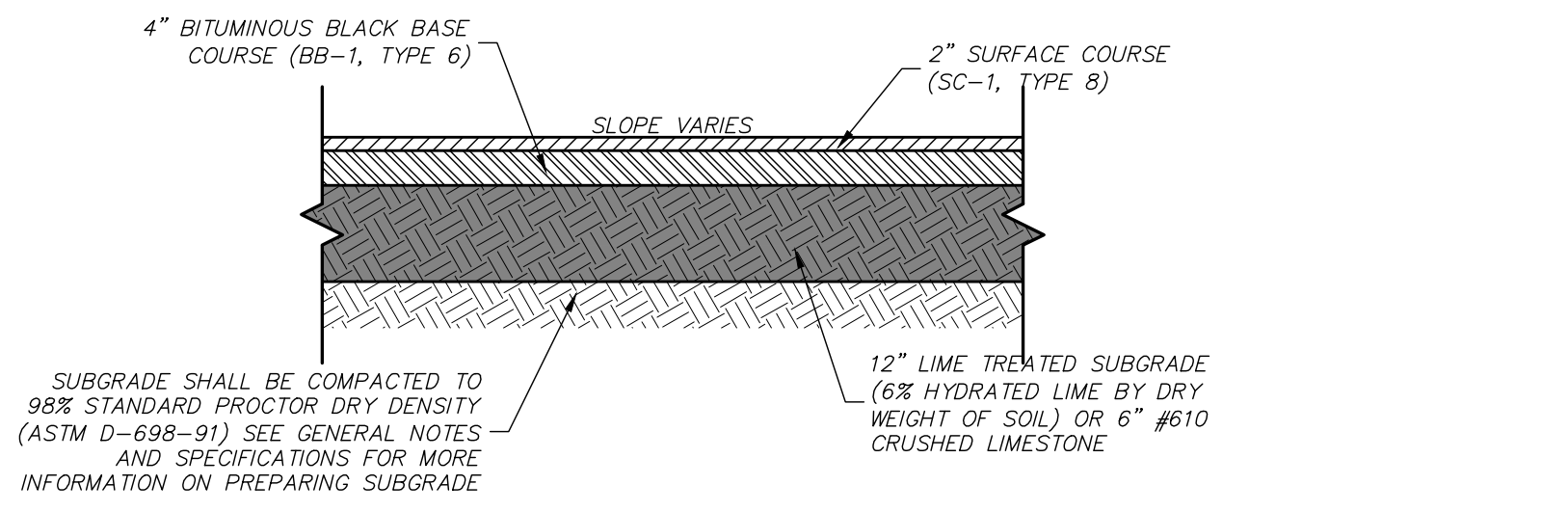
BID DOCUMENT - NOT FOR CONSTRUCTION



ELEVATION



SECTION

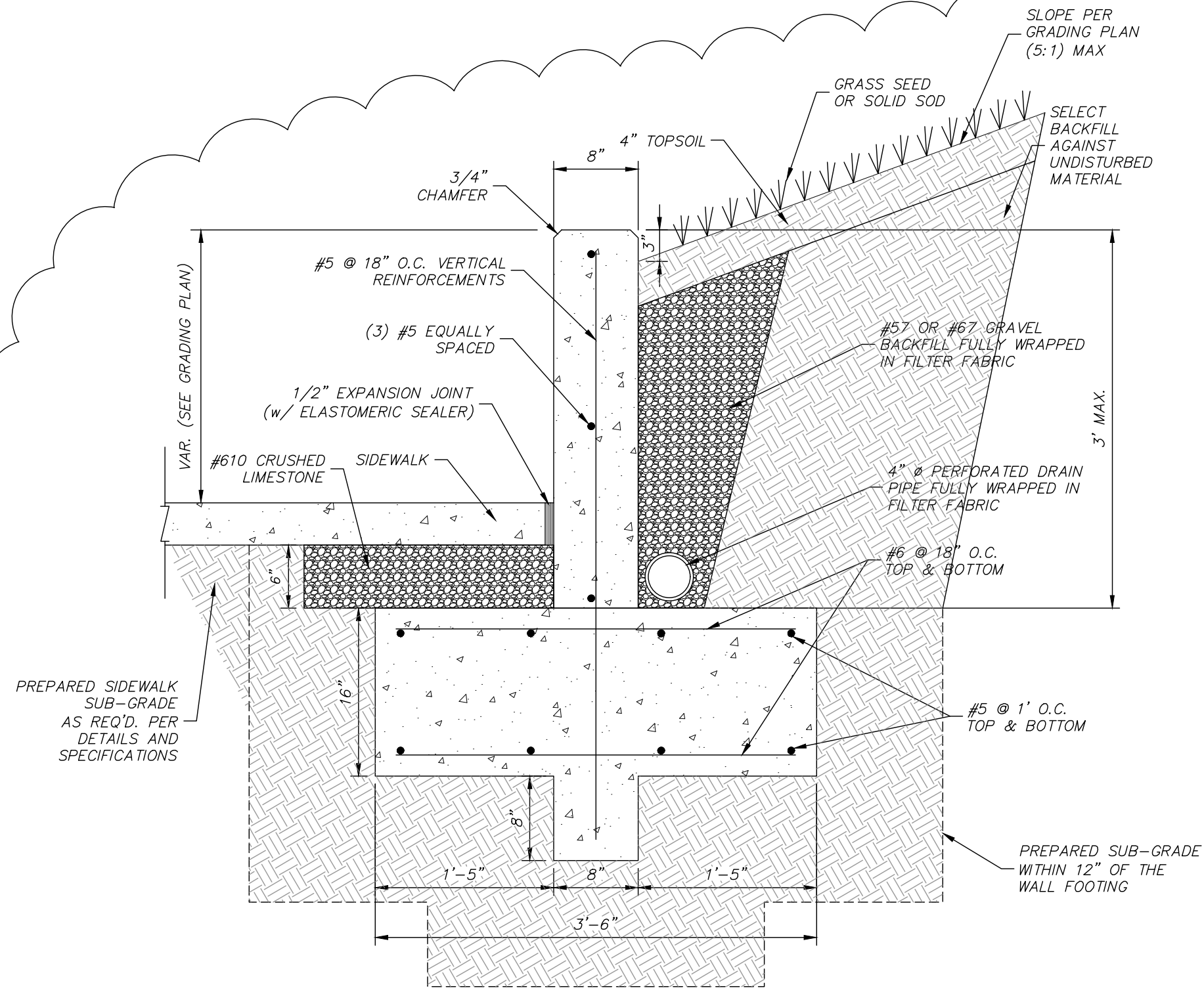


TYPICAL LIGHT DUTY PAVEMENT SECTION

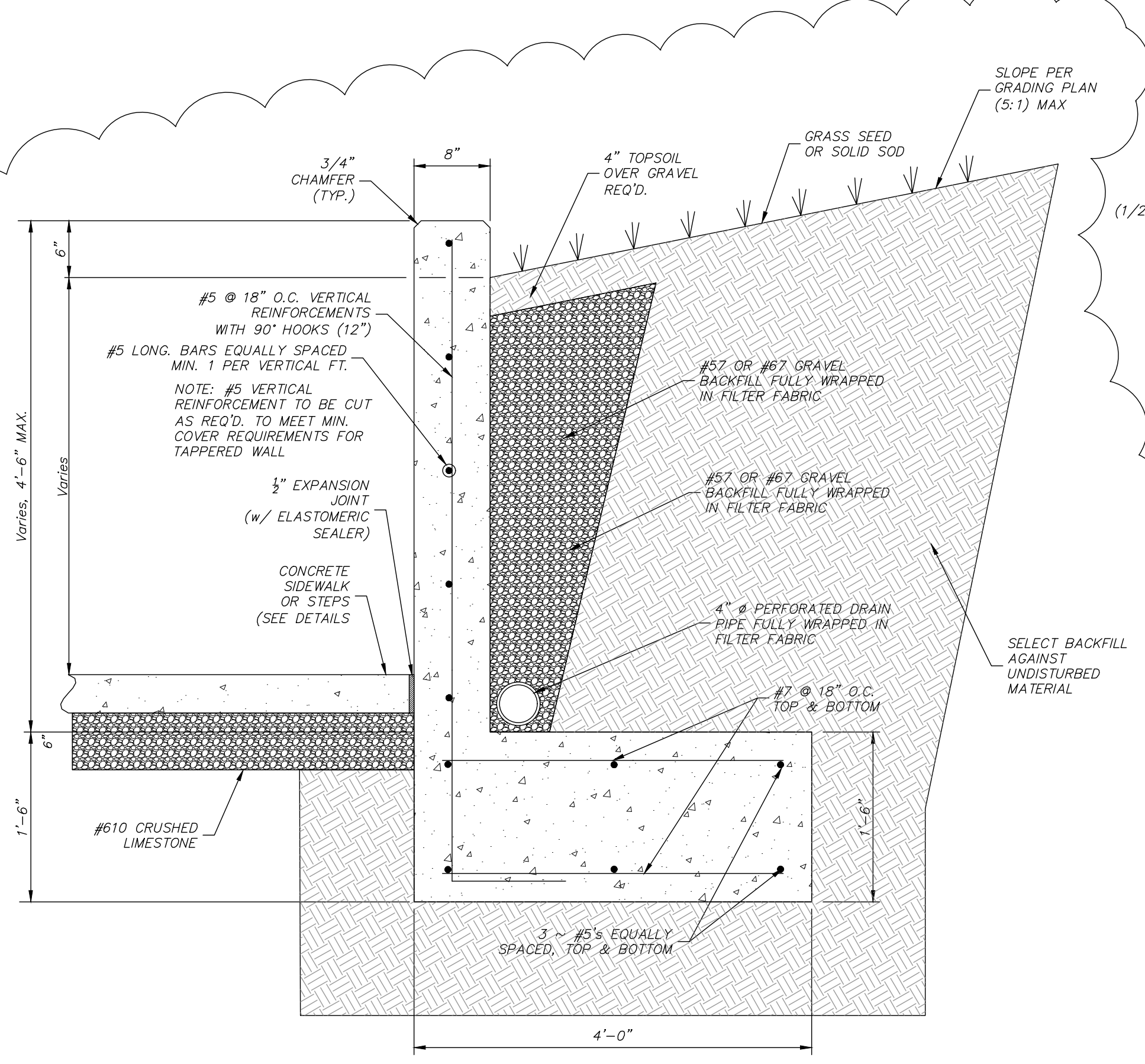
- TYPICAL SECTION NOTES:
- SEE SHEET 2 & PROJECT SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS OF ROADWAY CONSTRUCTION.
  - SUB-BASE MAY BE 12" OF LIME TREATED MATERIAL MEETING SPECIFICATIONS FOR SELECT MATERIAL OR 6" OF #610 CRUSHED LIMESTONE.
  - ALL FILL MATERIAL SHALL MEET REQUIREMENTS NOTED ON OTHER SHEETS AND IN PROJECT SPECIFICATIONS.
  - SUB-GRADE SHALL BE PREPARED WITH SELECT FILL MATERIAL MEETING REQUIRED VERTICAL AND HORIZONTAL BUFFER (3') FROM ANY EXPANSIVE MATERIALS (CH) COMPACTED TO 98% STANDARD PROCTOR DRY DENSITY PER ASTM D-698.
  - CONTRACTOR TO COORDINATE WITH THE PROJECT ENGINEER FOR A PROOF ROLL OF THE SUB-GRADE OF PAVED AREAS, HOWEVER, PRIOR TO THE CONTRACTOR CONTACTING THE ENGINEER FOR A PROOF ROLL, IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE WITH THE EARTHWORK TESTING AGENCY AND REQUEST THAT THEY PROVIDE ALL TEST RESULTS TO ENGINEER AND TO CERTIFY TO THE ENGINEER THAT ALL REQUIREMENTS OF THE CONSTRUCTION PLANS, SPECIFICATIONS AND GEOTECHNICAL REPORT IN RELATION TO THE PREPARATION OF THE ROADWAY SUB-GRADE HAVE BEEN MET OR EXCEEDED. ONCE THIS INFORMATION IS RECEIVED, ENGINEER WILL SCHEDULE PROOF ROLL WITH CONTRACTOR. ONCE THE PROOF ROLL PASSES THE ENGINEER'S INSPECTION AND PRIOR TO THE NEXT RAIN EVENT, CONTRACTOR MAY BEGIN PAVING OPERATIONS.

- FENCE NOTES:
- FENCING TO BE TIED TO RAILS AND BRACING WITH 9 GAGE TIE WIRES @ 24" O.C.
  - ALL FABRIC, FENCE COMPONENTS, POSTS, GATE COMPONENTS AND FASTENERS SHALL BE BLACK VINYL COATED.
  - FABRIC SHALL BE 2" MESH FABRIC (9 GAGE WIRES)
  - PIPE SIZES SHOWN FOR POSTS, BRACES AND RAILS ARE NOMINAL INSIDE DIAMETER.
  - MESH TO BE ATTACHED AT 24" INTERVALS TO BRACES, RAILS AND TENSION WIRES WITH HOG RINGS OR OTHER APPROVED METHOD.
  - GATE SHALL HAVE A LOCKABLE LATCH, PLUNGER AND ROD WITH A CENTER STOP.
  - ALL JOINTS FOR GATE SHALL BE WELDED TO MAKE SOLID FRAME.
  - CORNER, TERMINAL AND LINE POSTS THICKNESS SHALL MEET INDUSTRY STANDARD FOR PROPOSED APPLICATION.

6" CHAIN LINK FENCE w/ BARBED WIRE

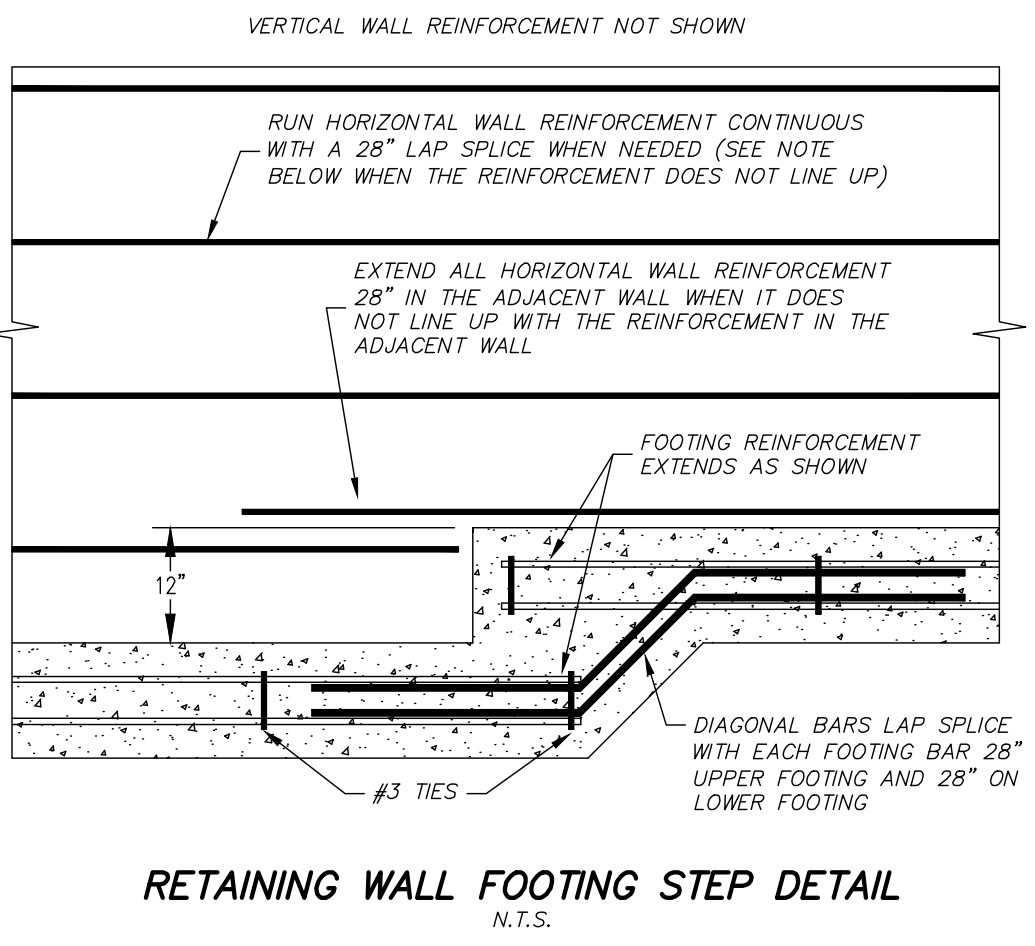


SECTION OF RETAINING WALL ADJACENT TO SIDEWALK AT SOUTH END OF BUILDING

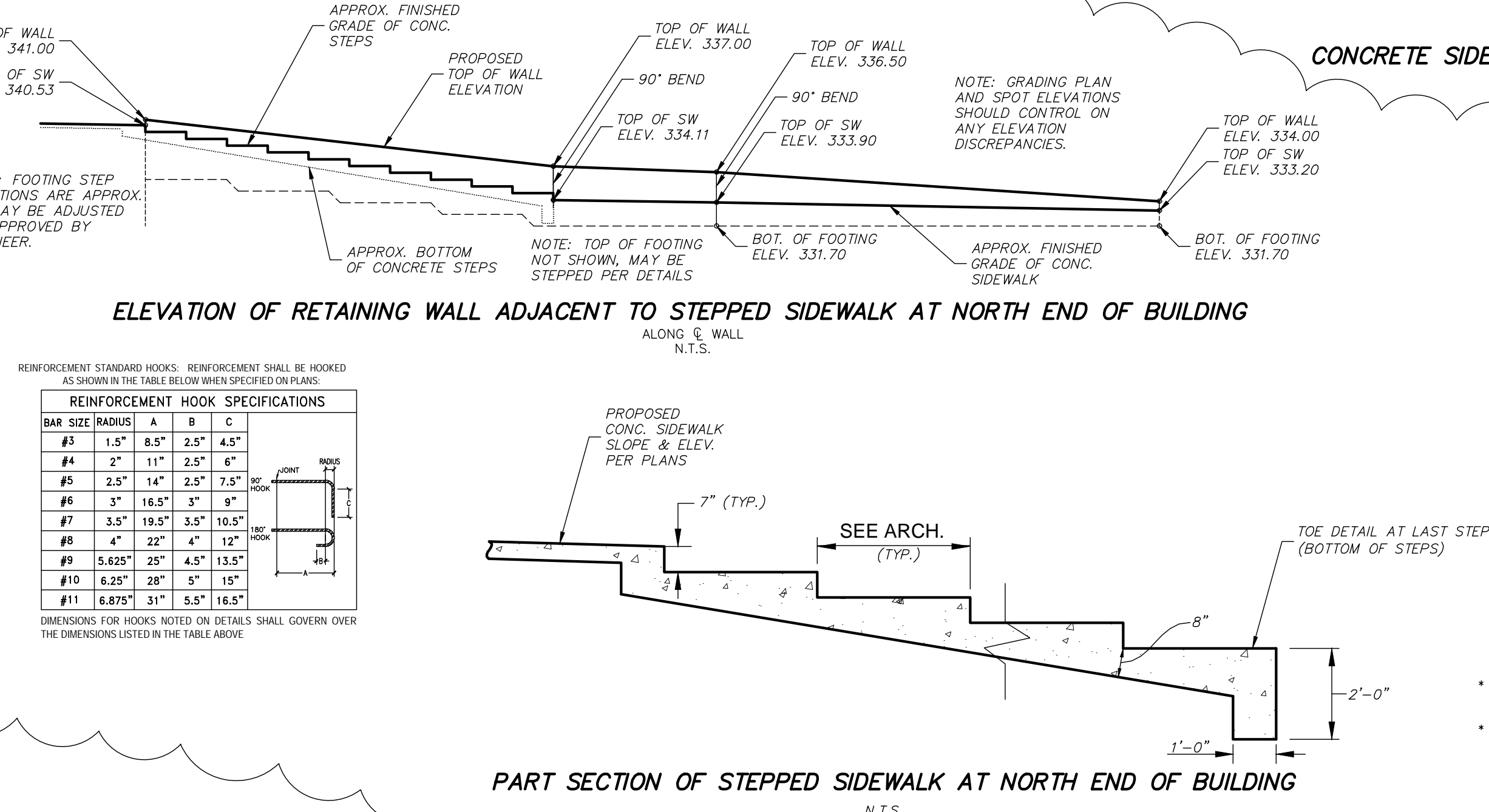


SECTION OF RETAINING WALL ADJACENT TO STEPPED SIDEWALK AT NORTH END OF BUILDING

- CONCRETE & REINFORCING STEEL NOTES:
- FOOTING AND VERTICAL WALL SHALL BE PORTLAND CEMENT CONCRETE WITH A NORMAL WEIGHT OF 145 PCF AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI (ASTM C 39/C 39M) WITH NO ADMIXTURES.
  - THE FOOTING/KEY SHALL NOT BEAR ON EXPANSIVE CLAYS (CH) MATERIALS. IF SUCH MATERIALS ARE ENCOUNTERED WHILE EXCAVATING FOR THE FOOTING, SUCH MATERIALS SHALL BE REMOVED AND REPLACED WITH SELECT FILL AND COMPACTED TO 98% STANDARD PROCTOR DRY DENSITY.
  - ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301-05 PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE.
  - USE ASTM C 150 TYPE I OR II CEMENT. FLY ASH CONFORMING TO ASTM C 618 MAY BE USED IN THE MIX.
  - AGGREGATES SHALL CONFORM TO ASTM C 33, OR HAVE SHOWN BY SPECIAL TESTS OR ACTUAL SERVICE TO PRODUCE CONCRETE OF ADEQUATE STRENGTH AND THE NOMINAL MAXIMUM SIZE OF AGGREGATE SHALL NOT BE LARGER THAN 1/5 THE NARROWEST DIMENSION BETWEEN SIDES OF FORMS, 1/3 THE DEPTH OF SLABS, NOR 3/4 THE MINIMUM CLEAR SPACING BETWEEN INDIVIDUAL REINFORCING BARS OR WIRES.
  - CONCRETE SHALL BE HAVE AN AIR CONTENT OF 4.5% (+/- 1.5%)
  - THE CONCRETE SLUMP SHALL BE 4" AT THE POINT OF PLACEMENT. THE USE OF WATER-REDUCING ADMIXTURES MAY BE USED IF AN INCREASED SLUMP IS DESIRED FOR WORKABILITY. THE SLUMP SHALL NOT EXCEED 5" WITH THE USE OF NORMAL WATER-REDUCERS & SHALL NOT EXCEED 7" WITH THE USE OF MID-RANGE OR HIGH-RANGE WATER-REDUCERS.
  - WHEN CONCRETE ARRIVES AT THE POINT OF PLACEMENT WITH A SLUMP BELOW WHAT WILL RESULT IN THE SPECIFIED SLUMP AT THE POINT OF PLACEMENT AND IS UNSUITABLE FOR PLACING AT THAT SLUMP, A ONE-TIME SLUMP ADJUSTMENT MAY BE MADE IN ORDER TO ACHIEVE THE SPECIFIED SLUMP BY ADDING WATER UP TO THE ALLOWABLE AMOUNT AS DOCUMENTED IN THE ACCEPTED MIXTURE PROPORTIONS. THE SPECIFIED SLUMP AND WATER-CEMENTITIOUS MATERIAL RATIO SHALL NOT BE EXCEEDED. ALL TESTING SHALL BE COMPLETED AFTER THE ADDITION OF WATER. THE ADDITION OF WATER SHALL BE IN ACCORDANCE WITH ASTM C 94.8.
  - THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONCRETE TESTING. ALL TESTING SHALL BE IN COMPLIANCE WITH THE REQUIREMENTS OF ACI 301-05. TESTING SHALL INCLUDE SLUMP, COMPRESSIVE STRENGTH, TEMPERATURE, AND AIR CONTENT. THREE TEST CYLINDERS SHALL BE MOLDED AND CURED AT LEAST ONCE PER DAY AND FOR EVERY 75 CUBIC YARDS OF CONCRETE. ONE CYLINDER SHALL BE TESTED AT 7 DAYS AND ONE SHALL BE TESTED AT 28 DAYS. THE REMAINING CYLINDER SHALL BE AVAILABLE AS A BACKUP.
  - THE CONCRETE MIX DESIGN, STRENGTH, AND ALL SPECIFIED TESTING SHALL BE FULLY DOCUMENTED AND SUBMITTED TO THE ENGINEER FOR APPROVAL.
  - ALL FORMWORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301-05. FORMS SHALL NOT BE REMOVED UNTIL 48 HOURS AFTER CONCRETE PLACEMENT.
  - REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60. WELDED WIRE REINFORCEMENT SHALL CONFORM TO ASTM A62 AND A185. MINIMUM CLEAR COVER FOR REINFORCEMENT SHALL BE 2".
  - SELECT FILL MATERIAL FOR FOOTING SHALL MEET REQUIREMENTS FOR SELECT MATERIAL PER THE CONTRACT DOCUMENTS.



RETAINING WALL FOOTING STEP DETAIL



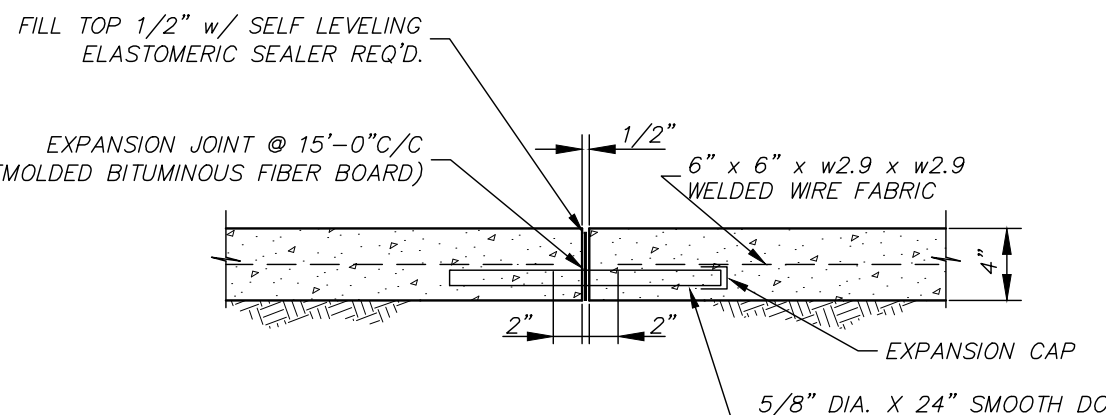
ELEVATION OF RETAINING WALL ADJACENT TO STEPPED SIDEWALK AT NORTH END OF BUILDING

REINFORCEMENT STANDARD HOOKS: REINFORCEMENT SHALL BE HOOKED AS SHOWN IN THE TABLE BELOW WHEN SPECIFIED ON PLANS.

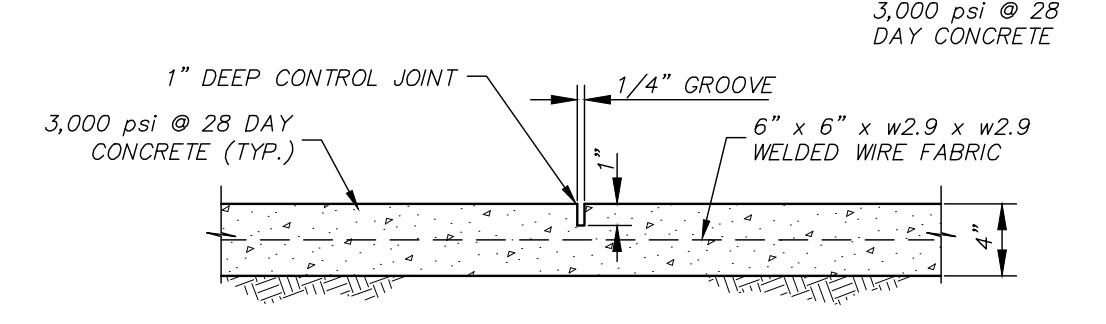
BAR SIZE	RADIUS	A	B	C
#3	1.5"	8.5"	2.5"	4.5"
#4	2"	11"	2.5"	6"
#5	2.5"	14"	2.5"	7.5"
#6	3"	16.5"	3"	9"
#7	3.5"	19.5"	3.5"	10.5"
#8	4"	22"	4"	12"
#9	5.025"	25"	4.5"	13.5"
#10	6.25"	28"	5"	15"
#11	6.875"	31"	5.5"	16.5"

DIMENSIONS FOR HOOKS NOTED ON DETAILS SHALL GOVERN OVER THE DIMENSIONS LISTED IN THE TABLE ABOVE.

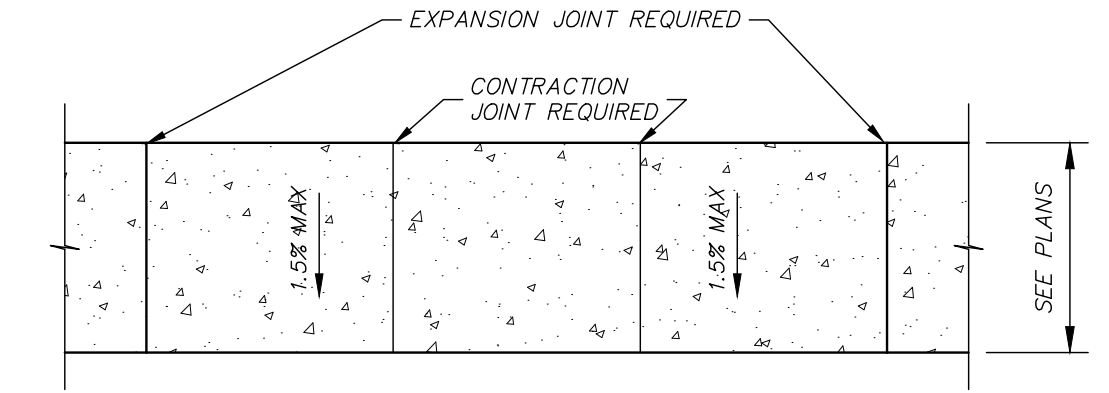
PART SECTION OF STEPPED SIDEWALK AT NORTH END OF BUILDING



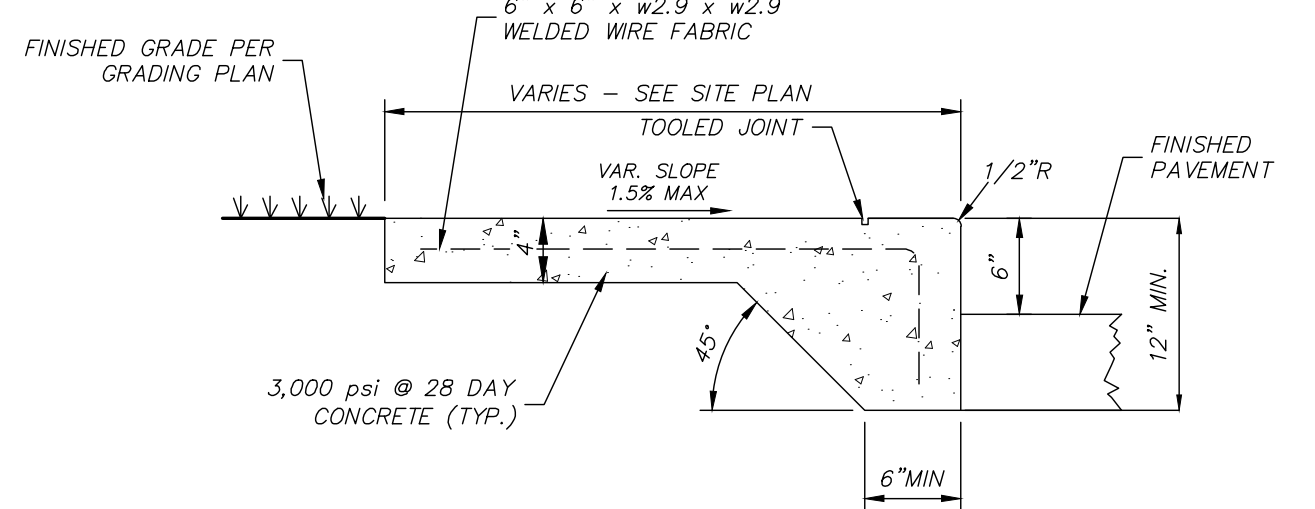
SIDEWALK EXPANSION JOINT DETAIL



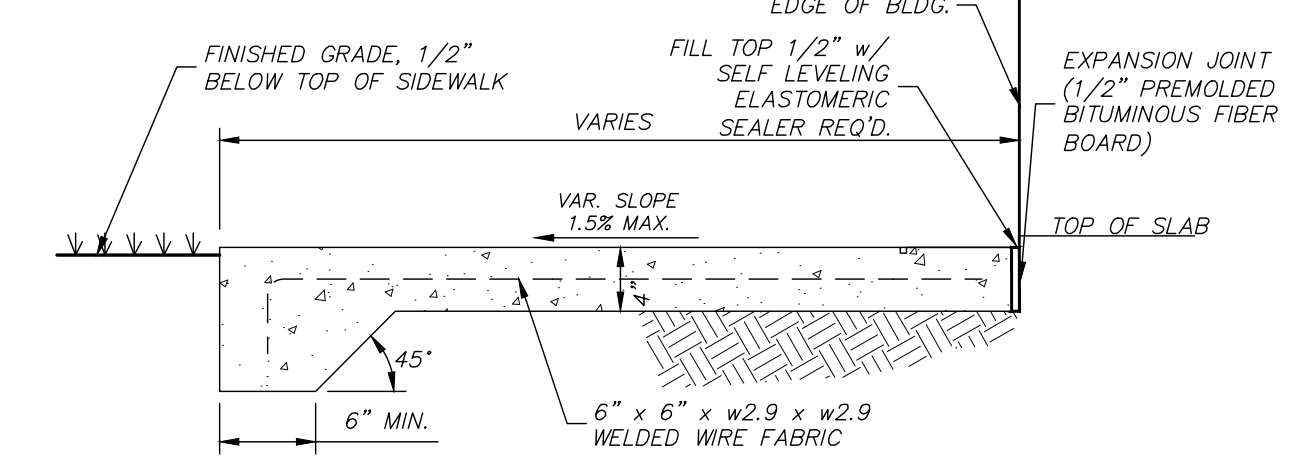
SIDEWALK CONTRACTION JOINT DETAIL



SIDEWALK JOINT LAYOUT DETAIL



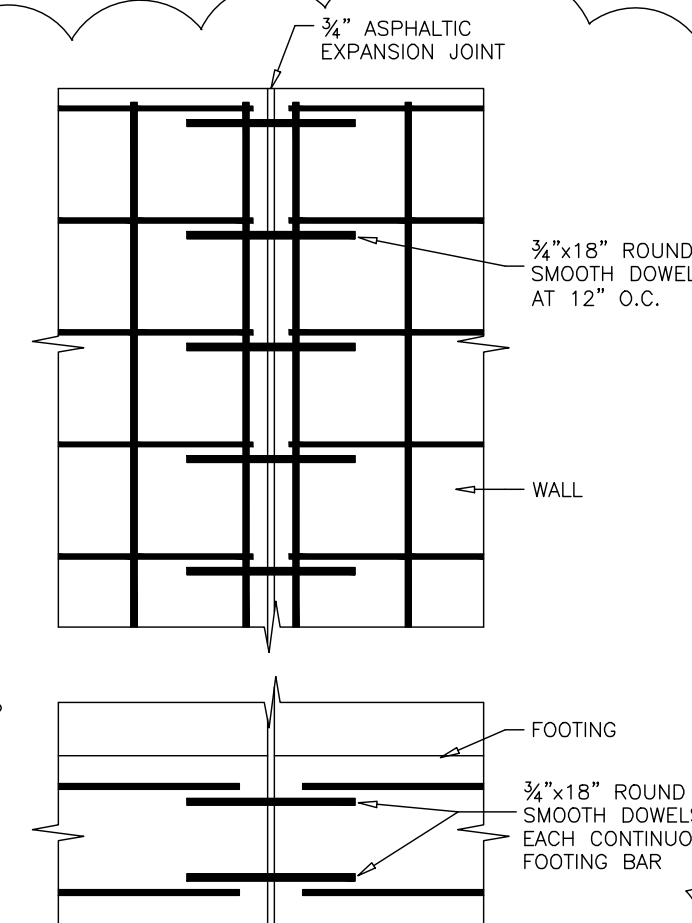
SIDEWALK AT EDGE OF PAVEMENT



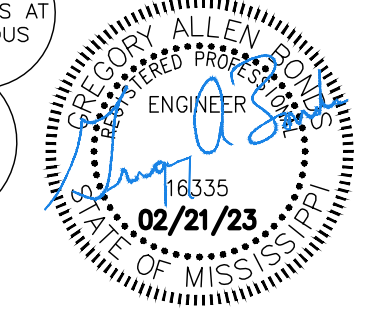
SIDEWALK EDGE ALONG GRASSED AREAS

- SIDEWALK CONSTRUCTION NOTES:
- SUB-GRADE FOR SIDEWALK SHALL BE PREPARED IN SAME MANNER AS REQUIRED FOR PAVEMENT CONSTRUCTION.
  - EXPANSION JOINT REQ'D. WHERE TYING SIDEWALK TO BUILDING OR BACK OF CURB WITH 3" PREMOLDED BITUMINOUS FIBER BOARD AND TOP FILLED WITH 1" SELF-LEVELING ELASTOMERIC SEALER.
  - WELDED WIRE FABRIC SHALL BE SHEETS AND NOT ROLLS.
  - CONTRACTION JOINTS REQUIRED AT 5' O.C. & EXPANSION JOINTS REQUIRED AT 15' O.C. UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
  - CONTRACTION CONTROL JOINTS SHALL BE TOOLED OR SAWCUT IN STRAIGHT LINES TO THE DEPTHS SHOWN AT THE PROPER CURE TIME AND WITH THE PROPER MATERIALS/EQUIPMENT TO LEAVE THE FINISHED CONCRETE SURFACE ALONG THE JOINTS LOOKING SMOOTH AND FREE FROM CHIPPING OR OTHER DEFECTS. JOINTS WHERE CHIPPING OR OTHER IRREGULARITIES THAT COMPROMISE THE LOOK OF THE CONCRETE PER THE OWNER/ENGINEER SHALL BE REJECTED AND SUBSEQUENTLY REMOVED AND REPLACED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
  - CONTRACTOR SHALL SEE SPOT ELEVATIONS ON GRADING LAYOUT FOR SIDEWALK SLOPES.

CONCRETE SIDEWALK DETAILS



- CONTROL JOINTS MUST BE INSTALLED AT A MAXIMUM SPACING OF 25' O.C.
- HORIZONTAL WALL & FOOTING REINFORCEMENT MUST NOT RUN THROUGH THE CONTROL JOINT.



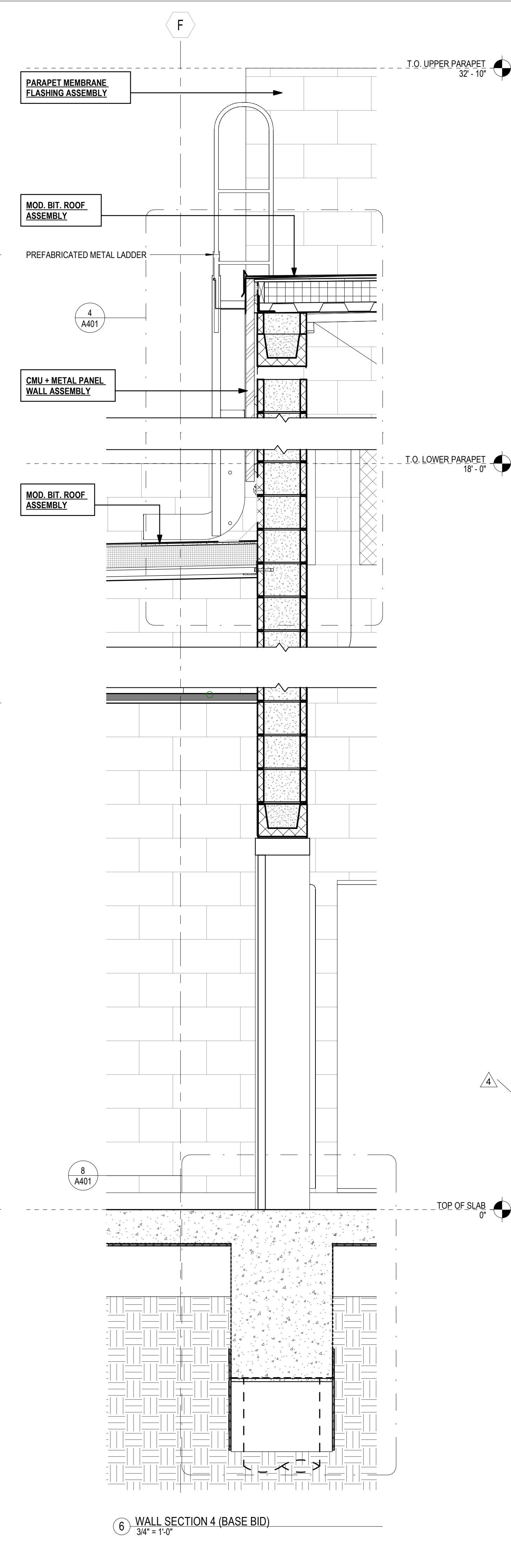
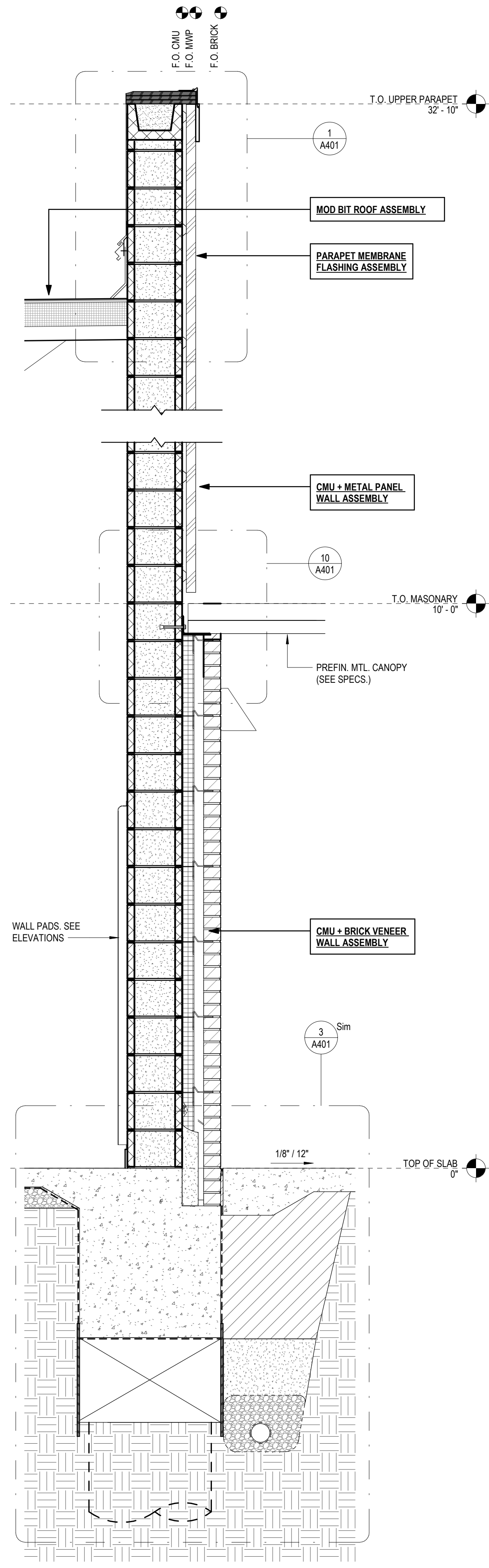
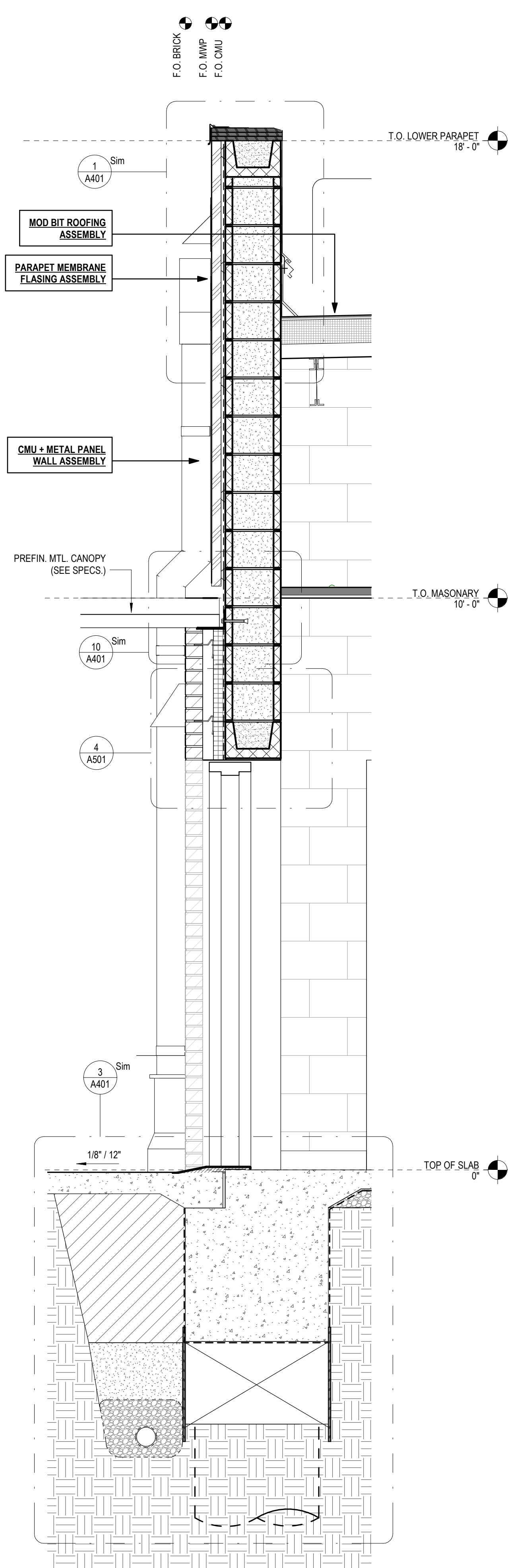
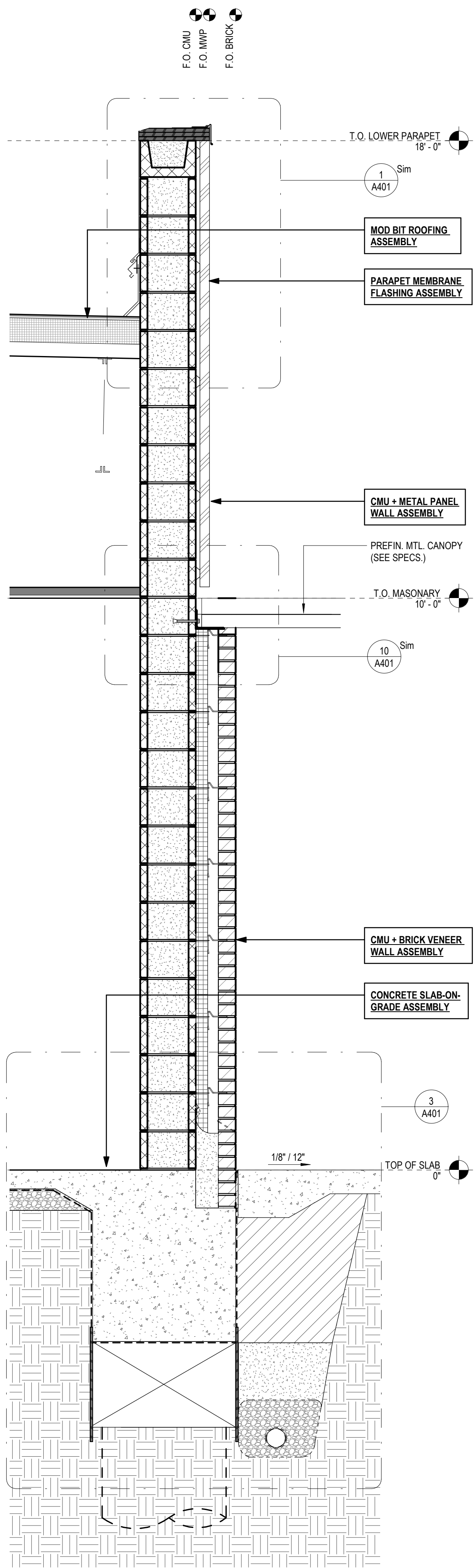
02/21/23

CONSTRUCTION DOCUMENTS  
 WBA # 22-056

REVISIONS  
 NO. DESCRIPTION DATE  
 4 RET. WALL 03/30/23

C400  
 GENERAL CONST. NOTES





**ROOF ASSEMBLIES:**

**MODIFIED BITUMINOUS ROOFING ASSEMBLY**  
 -2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM  
 -COVER BOARD  
 -5" MINIMUM RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS / TAPERED INSULATION)  
 -METAL ROOF DECK (REFER TO STRUCTURAL)

**PARAPET MEMBRANE FLASHING ASSEMBLY**  
 (SEE SECTION DETAILS)  
 -FULLY ADHERED CONT. TPO MEMBRANE SYSTEM  
 -CONT. SHEET MTL. GRAVEL GUARD @ T.O. WALL  
 -CONT. SHEET MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERMINATION BAR.

**WALL ASSEMBLIES:**

**CMU AND BRICK VENEER WALL ASSEMBLY**  
 -BRICK MASONRY VENEER  
 -AIR SPACE  
 -MASONRY TIES @ 16" O.C. MAX. VERT. & 24" O.C. MAX. HORIZ. TYP.  
 -CONT. 2-1/2" RIGID INSULATION (R=10)  
 -CONT. WEATHER BARRIER  
 -12" CMU (REFER TO STRUCTURAL FOR ADDITIONAL REQUIREMENTS)

**CMU AND METAL PANEL WALL ASSEMBLY**  
 -INSULATED METAL WALL PANEL SYSTEM  
 -CONTINUOUS 7/8" FURRING CHANNEL RUNNING HORIZONTAL @ 2' O.C. VERTICAL  
 -CONT. WEATHER BARRIER  
 -12" CMU (REFER TO STRUCTURAL FOR ADDITIONAL REQUIREMENTS)

**CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY**  
 -BRICK MASONRY VENEER  
 -AIR SPACE  
 -MASONRY TIES @ 16" O.C. MAX. VERT. & 24" O.C. MAX. HORIZ. TYP.  
 -CONT. WEATHER BARRIER  
 -CAST-IN-PLACE FOUNDATION WALL (REFER TO STRUCTURAL)

**FOUNDATION ASSEMBLIES:**

**TYP. SLAB-ON-GRADE ASSEMBLY**  
 -CONCRETE FOUNDATION SYSTEM (REFER TO STRUCTURAL)  
 -UNDER SLAB VAPOR BARRIER  
 -GRAVEL FILL (REFER TO STRUCTURAL AND CONFIRM WITH GEO-TECH)  
 -COMPACTED SUBGRADE

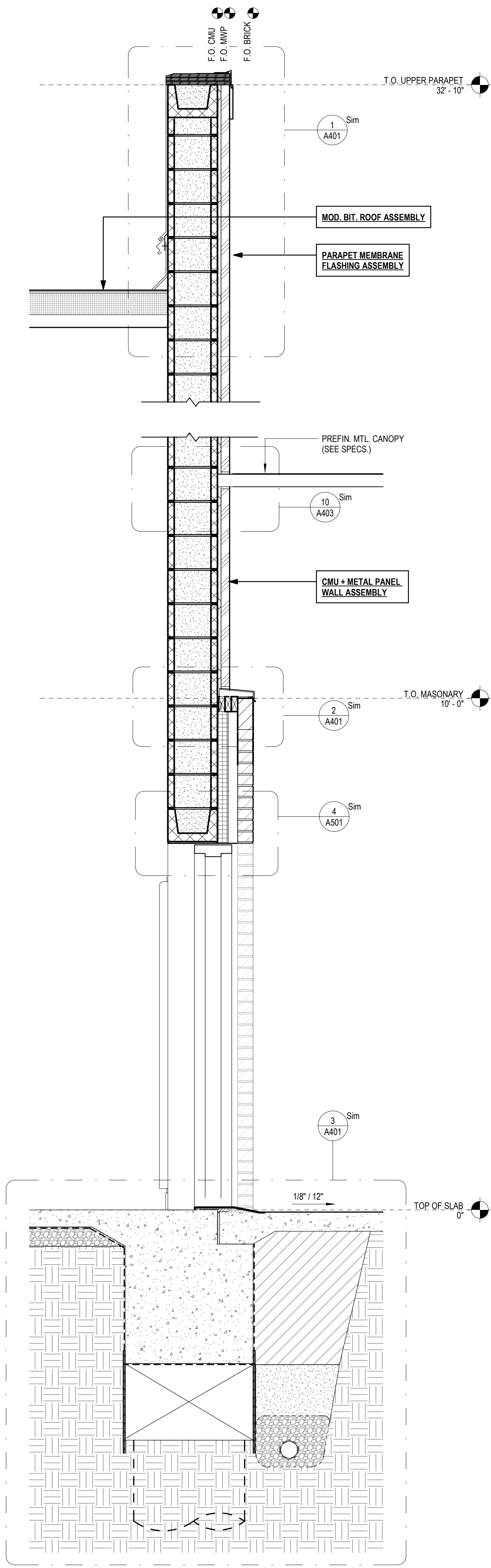
**TYP. FOUNDATION DRAINAGE SYSTEM ASSEMBLY**  
 -MTL. S.S. CAP FLASHING (SEAL TOP EDGE) ANCHOR @ 12" O.C. MAX. TYP.  
 -WEATHER BARRIER  
 -PROTECTION BOARD  
 -DRAINAGE MAT  
 -GRAVEL FILL ENCAPSULATED IN FILTER FABRIC  
 -4" PERF. FOUNDATION DRAIN WRAPPED IN FILTER FABRIC (COORD. W/ CIVIL & PLUMBING DRAWINGS)  
 -HIGH POINT OF FRENCH DRAIN TO BE 1" BELOW THE VOID FORM. TYP. SLOPE PIPE TO DRAIN. SEE CIVIL FOR STORM CONNECTION.

\*REFER TO SPECIFICATION SECTIONS FOR ADDITIONAL DETAILED REQUIREMENTS\*

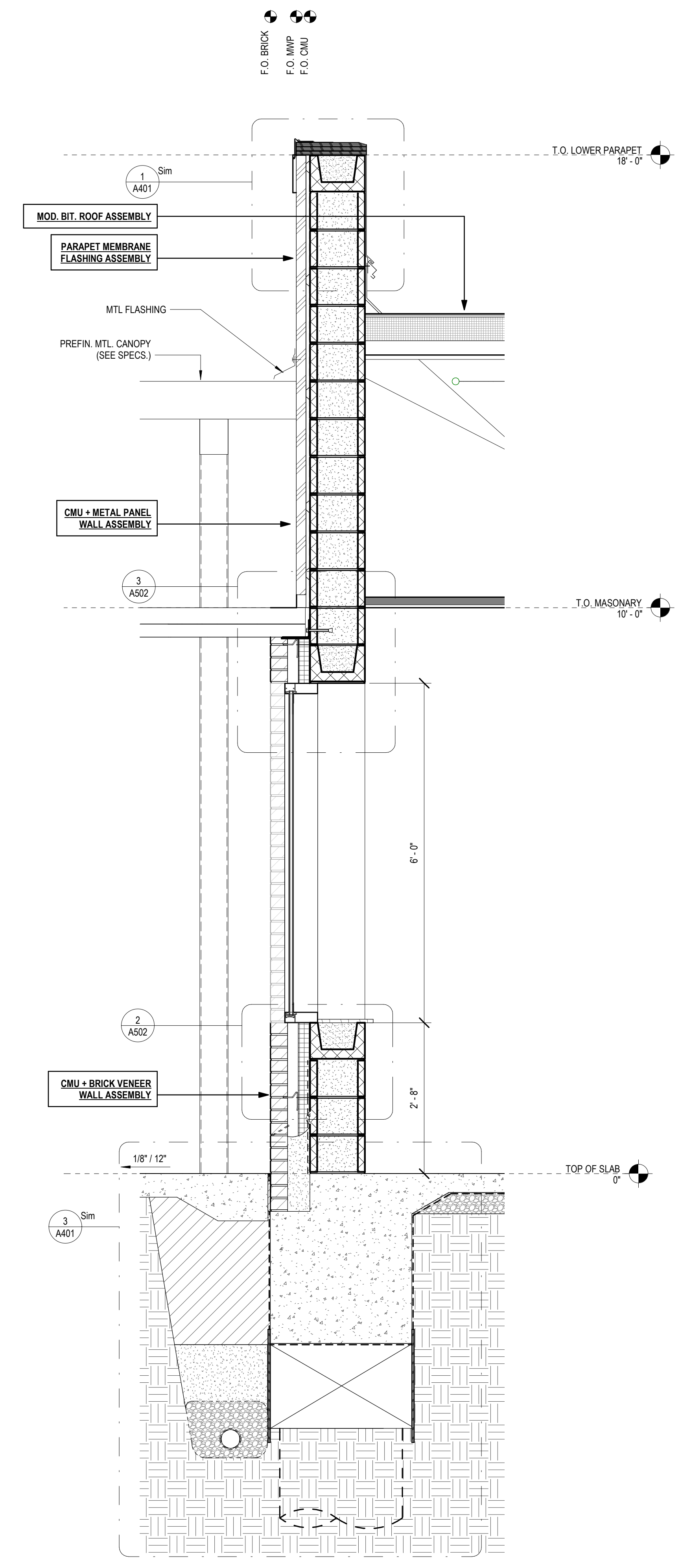
02/21/23

CONSTRUCTION DOCUMENTS  
WBA # 22-056

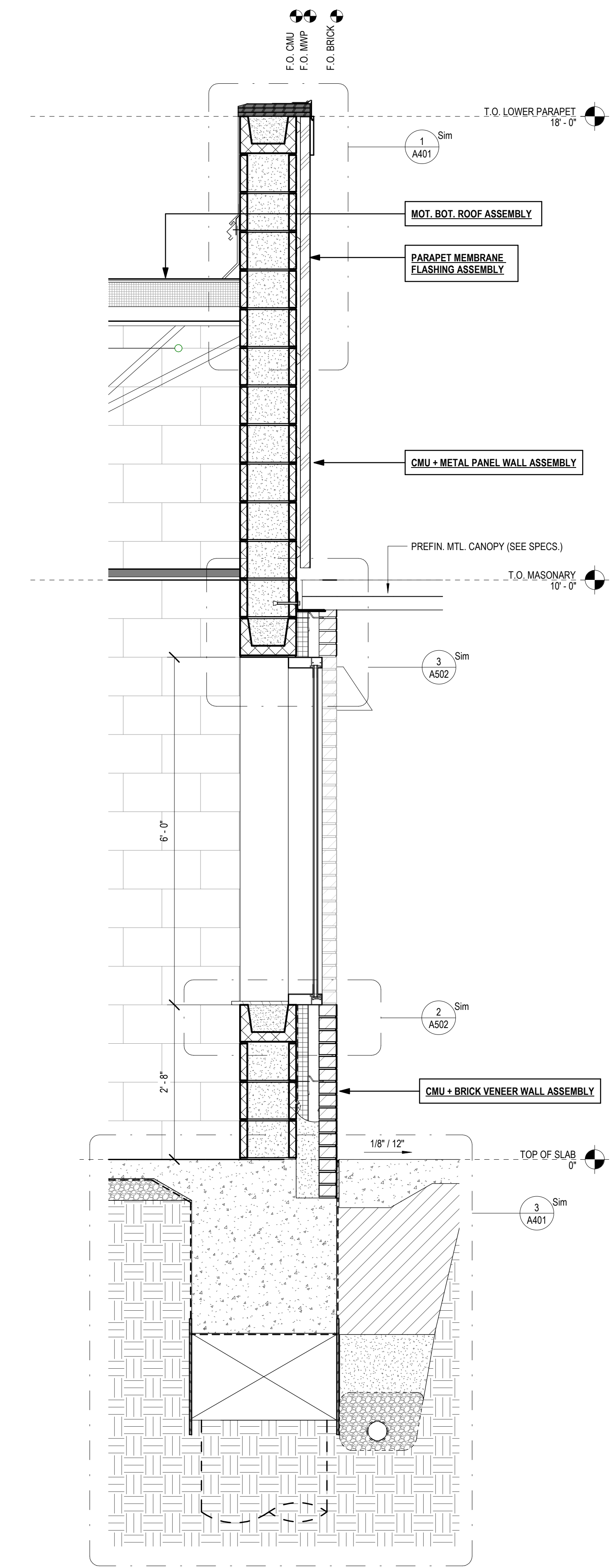
REVISIONS		
NO.	DESCRIPTION	DATE
4	Revision 4	3.30.2023



1 WALL SECTION 5 (BASE BID)  
3/4" = 1'-0"



3 WALL SECTION 6 (BASE BID)  
3/4" = 1'-0"



4 WALL SECTION 7 (BASE BID)  
3/4" = 1'-0"

**ROOF ASSEMBLIES:**

**MODIFIED BITUMINOUS ROOFING ASSEMBLY**

- 2 PLY SBS MODIFIED BITUMEN ROOFING SYSTEM
- COVER BOARD
- 5" MINIMUM RIGID INSULATION (SEE ROOF PLAN FOR AREAS REQ. CRICKETS / TAPERED INSULATION.)
- METAL ROOF DECK (REFER TO STRUCTURAL)

**PARAPET MEMBRANE FLASHING ASSEMBLY**  
(SEE SECTION DETAILS)

- FULLY ADHERED CONT. TPO MEMBRANE SYSTEM.
- CONT. SHEET MTL. GRAVEL GUARD @ T.O. WALL
- CONT. SHEET MTL. REGLET & COUNTER FLASHING ASSEMBLY ABOVE TERMINATION BAR.

**WALL ASSEMBLIES:**

**CMU AND BRICK VENEER WALL ASSEMBLY**

- BRICK MASONRY VENEER
- AIR SPACE
- MASONRY TIES @ 16" O.C. MAX. VERT. & 24" O.C. MAX. HORIZ. TYP.
- CONT. 2-1/2" RIGID INSULATION (R=10)
- CONT. WEATHER BARRIER
- 12" CMU (REFER TO STRUCTURAL FOR ADDITIONAL REQUIREMENTS)

**CMU AND METAL PANEL WALL ASSEMBLY**

- INSULATED METAL WALL PANEL SYSTEM
- CONTINUOUS 7/8" FURRING CHANNEL RUNNING HORIZONTAL @ 2' O.C. VERTICAL
- CONT. WEATHER BARRIER
- 12" CMU (REFER TO STRUCTURAL FOR ADDITIONAL REQUIREMENTS)

**CONCRETE FOUNDATION WALL W/ BRICK VENEER ASSEMBLY**

- BRICK MASONRY VENEER
- AIR SPACE
- MASONRY TIES @ 16" O.C. MAX. VERT. & 24" O.C. MAX. HORIZ. TYP.
- CONT. WEATHER BARRIER
- CAST-IN-PLACE FOUNDATION WALL (REFER TO STRUCTURAL)

**FOUNDATION ASSEMBLIES:**

**TYP. SLAB-ON-GRADE ASSEMBLY**

- CONCRETE FOUNDATION SYSTEM (REFER TO STRUCTURAL)
- UNDER SLAB VAPOR BARRIER
- GRAVEL FILL (REFER TO STRUCTURAL AND CONFIRM WITH GEO-TECH)
- COMPACTED SUBGRADE

**TYP. FOUNDATION DRAINAGE SYSTEM ASSEMBLY**

- MTL. S.S. CAP FLASHING (SEAL TOP EDGE) ANCHOR @ 12" O.C. MAX. TYP.
- WEATHER BARRIER
- PROTECTION BOARD
- DRAINAGE MAT
- GRAVEL FILL ENCAPSULATED IN FILTER FABRIC
- 4" PERF. FOUNDATION DRAIN WRAPPED IN FILTER FABRIC (COORD. W/ CIVIL & PLUMBING DRAWINGS)
- HIGH POINT OF FRENCH DRAIN TO BE 1" BELOW THE VOID FORM. TYP. SLOPE PIPE TO DRAIN. SEE CIVIL FOR STORM CONNECTION.

\*REFER TO SPECIFICATION SECTIONS FOR ADDITIONAL DETAILED REQUIREMENTS\*

02/21/23

CONSTRUCTION DOCUMENTS  
WBA # 22-056

NO.	DESCRIPTION	DATE
4	Revision 4	3.30.2023

# TABLE OF CONTENTS

## **DIVISION 00 | PROCUREMENT AND CONTRACTING REQUIREMENTS**

- 00.0110 – Table of Contents
- 00.0113 – Advertisement for Bids
- 00.2113 – Instruction to Bidders
- 00.3100 – Available Project Information
- 00.4100 – Bid Form
- 00.5000 – Contracting Forms and Supplements
- 00.6000 – Bonds and Certificates
- 00.7200 – General Conditions
- 00.7300 – Supplementary Conditions
- 00.8100 – Federal Labor Wage Rate Compliance Forms
- 00.9100 – Bidding Addendum

## **DIVISION 01 | GENERAL REQUIREMENTS**

- 01.1000 – Summary
- 01.2000 – Price and Payment Procedures
- 01.2100 – Allowances
- 01.2200 – Unit Prices
- 01.2300 – Alternates
- 01.3000 – Administrative Requirements
- 01.3216 – Construction Progress Schedule
- 01.4000 – Quality Requirements
- 01.4216 – Definitions
- 01.4219 – Reference Standards
- 01.5000 – Temporary Facilities and Controls
- 01.6000 – Product Requirements
- 01.7000 – Execution and Closeout Requirements
- 01.7100 – Final Cleaning
- 01.7800 – Closeout Submittals
- 01.7900 – Demonstration and Training

## **DIVISIONS 02 | EXISTING CONDITIONS**

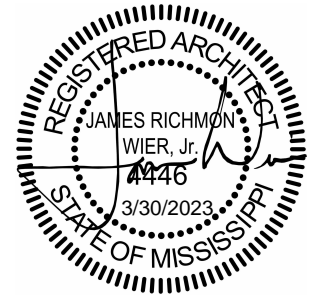
- 02.3000 – Subsurface Investigation
- 02.4100 – Demolition

## **DIVISION 03 | CONCRETE**

- 03.0516 – Underslab Vapor Barrier
- 03.1000 – Concrete Formwork
- 03.2000 – Concrete Reinforcement
- 03.3000 – Cast-In-Place Concrete

## **DIVISION 04 | MASONRY - VERIFY W/ STRUCT SPEC**

- 04.0513 – Masonry Mortaring
- 04.0516 – Masonry Grout
- 04.2000 – Unit Masonry



04.2200 – Concrete Masonry Units

#### **DIVISION 05 | METALS**

05.1200 – Structural Steel  
05.2100 – Steel Joists  
05.3100 – Steel Deck  
05.5000 – Metal Fabrications  
05.5213 – Pipe and Tube Railings

#### **DIVISION 06 | WOOD, PLASTICS, AND COMPOSITES**

06.1000 – Rough Carpentry  
06.4100 – Architectural Wood Casework

#### **DIVISION 07 | THERMAL AND MOISTURE PROTECTION**

07.1300 – Sheet Waterproofing  
07.1400 – Fluid-Applied Waterproofing  
07.2100 – Thermal Insulation  
07.2500 – Weather Barriers  
07.4123 – Metal Wall Panels  
07.5200 – Modified Bituminous Membrane Roofing  
07.6200 – Sheet Metal Flashing and Trim  
07.7100 – Roof Specialties  
07.7200 – Roof Accessories  
07.9005 – Joint Sealers

#### **DIVISION 08 | OPENINGS**

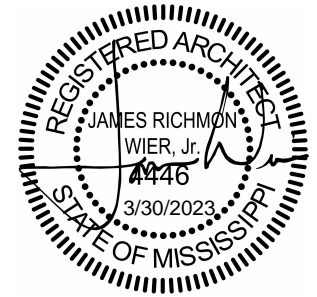
08.1113 – Hollow Metal Doors and Frames  
08.1416 – Flush Wood Doors  
08.3100 – Access Doors and Panels  
08.4315 – Aluminum-Framed Storefronts  
08.7100 – Door Hardware  
08.8000 – Glazing  
08.8300 – Mirrors  
08.8723 – Safety and Security Films  
08.9200 – Louvered Equipment Enclosures

#### **DIVISION 09 | FINISHES**

09.2116 – Gypsum Board Assemblies  
09.3000 – Tiling  
09.5100 – Acoustical Ceilings  
09.6566 – Resilient Athletic Flooring  
09.6813 – Tile Carpeting  
09.8430 – Sound-Absorbing Wall Units  
09.9113 – Exterior Painting  
09.9123 – Interior Painting

#### **DIVISION 10 | SPECIALTIES**

10.1101 – Visual Display Units



- 10.1400 – Signage
- 10.2113.19 – Plastic Toilet Compartments
- 10.2800 – Toilet, Bath and Laundry Accessories
- 10.5114 – Locker Room Specialties
- 10.4400 – Fire Protection Specialties
- 10.7300 – Architectural Canopies
- 10.8213 – Exterior Grilles and Screens

**DIVISION 11 | EQUIPMENT**

- 11.6623– Gymnasium Equipment

**DIVISION 12 | FURNISHINGS**

- 12.2400 – Window Shades
- 12.6100 – Fixed Audience Seating

**DIVISIONS 13-19 | NOT USED**

**DIVISION 20| FIRE PROTECTION, PLUMBING AND HVAC GENERAL PROVISIONS**

- 20.0010– Mechanical General Provisions
- 20.0020– Basic Mechanical Requirements
- 20.0030– Mechanical Submittals and Shop Drawings
- 20.0035– Mechanical Systems and Equipment Warranties
- 20.0040– Mechanical Close-Out Requirements
- 20.0050– Basic Mechanical Materials and Methods
- 20.0060– Pipes and Pipe Fittings
- 20.0100– Valves
- 20.0120– Piping Specialties
- 20.0140– Supports and Anchors
- 20.0170– Electrical Requirements
- 20.0190– Mechanical Identification
- 20.0240– Mechanical Sound and Vibration Control
- 20.0250– Mechanical Insulation

**DIVISION 21 | FIRE SUPPRESSION**

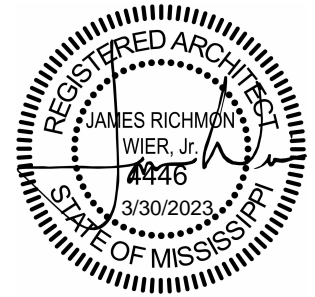
- 21.0330– Fire Sprinkler System

**DIVISION 22 | PLUMBING**

- 22.0430– Plumbing Specialties
- 22.0440– Plumbing Fixtures, Trim and Accessories
- 22.0450– Domestic Water Heaters and Accessories

**DIVISION 23 | HEATING, VENTILATING, AND AIR CONDITIONING (HVAC)**

- 23.0670– Packaged Air Conditioners
- 23.0830– Heating / Cooling Terminal Units
- 23.0860– Fans
- 23.0885– Air Cleaning/Treatment
- 23.0890– Ductwork
- 23.0910– Ductwork Accessories
- 23.0980– Controls and Instrumentation



23.0990– Testing, Adjusting and Balancing

**DIVISIONS 24-25 | NOT USED**

**DIVISION 26 | ELECTRICAL**

- 26.0511 – Electrical General and Work in Existing Facilities
- 26.0519 – 600V Conductors
- 26.0526 – Grounding and Bonding for Electrical Systems
- 26.0533 – Raceways, Outlet Boxes, and Junction Boxes for Electrical Systems
- 26.0573 – Electrical Studies
- 23.0923 – Switches and Receptacles
- 26.0926 – Vacancy Sensors
- 26.2200 – Transformers
- 26.2400 – Panelboards
- 26.2800 – Disconnects and Separately-Mounted Circuit Breakers
- 26.5100 – Lighting

**DIVISION 27 | COMMUNICATIONS**

- 27.3000 – Data System Cabling
- 27.5117 – Intercom System

**DIVISION 28 | ELECTRONIC SAFETY AND SECURITY**

- 28.1301 – Access Control System Conduits
- 28.1614 – Security Conduit System
- 28.2301 – Camera System
- 28.3100 – Fire Alarm System Conduits

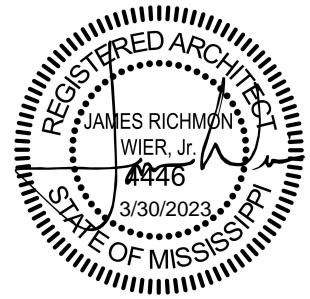
**DIVISION 29-30 | NOT USED**

**DIVISIONS 31 | EARTHWORK**

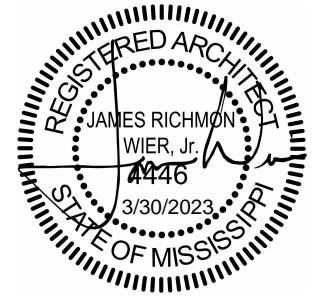
- 31.2000 – Earthwork for Buildings
- 31.2200 – Site Clearing
- 31.2205 – Removal of Obstructions and Structures
- 31.2207 – Removal and Restoration of Improved Surfaces
- 31.2210 – Soil Erosion and Sediment Control
- 31.2220 – Excavation and Embankment
- 31.2230 – Excavation and Backfill for Conduits and Structures
- 31.2240 – Crushed Stone and Clay Gravel
- 31.2271 – Geotextile Fabric
- 31.2271 – Riprap and Slope Paving
- 31.2280 – Establishment of Vegetation
- 31.2282 – Solid Sodding
- 31.3116 – Termite Control
- 31.6329 – Drilled Concrete Piers

**DIVISIONS 32 | EXTERIOR IMPROVEMENTS**

- 32.2475 – Concrete Curbs and Walks
- 32.2500 – Asphalt Paving
- 32.2580 – Pavement Markings



- 32.3112 – Chain Link Fences & Gates
- 32.3300 – Cast-in-Place Reinforced Concrete for Pavements and Sidewalks
- 32.3345 – Concrete Finishing for Pavements and Sidewalks



**DIVISIONS 33 | UTILITIES**

- 33.2710 – High Density Corrugated Polyethylene Pipe
- 33.2720 – Concrete Storm Drain Pipe and Precast Box Culverts
- 33.2730 – Storm Drain Structures
- 33.2800 – Water Distribution System
- 33.2825 – Tracer Wire
- 33.2900 – Wastewater Collection System

**NO ADDITIONAL DIVISIONS**

**DRAWINGS**

See Index on Sheet R101

**END OF SECTION**

SECTION 02.3000  
**SUBSURFACE INVESTIGATION**

**PART 1 GENERAL**

**1.1 SECTION INCLUDES**

- A. Report on Geotechnical Exploration produced by Ladner Testing Laboratories, Inc. and dated February 24, 2023.
  - 1. This report is included herein for the Contractor's reference.
  - 2. Should subsurface conditions be found to vary substantially from this report, changes in the design and construction of the work will be made, with resulting credits or expenditures to the Contract Price/Sum accruing to the Owner.

**1.2 RELATED REQUIREMENTS**

- A. Section 01.1000 - Summary: Contract descriptions, description of alterations work, work by others, future work, occupancy conditions, use of site and premises, work sequence.
- B. Section 01.3000 - Administrative Requirements: Submittal procedures, project meetings, progress schedules and documentation, reports, coordination.
- C. Section 01.4000 - Quality Requirements: Procedures for testing, inspection, mock-ups, reports, certificates; use of reference standards.
- D. Section 01.7000 - Execution and Closeout Requirements: Examination, preparation, and general installation procedures; preinstallation meetings; cutting and patching; cleaning and protection; starting of systems; demonstration and instruction; closeout procedures except payment procedures; requirements for alterations work.

**PART 2 PRODUCTS - NOT USED**

**PART 3 EXECUTION - NOT USED**

**END OF SECTION**

# LADNER TESTING, Inc.

JACKSON  
(601) 362-5421

HATTIESBURG  
(601) 544-5782

GULFPORT  
(228) 604-2527

February 24, 2023

Pearl Public School District  
3375 Highway 80 East  
Pearl, MS 39208

RE: Report of Geotechnical Exploration – Revised Addendum  
Pearl Junior High School Multipurpose Addition  
Pearl, Mississippi

W Geotechnical Project No. G-1311B  
Ladner Project No. 17-23-A

Dear Sir or Madam:

Thank you for retaining Ladner Testing Inc. to complete a geotechnical exploration for the above referenced site. This addendum is in addition to the original report for this project dated February 2, 2023, and replaces the 1<sup>st</sup> Addendum dated February 22, 2023. Please see original report for full recommendations and field investigation results.

We understand that drilled shaft “belled” piers may be used to support the new building. The piers should be installed so that the pile bottoms are supported in the unweathered heavy blue clay at a minimum depth of **35 feet** below the existing ground surface, and the under-reamed portion or “bell” should be designed for an allowable bearing capacity of **8,000 psf**.

Slabs should be designed as structural slabs with a minimum of 12 inches void space beneath the slab. This void space is to allow for heave of the expansive soils without distress to the slab. This may be accomplished in several ways, but one of the most common is the use of void forms. Additional information can be provided upon request.

It is our opinion that for foundations constructed in accordance with the requirements outlined in this report, the maximum total settlement is expected to be less than 1 inch, with the maximum differential settlement between adjacent columns expected to be less than approximately ½ inch.

The above bearing capacity and settlement values are based on our engineering experience with similar soil conditions and the anticipated structural loading, and are to guide the structural engineer with the design. To minimize difficulties during the foundation installation phase, we recommend that Ladner Testing, Inc. be retained to observe pier construction.

This addendum letter has been prepared in accordance with generally accepted geotechnical engineering practice. No other warranty is expressed or implied. The evaluations and recommendations presented in this report are based on the available project information, as well as on the results of the exploration performed for the adjacent building. Ladner Testing, Inc. should be given the opportunity to review the final drawings and site plans for this project to

Report of Geotechnical Exploration – Revised Addendum  
Pearl Junior High School Multipurpose Addition – Pearl, MS  
G-1311B/17-23-A  
February 24, 20223

determine if changes to the recommendations outlined in this report are needed. Should the nature of the project change, these recommendations should be reevaluated. No third party is given permission to rely on this report or data without the express written consent of Ladner Testing Inc.

Thank you for the opportunity to provide geotechnical engineering services on this project. Should you have questions regarding our findings or need additional consultations, please do not hesitate to contact our office.

Respectfully,

**Ladner Testing Laboratories, Inc.**

Represented by:



Heath S. Williams, P.E.  
Principal Engineer  
MS Registration No. 17702

**REPORT OF GEOTECHNICAL EXPLORATION**  
**PEARL JUNIOR HIGH SCHOOL MULTIPURPOSE ADDITION**  
**PEARL, MS**  
**G-1311B/17-23-A**  
**FEBRUARY 2, 2023**

**General**

This report presents the results of our geotechnical exploration findings and our geotechnical recommendations for a proposed new building in Pearl, Mississippi.

**Project Information**

The information presented in this section is based on information provided and our own site reconnaissance. The site is located at 200 Mary Ann, Drive in Pearl, Mississippi. The approximate coordinates of the site are 32.278932°, -90.107073°. We understand that developments include a new building and new parking and drive areas. Standard commercial construction techniques are anticipated.

If any of the information presented is incorrect or has changed, please advise Ladner Testing, Inc. to allow us to reevaluate our recommendations in the light of changes in the present project concept.

**Purposes of Exploration**

The purposes of this exploration were to explore the soil and groundwater conditions at the site and to identify any foreseeable special geotechnical considerations needed for the proposed construction. We accomplished the purposes of the study by:

1. Performing a general site reconnaissance and drilling borings to explore the subsurface soil and groundwater conditions,
2. Performing laboratory tests on selected representative soil samples from the borings to evaluate pertinent engineering properties, and
3. Evaluating the field and laboratory data to develop appropriate geotechnical engineering recommendations.

**Field Exploration**

To explore the subsurface conditions at this site, a total of four (4) Standard Penetration Test (SPT) borings were drilled to depths of 20 feet below the existing ground surface within the footprint of the proposed building. Five (5) auger borings were performed to 8 feet below the existing ground surface in the proposed parking and drive areas. Boring locations were determined in the field by a Ladner Testing representative who measured distances and estimated right angles from existing site features, or by use of a handheld GPS device. The boring locations should be considered approximate, and boring elevations should be considered from the ground surface elevation at the time of our fieldwork on January 16, 2023. The soil test borings were performed with a truck mounted drill rig, which utilized continuous flight solid stem augers to advance the boreholes. Representative soil samples were taken for visual classification and further laboratory testing. The drill crew maintained a field log of the soils encountered in the borings.

## **Laboratory Testing Program**

Representative soil samples were selected and tested in our laboratory to check visual classifications and to determine pertinent engineering properties. The laboratory testing program included visual classifications of all soil samples, and natural moisture content, Atterberg Limit, and sieve analysis testing of selected samples. The laboratory test results are presented on the attached boring logs. An experienced geotechnical technician classified each soil sample on the basis of texture and plasticity in accordance with the Unified Soil Classification System. The group symbols for each soil type are indicated in the appropriate column of the attached boring logs. The geotechnical technician grouped the various soil types into the major zones noted on the boring logs. The stratification lines designating the interfaces between earth materials on the boring logs and profiles are approximate; in-situ, the transitions may be gradual. The soil samples will be retained in our laboratory for a period of 60 days, after which, the samples will be discarded unless instructed otherwise.

## **Subsurface Soil Conditions**

The subsurface soil conditions are presented in more detail on the attached boring logs. The subsurface conditions discussed in the following paragraphs and those shown on the boring logs represent an estimate of the subsurface conditions based on interpretation of the boring data using normally accepted geotechnical engineering judgments. We note that the transition between different soil strata is usually less distinct than those shown on the boring logs. Subsurface conditions in unexplored locations may vary somewhat from those reported herein.

The borings performed for this exploration primarily encountered Lean Clay and Heavy Clay. These strata are generalized in the following paragraphs. Asphalt ranging in thickness from 6 to 8 inches was also observed across the site. For more specific information, refer to the boring logs in the appendix.

### Lean Clay

Material described as lean clay was generally observed from the ground surface to depths typically ranging from 1 to 3 feet below the existing ground surface. Gravel and brick debris were often present in this material. SPT samples were not taken in this material and the shear strength was not evaluated.

### Heavy Clay

Material described as heavy clay was typically encountered below depths ranging from 1 to 3 feet from the existing ground surface. The shear strength of the heavy clay was soft to stiff, with SPT-n values ranging from 4 to 11 blows per foot (bpf). The heavy clays had plasticity index (PI) values ranging from 31 to 75%. These PI values are within a range of high plasticity where the clays are considered prone to shrink and swell with fluctuations in moisture content.

## **Groundwater Conditions**

Visual observation of the soil samples retrieved during the drilling exploration can often be used in evaluating the groundwater conditions. Groundwater was not observed during the field investigation at this site. Variations in the location of the long-term water table may occur as a result of changes in precipitation, evaporation, surface water runoff, and other factors not immediately apparent at the time of this exploration.

## **Geotechnical Recommendations**

The following geotechnical recommendations are based on our observations at the site, interpretation of the field data obtained during the exploration, laboratory test results, and our experience with similar subsurface conditions.

In general, poor soils were encountered on this site. Heavy clay soils that are prone to shrink and swell with fluctuations in moisture content were observed near the ground surface. Site preparation should include excavation and replacement of the entire structure footprint with structural fill materials so that at least 8 feet of structural fill materials separates the lowest foundation members of the structure from the heavy clay soils encountered. The thickness of fill should be uniform across the entire footprint of the new structure.

Limiting differential settlement near heavy clay soils that are prone to shrink and swell is dependent on moisture control around the footprint of the structure. Good site drainage is required to prevent runoff water from infiltrating the subgrade near the structure. We recommend that runoff is not allowed to infiltrate the subgrade within 10 feet of the structure. If runoff and irrigation cannot be diverted away from the structure using surface drainage, we recommend the use of subsurface drainage systems such as French drains. Perforated piping should also be placed in the bottom of the structural fill pads and daylighted to stormwater collection areas if excessive runoff or groundwater are expected near the building pad.

Contractor is solely responsible for supporting excavation sides and structures adjacent to the excavation. Underpinning of the existing structures on the excavation edges may be required. To avoid excavation and replacement of the building subgrade, the new structure can be supported on deep foundations such as helical piles.

## **Foundation Design Recommendations**

Assuming proper site preparation (removal of topsoil, soft / loose soils, debris and organic materials, and 8 feet low plasticity clay fill buffer, uniform fill and compaction), this site appears suitable for the construction of the proposed structure on shallow foundations. We recommend that foundations are designed for a maximum net allowable soil bearing pressure of **2,000** psf. We also recommend that the slabs be made more rigid using perimeter and interior grade beams and by incorporating both top and bottom reinforcing steel into both the footings and grade beams, or by the use of post-tension reinforcement. Interior grade beams should have a maximum spacing of 20 feet.

Settlement values are based on the stated assumption that the site is properly prepared, and any deleterious material will be removed if found during construction. Conventional values for allowable settlement are typically 1 inch of total settlement with 1/2 inch of differential settlement. It is our opinion that for structures supported on shallow foundations constructed in accordance with the requirements outlined in this report, the maximum total settlement is expected to be less than 1 inch, with the maximum differential settlement expected to be up to 1/2 inch.

To reduce the risk of foundation bearing failure and excessive settlement due to local shear or "punching" action, we recommend that continuous footings have a minimum width of 1.5 feet and that isolated column footings have a minimum lateral dimension of 2 feet. In addition, footings should be placed at a depth to provide adequate bearing capacity and resist undermining of the footing by erosion. For this site, we recommend that footing bottoms are placed at a minimum depth of 2 feet below lowest adjacent finished grade.

The above bearing capacity and settlement values are based on our engineering experience with similar soil conditions and the anticipated structural loading, and are to guide the structural engineer with the foundation design. To minimize difficulties during the foundation installation phase, we recommend that Ladner Testing, Inc. be retained to observe the foundation bearing surfaces and to confirm that the recommended bearing pressures are developed.

As an alternative to shallow foundations, helical piles can be used to support the structure at the site. Helical piles should be installed to a depth where torque measurements indicate that the required capacity is achieved with appropriate factors of safety. We recommend that piles be installed to at least 30 feet below the existing grade, but depending on loads required, piles may need to be installed much deeper. Void space with a structural slab is also required to allow movement of the subgrade soil. Helical piles are typically a proprietary product and suppliers provide their own design support. Helical pile contractor should ensure that settlements are limited to less than 1 inch. For a potential supplier/installer, you may contact Wayne Nall of Ramjack Mississippi at 601-707-3012 ([wnall@ramjackms.com](mailto:wnall@ramjackms.com)) or Jim Parrish with Southern Industrial Piling at 870-866-9002 ([jparrish@sipiling.com](mailto:jparrish@sipiling.com)), for more details and estimated costs. Other foundation solutions can be discussed upon request.

### **Pavement Design Recommendations**

*Assuming that the pavement footprint is cleared of topsoil, organic materials and debris, and thoroughly proofrolled as described in the subgrade preparation section of this report, a typical minimum pavement section for the expected soil subgrade is shown below. Additionally, heavy clay must not be present within 12 inches of the pavement base materials. If CBR values of less than 5 are encountered at the subgrade elevation at the time of construction, the subgrade may need to be stabilized by over excavation and replacement or by use of a geogrid reinforcement such as Tensar® TX5. Lime treatment can be specified as Class C.*

**Typical Asphalt Pavement Sections**

<b>Material Type</b>	<b>Parking Stalls and Light Duty Driveways</b>	<b>Heavy Duty Truck Driveways</b>
AC Surface Course (ST 9.5mm)	2.0 inches	2.0 inches
AC Base Course (ST 12.5mm)	4.0 inches	6.0 inches
Crushed Aggregate Base (#610 Gradation) OR	6.0 inches	8.0 inches
Lime Treated Base (6% By Weight)	12.0 inches	12.0 inches

The heavy-duty truck driveways section should be used in areas expecting heavy trucks, deliveries and/or garbage truck service. As an alternative option to asphalt, typical rigid pavement sections are also provided below. At a minimum, we recommend that rigid concrete sections are used in areas with tight turns, frequent braking, entry and exit aprons, dumpster pads, and large or sustained loads. Concrete joint spacing, reinforcement and layout should be designed in accordance with American Concrete Institute (ACI) 330.

**Typical Rigid Pavement Sections**

<b>Material Type</b>	<b>Parking Stalls and Light Duty Driveways</b>	<b>Heavy Duty Truck Driveways</b>
4,000 psi Concrete	6.0 inches	8.0 inches
Crushed Aggregate Base (#610 Gradation) OR	6.0 inches	8.0 inches
Lime Treated Base (6% By Weight)	12.0 inches	12.0 inches

An important consideration with the design and construction of pavements is surface and subsurface drainage. Where standing water develops, either on the pavement surface or within the base course layer, softening of the subgrade and other problems related to the deterioration of the pavement can be expected. Furthermore, good drainage should minimize the risk of the subgrade materials becoming saturated over a long period of time. The use of a triaxial geosynthetic, such as Geogrid® TX5, may be used to stabilize the subgrade if needed.

**Subgrade Preparation**

The subgrade preparation should consist of removing topsoil, organic materials, loose or soft soils, piping, concrete, old utilities and any miscellaneous debris encountered. Observation is required to ensure that all the unwanted material is removed. We also recommend that at least 8 feet of structural fill materials separate the lowest foundation members of the structure from any heavy clay soils. We recommend that the thickness of fill is uniform across the entire footprint of the structure. At least 12 inches of structural fill should underly the base layer of the parking and drive areas. Compaction should meet the specifications in the Fill Placement section. We recommend that the subgrade preparation be extended to the expanded project limits, which are defined as a minimum of 8 feet beyond the footprint of the structure and 3 feet beyond the edge of any pavement.

The prepared subgrade must be observed to be free of substantial amounts of organic material and of sufficient consistency to support the required structural loads. The resulting subgrade should be evaluated by a qualified geotechnical technician. *Proofrolling of the entire site using a loaded dump truck, having an axle weight of at least 10 tons, is required to aid in identifying any additional localized soft or unsuitable material that should be removed.* Any soft or unsuitable materials encountered during this proofrolling should be removed and replaced with an approved backfill compacted to the criteria below.

### **Fill Placement**

The preparation of proposed subgrade should be observed on a periodic basis by a representative of a qualified construction materials testing company to document that the subgrade is suitable for support of the proposed construction and/or fills. Structural fill materials should consist of approved material with less than 2 percent organic matter, free of debris, with rocks no greater than 6 inches and a Liquid Limit less than 40 and a Plasticity Index between 10 and 20. Unacceptable fill materials include topsoil, ash, low-density soils with a maximum unit weight less than 95 pcf, organic materials, and highly plastic silts and clays. The lean clay soils observed at the site are generally considered suitable for use as fill materials.

Grade control should be maintained throughout the fill placement operations. All fill operations should be observed on a periodic basis by a qualified soil technician from Ladner Testing, Inc. to determine that minimum compaction requirements are being met. A minimum of one compaction test per 2,500 square foot area should be performed in every other lift placed. The elevation and location of the tests should be clearly identified and recorded at the time of fill placement. Fill materials should be placed in lifts not exceeding 8 inches in loose thickness and moisture conditioned to within  $\pm 2$  percent of the optimum moisture content to facilitate proper compaction. Structural fill material should be compacted to a minimum of **98 percent** of the maximum dry density obtained in accordance with ASTM Specification D-698, Standard Proctor Method.

### **Additional Considerations**

Exposure to the environment may weaken the soils at the subgrade level if excavations remain open for too long a time. If surface water intrusion or exposure softens the bearing soils, the softened soils must be removed from the excavation bottom immediately prior to placement of fill.

Positive site drainage should be maintained during earthwork operations, which should help maintain the integrity of the soil. Placement of fill on the near surface soils, which have become saturated, may be very difficult. When wet, these soils will degrade quickly with disturbance from contractor operations and will be extremely difficult to stabilize for fill placement.

The surface of the site should be kept properly graded in order to enhance drainage of the surface water away from the proposed building areas during the construction phase. We recommend that an attempt be made to enhance the natural drainage without interrupting its pattern.

Report of Geotechnical Exploration  
Pearl Junior High School Multipurpose Addition - Pearl, MS  
G-1311B/17-23-A  
February 2, 2023

The surficial soils contain fines, which are considered moderately erodible. All erosion and sedimentation shall be controlled in accordance with Best Management Practices and current City and DEQ requirements. At the appropriate time, we would be pleased to provide a proposal for construction materials testing and related services.

### **Closing**

This report has been prepared in accordance with generally accepted geotechnical engineering practice. No other warranty is expressed or implied. The evaluations and recommendations presented in this report are based on the available project information, as well as on the results of the exploration. Ladner Testing, Inc. should be given the opportunity to review the final drawings and site plans for this project to determine if changes to the recommendations outlined in this report are needed. Should the nature of the project change, these recommendations should be reevaluated. No third party is given permission to rely on this report or data without the express written consent of Ladner Testing, Inc. We recommend that the construction activities be observed by a qualified geotechnical engineer to provide the necessary overview and to check the suitability of the subgrade soils for supporting the footings. We would be pleased to provide an estimated cost for these services at the appropriate time.



# ladner testing laboratories, inc

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 2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782  
 P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> PEARL JUNIOR HIGH SCHOOL MULTIPURPOSE ADDITION PEARL, MS	<b>CLIENT:</b> PEARL PUBLIC SCHOOL DISTRICT 3375 HIGHWAY 80 EAST  PEARL, MS 39208	<b>DATE:</b> 1/16/2023 <b>LAB NO:</b> 0017-23-A <b>BORE NO:</b> B-1 <b>TECHNICIAN:</b> B.H. / D.I.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0----- -----		8" ASPHALT PAVING GRAY, TAN, & RED LEAN CLAY (0 - 1')		20.2	45.0	26.0	94.1	CL	
----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (1 - 3.5')	MEDIUM	24.5	60.0	38.0	95.5	CH	7
----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (3.5 - 10')	STIFF	33.6	69.0	44.0	95.9	CH	11
5----- -----									
----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (10 - 20')	MEDIUM	42.7	98.0	64.0	95.0	CH	8
10----- -----									
----- -----	X		MEDIUM						6
15----- -----									
----- -----	X		SOFT						4
20----- -----									
----- -----									
25----- -----									
----- -----									
30----- -----									

<b>WATER DEPTH</b> _____ <b>0</b> _____ <b>FT.</b>	<b>AFTER</b> _____ <b>0</b> _____ <b>HRS.</b>	<b>BORING ELEVATION</b> _____ <b>0</b> _____ <b>FT.</b>
<b>WATER DEPTH</b> _____ <b>0</b> _____ <b>FT.</b>	<b>AFTER</b> _____ <b>0</b> _____ <b>HRS.</b>	<b>BORING TERMINATED AT</b> _____ <b>20</b> _____ <b>FT.</b>





# ladner testing laboratories, inc

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 2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782  
 P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> PEARL JUNIOR HIGH SCHOOL MULTIPURPOSE ADDITION PEARL, MS	<b>CLIENT:</b> PEARL PUBLIC SCHOOL DISTRICT 3375 HIGHWAY 80 EAST  PEARL, MS 39208	<b>DATE:</b> 1/16/2023 <b>LAB NO:</b> 0017-23-A <b>BORE NO:</b> B-3 <b>TECHNICIAN:</b> B.H. / D.I.
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**SAMPLES:**  AUGER(ASTM D-1452)  TUBE(ASTM D-1587)  PENETRATION TEST(ASTM D-1586)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0----- -----		GRAY & TAN LEAN CLAY (0 - 1')		21.2	43.0	24.0	72.9	CL	
----- ----- ----- -----	X	TAN, GRAY, & RED HEAVY CLAY (YAZOO CLAY) (1 - 3.5')	SOFT	25.9	56.0	34.0	91.4	CH	4
----- ----- ----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (3.5 - 10')	MEDIUM	31.9	78.0	50.0	97.4	CH	7
5----- ----- ----- -----									
----- ----- ----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (10 - 20')	MEDIUM	38.5	109.0	75.0	96.3	CH	6
10----- ----- ----- -----									
----- ----- ----- -----	X		MEDIUM						6
15----- ----- ----- -----									
----- ----- ----- -----	X		SOFT						5
20----- ----- ----- -----									
----- ----- ----- -----									
25----- ----- ----- -----									
----- ----- ----- -----									
30----- ----- ----- -----									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
 WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 20 FT.



# ladner testing laboratories, inc

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 2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782  
 P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> PEARL JUNIOR HIGH SCHOOL MULTIPURPOSE ADDITION PEARL, MS	<b>CLIENT:</b> PEARL PUBLIC SCHOOL DISTRICT 3375 HIGHWAY 80 EAST  PEARL, MS 39208	<b>DATE:</b> 1/16/2023 <b>LAB NO:</b> 0017-23-A <b>BORE NO:</b> B-4 <b>TECHNICIAN:</b> B.H. / D.I.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input type="checkbox"/> TUBE(ASTM D-1587)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1586)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0----- -----		8" ASPHALT PAVING GRAY LEAN CLAY W/ BRICK DEBRIS (0 - 1')		19.5	42.0	25.0	87.4	CL	
----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (1 - 2.5')	STIFF	26.6	66.0	38.0	89.0	CH	9
----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (2.5 - 5')		25.5	58.0	34.0	96.9	CH	
5----- -----	X	TAN & GRAY HEAVY CLAY (YAZOO CLAY) (5 - 20')	MEDIUM	35.9	96.0	61.0	98.0	CH	8
----- -----	X		MEDIUM						6
10----- -----	X		MEDIUM						6
----- -----	X		MEDIUM						7
15----- -----	X		MEDIUM						
----- -----	X		MEDIUM						
20----- -----	X		MEDIUM						
----- -----									
----- -----									
25----- -----									
----- -----									
----- -----									
30----- -----									

WATER DEPTH	0	FT.	AFTER	0	HRS.	BORING ELEVATION	0	FT.
WATER DEPTH	0	FT.	AFTER	0	HRS.	BORING TERMINATED AT	20	FT.





# ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421  
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782  
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

<b>PROJECT:</b> PEARL JUNIOR HIGH SCHOOL MULTIPURPOSE ADDITION PEARL, MS	<b>CLIENT:</b> PEARL PUBLIC SCHOOL DISTRICT 3375 HIGHWAY 80 EAST  PEARL, MS 39208	<b>DATE:</b> 2/8/2023 <b>LAB NO:</b> 0017-23-A <b>BORE NO:</b> B-5 <b>TECHNICIAN:</b> B.H. / D.I.
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<b>SAMPLES:</b>	<input type="checkbox"/> AUGER(ASTM D-1452)	<input type="checkbox"/> TUBE(ASTM D-1582)	<input checked="" type="checkbox"/> PENETRATION TEST(ASTM D-1596)
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DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
30----									
35----	X	BLUE UNWEATHERED HEAVY CLAY (YAZOO CLAY) ( 35' - 40')	VERY HARD	37.0				CH	77
40----									
45----									
50----									
55----									
60----									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.  
WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 40 FT.











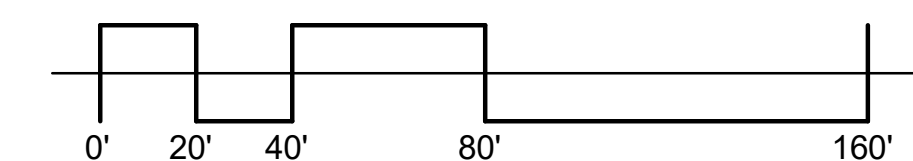
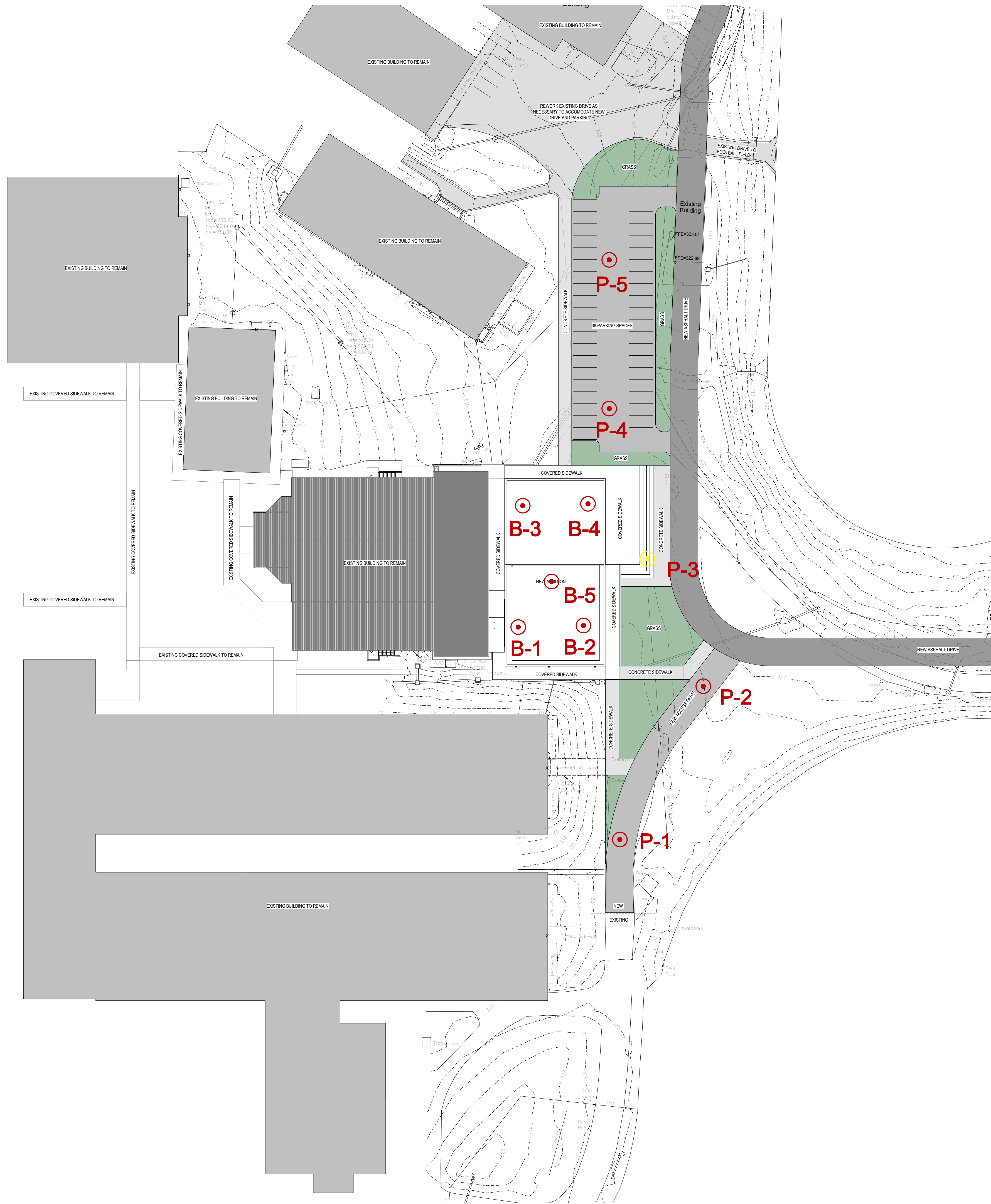
**PEARL JUNIOR HIGH SCHOOL  
MULTIPURPOSE ADDITION  
PEARL, MS**

**LAB NO.: 0017-23-A**



**PPSD JH  
MULTIPURPOSE**

200 Mary Ann Drive  
Pearl, MS 39208



# Louvered Equipment Enclosures

## PART 1 GENERAL

1. SECTION INCLUDES
  - A. Louvered aluminum screens for concealing rooftop and grade-mounted equipment as indicated in the drawings.
2. RELATED REQUIREMENTS
  - A. Section 05.5000 - Metal Fabrications: Superstructure support and bracing of equipment screens and grilles.
3. REFERENCE STANDARDS
  - A. AAMA 2605 - Voluntary Specification, Performance Requirements and Test Procedures for Superior Performing Organic Coatings on Aluminum Extrusions and Panels (with Coil Coating Appendix) 2017a.
  - B. ASTM B209 - Standard Specification for Aluminum and Aluminum-Alloy Sheet and Plate 2014.
  - C. ASTM B209M - Standard Specification for Aluminum and Aluminum-Alloy Sheet and Plate (Metric) 2014.
  - D. ASTM B221 - Standard Specification for Aluminum and Aluminum-Alloy Extruded Bars, Rods, Wire, Profiles, and Tubes 2020.
4. SUBMITTALS
  - A. See Section 01.3000 - Administrative Requirements - Administrative Requirements, for submittal procedures.
  - B. Product Data: Manufacturer's data sheets on each product to be used, including:
    1. Storage and handling requirements and recommendations.
    2. Installation methods.
  - C. Shop Drawings: Include plans, sections, and details of connections and bracing.
    1. Include structural calculations indicating compliance with wind loading requirements.
    2. Show field measurements of roof openings on shop drawings.
  - D. Selection Samples: For each finish product specified, two complete sets of color chips representing manufacturer's full range of available finishes, colors, and textures.
  - E. Verification Samples: For each product specified, two samples, minimum size 8 inches (200 mm) square, representing actual product configuration, color, and texture.
  - F. Manufacturer's Qualification Statement.
5. QUALITY ASSURANCE
  - A. Manufacturer Qualifications: Company specializing in manufacturing products of the type specified in this section, with not less than ten years of documented experience.
6. DELIVERY, STORAGE, AND HANDLING
  - A. Store products in manufacturer's unopened packaging until ready for installation.
  - B. Comply with manufacturer's instructions for handling of grille and screen products.
7. WARRANTY
  - A. See Section 01.7800 - Closeout Submittals, for additional warranty requirements.
  - B. Submit manufacturer's standard ten-year finish warranty.

## PART 2 PRODUCTS

1. MANUFACTURERS
  - A. Louvered Equipment Enclosures:
    1. Omega II Fence Systems - A division of Metaltech - Omega Inc. [www.omegatwo.com](http://www.omegatwo.com)
    2. Substitutions: See Section 01.6000 - Product Requirements.
  - B. Basis of Design : Omega II Louver 100 System. Orsogrill Style. 100% concealment and 35% free-air flow.
  - C. Louver spacing: 1-3/4".
  - D. Color: Custom Color to match RAL number provided by Architect.

- E. Accessories: Clip angles and/or other steel members for anchorage to structural steel framing.
  - F. Hinged Panels: as indicated in drawings.
  - G. Provide lockable latch.
2. PERFORMANCE REQUIREMENTS
- A. Wind Resistance: Design grilles and screens, including superstructure support system, to withstand positive and negative wind loading in accordance with applicable building code.
3. ACCESSORIES
- A. Miscellaneous Trim: Aluminum sheet, alloy 3005-H25 temper, or equivalent in accordance with ASTM B209 (ASTM B209M), formed to shapes indicated and finished to match other components.
  - B. Support Structure: As specified in Section 05.5000.
  - C. Fasteners: Self-tapping stainless steel screws, as approved by manufacturer of rooftop equipment screens.

### **PART 3 EXECUTION**

1. EXAMINATION
- A. Do not begin installation until substrates have been properly prepared.
  - B. If substrate preparation is the responsibility of another installer, notify Architect of unsatisfactory preparation before proceeding.
2. PREPARATION
- A. Prepare surfaces using the methods recommended by the manufacturer for achieving the best result for the substrate under the project conditions.
3. INSTALLATION
- A. Install rooftop equipment screens in accordance with manufacturer's printed instructions and as indicated on shop drawings.
  - B. Form tight joints and fit exposed connections accurately.
  - C. Provide necessary fastenings and anchors required for a complete installation, and install units plumb, level, and in proper alignment with adjacent work.
4. PROTECTION
- A. Protect installed products until completion of project.
  - B. Protect metal surfaces from corrosion or galvanic action by application of a heavy coating of bituminous paint on surfaces that will be in contact with concrete, masonry or dissimilar metals.
  - C. Touch-up, repair or replace damaged products before Date of Substantial Completion.

### **END OF SECTION**



"General Decision Number: MS20230053 01/27/2023 22-056 Pearl Junior High Multipurpose  
Addendum # 4, Item No. 4

Superseded General Decision Number: MS20220053

State: Mississippi

Construction Type: Building  
BUILDING CONSTRUCTION PROJECTS (does not include single family homes or apartments up to and including 4 stories).

County: Rankin County in Mississippi.

Note: Contracts subject to the Davis-Bacon Act are generally required to pay at least the applicable minimum wage rate required under Executive Order 14026 or Executive Order 13658. Please note that these Executive Orders apply to covered contracts entered into by the federal government that are subject to the Davis-Bacon Act itself, but do not apply to contracts subject only to the Davis-Bacon Related Acts, including those set forth at 29 CFR 5.1(a)(2)-(60).

<p>If the contract is entered into on or after January 30, 2022, or the contract is renewed or extended (e.g., an option is exercised) on or after January 30, 2022:</p>	<ul style="list-style-type: none"> <li>. Executive Order 14026 generally applies to the contract.</li> <li>. The contractor must pay all covered workers at least \$16.20 per hour (or the applicable wage rate listed on this wage determination, if it is higher) for all hours spent performing on the contract in 2023.</li> </ul>
<p>If the contract was awarded on or between January 1, 2015 and January 29, 2022, and the contract is not renewed or extended on or after January 30, 2022:</p>	<ul style="list-style-type: none"> <li>. Executive Order 13658 generally applies to the contract.</li> <li>. The contractor must pay all covered workers at least \$12.15 per hour (or the applicable wage rate listed on this wage determination, if it is higher) for all hours spent performing on that contract in 2023.</li> </ul>

The applicable Executive Order minimum wage rate will be adjusted annually. If this contract is covered by one of the Executive Orders and a classification considered necessary for performance of work on the contract does not appear on this wage determination, the contractor must still submit a conformance request.

Additional information on contractor requirements and worker protections under the Executive Orders is available at <http://www.dol.gov/whd/govcontracts>.

Modification Number	Publication Date
0	01/06/2023
1	01/27/2023

ENGI0624-007 01/01/2017

22-056 Pearl Junior High Multipurpose  
Addendum # 4, Item No. 4

	Rates	Fringes
POWER EQUIPMENT OPERATOR		
Crane.....	\$ 26.20	12.30
Forklift.....	\$ 26.20	12.30

\* IRON0798-009 10/01/2022

	Rates	Fringes
IRONWORKER, REINFORCING.....	\$ 28.50	17.02

PLUM0568-008 11/01/2022

	Rates	Fringes
PIPEFITTER.....	\$ 28.41	10.57

\* SUMS2015-014 04/03/2017

	Rates	Fringes
BRICKLAYER.....	\$ 20.29	0.00
CARPENTER, Includes Drywall Hanging, and Form Work.....	\$ 18.98	1.82
CEMENT MASON/CONCRETE FINISHER...	\$ 16.92	0.00
ELECTRICIAN.....	\$ 19.87	3.36
IRONWORKER, STRUCTURAL.....	\$ 17.69	0.00
LABORER: Common or General.....	\$ 11.57 **	1.59
LABORER: Mason Tender - Cement/Concrete.....	\$ 16.00 **	1.28
OPERATOR: Backhoe/Excavator/Trackhoe.....	\$ 31.50	4.25
OPERATOR: Bulldozer.....	\$ 15.14 **	1.03
PAINTER (Brush and Roller).....	\$ 16.00 **	0.00
PLUMBER.....	\$ 19.78	2.14
ROOFER.....	\$ 16.93	0.00
SHEET METAL WORKER, Includes HVAC Duct Installation.....	\$ 18.00	0.35
TILE SETTER.....	\$ 18.00	0.00
TRUCK DRIVER: Dump Truck.....	\$ 14.47 **	0.41

WELDERS - Receive rate prescribed for craft performing  
operation to which welding is incidental.

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\*\* Workers in this classification may be entitled to a higher minimum wage under Executive Order 14026 (\$16.20) or 13658 (\$12.15). Please see the Note at the top of the wage determination for more information.

22-056 Pearl Junior High Multipurpose  
Addendum # 4, Item No. 4

Note: Executive Order (EO) 13706, Establishing Paid Sick Leave for Federal Contractors applies to all contracts subject to the Davis-Bacon Act for which the contract is awarded (and any solicitation was issued) on or after January 1, 2017. If this contract is covered by the EO, the contractor must provide employees with 1 hour of paid sick leave for every 30 hours they work, up to 56 hours of paid sick leave each year. Employees must be permitted to use paid sick leave for their own illness, injury or other health-related needs, including preventive care; to assist a family member (or person who is like family to the employee) who is ill, injured, or has other health-related needs, including preventive care; or for reasons resulting from, or to assist a family member (or person who is like family to the employee) who is a victim of, domestic violence, sexual assault, or stalking. Additional information on contractor requirements and worker protections under the EO is available at <https://www.dol.gov/agencies/whd/government-contracts>.

Unlisted classifications needed for work not included within the scope of the classifications listed may be added after award only as provided in the labor standards contract clauses (29CFR 5.5 (a) (1) (ii)).

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The body of each wage determination lists the classification and wage rates that have been found to be prevailing for the cited type(s) of construction in the area covered by the wage determination. The classifications are listed in alphabetical order of ""identifiers"" that indicate whether the particular rate is a union rate (current union negotiated rate for local), a survey rate (weighted average rate) or a union average rate (weighted union average rate).

#### Union Rate Identifiers

A four letter classification abbreviation identifier enclosed in dotted lines beginning with characters other than ""SU"" or ""UAVG"" denotes that the union classification and rate were prevailing for that classification in the survey. Example: PLUM0198-005 07/01/2014. PLUM is an abbreviation identifier of the union which prevailed in the survey for this classification, which in this example would be Plumbers. 0198 indicates the local union number or district council number where applicable, i.e., Plumbers Local 0198. The next number, 005 in the example, is an internal number used in processing the wage determination. 07/01/2014 is the effective date of the most current negotiated rate, which in this example is July 1, 2014.

Union prevailing wage rates are updated to reflect all rate changes in the collective bargaining agreement (CBA) governing this classification and rate.

#### Survey Rate Identifiers

Classifications listed under the ""SU"" identifier indicate that

no one rate prevailed for this classification in the survey and the published rate is derived by computing a weighted average rate based on all the rates reported in the survey for that classification. As this weighted average rate includes all rates reported in the survey, it may include both union and non-union rates. Example: SULA2012-007 5/13/2014. SU indicates the rates are survey rates based on a weighted average calculation of rates and are not majority rates. LA indicates the State of Louisiana. 2012 is the year of survey on which these classifications and rates are based. The next number, 007 in the example, is an internal number used in producing the wage determination. 5/13/2014 indicates the survey completion date for the classifications and rates under that identifier.

22-056 Pearl Junior High Multipurpose  
Addendum # 4, Item No. 4

Survey wage rates are not updated and remain in effect until a new survey is conducted.

#### Union Average Rate Identifiers

Classification(s) listed under the UAVG identifier indicate that no single majority rate prevailed for those classifications; however, 100% of the data reported for the classifications was union data. EXAMPLE: UAVG-OH-0010 08/29/2014. UAVG indicates that the rate is a weighted union average rate. OH indicates the state. The next number, 0010 in the example, is an internal number used in producing the wage determination. 08/29/2014 indicates the survey completion date for the classifications and rates under that identifier.

A UAVG rate will be updated once a year, usually in January of each year, to reflect a weighted average of the current negotiated/CBA rate of the union locals from which the rate is based.

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#### WAGE DETERMINATION APPEALS PROCESS

1.) Has there been an initial decision in the matter? This can be:

- \* an existing published wage determination
- \* a survey underlying a wage determination
- \* a Wage and Hour Division letter setting forth a position on a wage determination matter
- \* a conformance (additional classification and rate) ruling

On survey related matters, initial contact, including requests for summaries of surveys, should be with the Wage and Hour National Office because National Office has responsibility for the Davis-Bacon survey program. If the response from this initial contact is not satisfactory, then the process described in 2.) and 3.) should be followed.

With regard to any other matter not yet ripe for the formal process described here, initial contact should be with the Branch of Construction Wage Determinations. Write to:

Branch of Construction Wage Determinations  
Wage and Hour Division  
U.S. Department of Labor  
200 Constitution Avenue, N.W.  
Washington, DC 20210

22-056 Pearl Junior High Multipurpose  
Addendum # 4, Item No. 4

2.) If the answer to the question in 1.) is yes, then an interested party (those affected by the action) can request review and reconsideration from the Wage and Hour Administrator (See 29 CFR Part 1.8 and 29 CFR Part 7). Write to:

Wage and Hour Administrator  
U.S. Department of Labor  
200 Constitution Avenue, N.W.  
Washington, DC 20210

The request should be accompanied by a full statement of the interested party's position and by any information (wage payment data, project description, area practice material, etc.) that the requestor considers relevant to the issue.

3.) If the decision of the Administrator is not favorable, an interested party may appeal directly to the Administrative Review Board (formerly the Wage Appeals Board). Write to:

Administrative Review Board  
U.S. Department of Labor  
200 Constitution Avenue, N.W.  
Washington, DC 20210

4.) All decisions by the Administrative Review Board are final.

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END OF GENERAL DECISIO"